GENERAL STRUCTURAL NOTES:

1. BUILDING CODE: BUILDING CODE OF NORTH CAROLINA STATE, LATEST EDITION
2. CONSTRUCTION LOADING: DURING CONSTRUCTION, THE GENERAL CONTRACTOR SHALL LIMIT AND CONTROL CONSTRUCTION LOADING, INCLUDING BUT NOT LIMITED TO:
   a. MATERIAL STOCKPILING AND EQUIPMENT TO PRECLUDE OVERSTRESSING, CONSTRUCTION LIVE LOAD IN EXCESS OF 20 PSF, OR DAMAGE TO ANY STRUCTURAL ELEMENT.
3. COORDINATION WITH OTHER DISCIPLINES: THE CONTRACTOR SHALL COORDINATE ALL STRUCTURAL WORK WITH THE ARCHITECTURAL, ELECTRICAL, MECHANICAL, PLUMBING AND FIRE PROTECTION DRAWINGS AND SPECIFICATIONS.
4. EXISTING CONDITIONS: THE INFORMATION SHOWN ON THESE DOCUMENTS IS THE BEST REPRESENTATION OF EXISTING CONDITIONS AVAILABLE TO THE ENGINEER. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY AND BRING TO THE ENGINEER'S AND CONSTRUCTION MANAGER'S ATTENTION ANY DISCREPANCIES PRIOR TO COMMENCING WORK.
5. EXISTING STRUCTURES: ALL EXISTING STRUCTURES ADJACENT TO NEW WORK ARE TO BE ADEQUATELY PROTECTED AND/OR SUPPORTED DURING CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REPAIRING ANY NEW OR EXISTING CONSTRUCTION DAMAGED WHILE WORK IS IN PROGRESS.
6. OPENINGS: THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING SIZE AND LOCATION OF ALL OPENINGS IN NEW AND EXISTING CONSTRUCTION WITH THE DISCIPLINE REQUIRING THEM.

FOUNDATION NOTES:

1b. (NO GEOTECHNICAL INFORMATION WAS AVAILABLE AT THE TIME OF DESIGN. ASSUMED ALLOWABLE BEARING PRESSURE = 1,500 PSF.)
2. TAKE ALL NECESSARY PRECAUTIONS WHEN EXCAVATING OR DRILLING ADJACENT TO EXISTING STRUCTURES TO AVOID DISTURBING EXISTING FOUNDATIONS. DO NOT EXCAVATE BELOW EXISTING FOUNDATIONS. CONTACT THE ENGINEER IF EXISTING CONDITIONS DIFFER FROM THOSE SHOWN ON THE DRAWING.
3. ALL EXCAVATIONS SHALL FULLY CONFORM TO LOCAL, STATE AND FEDERAL SAFETY REGULATIONS.
4. DO NOT BACKFILL AGAINST CONCRETE ELEMENTS UNTIL PLACED CONCRETE HAS REACHED 75% OF ITS SPECIFIED 28-DAY COMPRESSIVE STRENGTH.
5. BACKFILL BOTH SIDES OF FOUNDATION WALLS IN EQUAL, ALTERNATE LIFTS IN ORDER TO AVOID IMPOSING UNBALANCED LATERAL PRESSURE ON THE WALLS.
6. ALLOW TESTING AGENCY TO INSPECT AND APPROVE ALL COMPACTED SUBGRADE AND FILL LAYERS PRIOR TO FURTHER BACKFILL AND/OR PLACEMENT OF CONCRETE. TESTING AND INSPECTION RESULTS SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER.
7. THE SUITABILITY AND STABILITY OF EXISTING SOILS AND FILL, THE DEPTHS AND LATERAL LIMITS OF UNSUITABLE MATERIAL TO BE REMOVED, AND ADEQUACY OF FOUNDATION BEARING GRADES SHALL BE DETERMINED BY THE PROJECT GEOTECHNICAL ENGINEER.
8. BACKFILL AND FILL MATERIALS SHALL BE COMPACTED TO 95% OF MAXIMUM DRY DENSITY ACCORDING TO THE MODIFIED PROCTOR TEST (ASTM D-1557). ALL EXISTING BACKFILL SHALL BE RECOMPACTED AS SUCH.
9. EXCAVATION AND BACKFILL OPERATIONS SHALL BE MAINTAINED IN A DRY CONDITION. SURFACE AND INFILTRATING WATER SHALL BE REMOVED BY SITE GRADING AND/OR BY PUMPING FROM SUMPS AS REQUIRED.

CONCRETE NOTES:

2. PROVIDE THE FOLLOWING MINIMUM CONCRETE CLEAR COVER FOR REINFORCING STEEL, UNLESS OTHERWISE NOTED.:
   a. CONCRETE PLACED AGAINST EARTH: 3.0 IN.
   b. FORMED SURFACES IN CONTACT WITH EARTH OR EXPOSED TO WEATHER
      #6 THROUGH #18 BARS: 2.0 IN.
      #5 BARS AND SMALLER: 1.5 IN.
   c. FORMED SURFACES NOT IN CONTACT WITH EARTH OR EXPOSED TO WEATHER
      #14 AND #18 BARS: 1.5 IN.
      #11 BARS AND SMALLER: 1.0 IN.
3. ALL CONCRETE WORK, CONSTRUCTION, AND REINFORCING DETAILS SHALL CONFORM TO THE "NORTH CAROLINA STATE BUILDING CODE, LATEST EDITION".
4. ALL REINFORCING STEEL SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH ACI 318.
5. ALL REINFORCING BARS SHALL CONFORM TO ASTM A615 GRADE 60.
6. ALL REINFORCING SHALL BE LAPPED OR EMBEDDED IN ACCORDANCE WITH ACI 318, UNLESS OTHERWISE NOTED.
7. PROVIDE CORNER BARS TO MATCH ALL HORIZONTAL REINFORCING AT CORNERS OR INTERSECTIONS.
8. CHAMFER EXTERIOR CORNERS AND EDGES OF PERMANENTLY EXPOSED CONCRETE.
9. PRIOR TO PLACEMENT OF CONCRETE, A FIELD REPRESENTATIVE SHALL BE INFORMED A MINIMUM OF 24 HOURS IN ADVANCE OF PLACEMENT, TO ALLOW INSPECTION OF REINFORCING STEEL, AND PREPARATION FOR TAKING CONCRETE SAMPLES. INDEPENDENT TESTS ARE REQUIRED FOR ALL CONCRETE PLACEMENTS.
10. INSTALLATION OF REINFORCEMENT SHALL BE COMPLETED AT LEAST 24 HOURS PRIOR TO THE SCHEDULED CONCRETE PLACEMENT.
13. VAPOR BARRIER: POLYETHYLENE SHEET, ASTM D 4397, NOT LESS THAN 15-MIL. LOCATED BELOW INTERIOR SLABS-ON-GRADE.
14. EPOXY ADHESIVE: HILTI HIT-HY 200 OR SIMPSON SET EPOXY.
15. GROUT: NON-METALLIC/NON-SHRINK STRUCTURAL GROUT. FIVE STAR GROUT OR APPROVED EQUAL.
16. SYNTHETIC MACRO-FIBER; FIBRILLATED POLYPROPYLENE MICRO-FIBERS ENGINEERED AND DESIGNED FOR USE IN CONCRETE, COMPLYING WITH ASTM C 1116/C 1116M, TYPE III.
17. PROTECT CONCRETE FROM PREMATURE DRYING IMMEDIATELY AFTER PLACEMENT. CURING OF CONCRETE SLABS MUST START WITHIN 2 HOURS AFTER FINISHING OPERATIONS ARE COMPLETE. SLABS-ON-GRADE SHALL BE WET CURED FOR 7 DAYS. CURING COMPOUNDS ARE PROHIBITED.
18. SLABS-ON-GRADE SHALL HAVE CONTROL JOINTS AS SHOWN ON PLANS. SAW CUT JOINTS SHALL BE MADE WITHIN 12 HOURS OF PLACING SLAB. AFTER CONCRETE IS CURED AND READY FOR PLACEMENT OF FLOOR FINISH, ALL SLABS INSIDE THE BUILDING SHALL HAVE CONTROL JOINTS FILLED WITH APPROVED JOINT FILLER.
19. CONCRETE SHALL BE CONTROLLED, PROPORTIONED, MIXED AND PLACED IN THE PRESENCE OF A REPRESENTATIVE OF AN APPROVED TESTING AGENCY.
20. CONDUIT OR PIPES SHALL BE PLACED UNDER SLABS-ON-GRADE.
21. ALUMINUM CONDUITS OR PIPES SHALL NOT BE PLACED IN CONCRETE.

MASONRY NOTES:

2. MASONRY CONSTRUCTION SHALL CONFORM TO THE LATEST EDITION OF THE "BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES" (ACI-530).
3. ALL CONCRETE BLOCK SHALL CONFORM TO ASTM-C90. PROVIDE NORMAL WEIGHT UNITS WITH MINIMUM AVERAGE NET-AREA COMPRESSIVE STRENGTH OF 2000 PSI.
4. MORTAR FOR UNIT MASONRY: COMPLY WITH ASTM C 270. PROVIDE THE FOLLOWING TYPES OF MORTAR FOR APPLICATIONS BELOW:
   a. FOR REINFORCED MASONRY, USE TYPE M.
   b. FOR MASONRY BELOW GRADE OR IN CONTACT WITH EARTH, USE TYPE M.
   c. FOR INTERIOR NONLOAD-BEARING PARTITIONS, USE TYPE N.
5. PLACE GROUT IN ALL REINFORCED CELLS. GROUT SHALL BE PLACED USING LOW-LIFT GROUTING NOT TO EXCEED 5' - 0".
6. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60. REINFORCING BARS MARKED "CONTINUOUS" SHALL BE LAPPED PER ACI 530. CONSTRUCT LAP SPLICES AND EMBEDMENT LENGTHS PER ACI 530. MAINTAIN A MINIMUM OF 1/2" CLEARANCE BETWEEN REINFORCING BARS AND MASONRY. PROVIDE #5 BARS UNLESS OTHERWISE NOTED.
7. JOINT REINFORCEMENT FACTORY FABRICATED FROM COLD-DRAWN STEEL WIRE, ASTM A 82, LADDER DESIGN, WITH 9 GAGE DEFORMED STEEL WIRE LONGITUDINAL RODS WELDED TO 9 GAGE STEEL WIRE CROSS TIES SPACED 16 INCHES ON CENTER MAXIMUM; WIDTH 1-1/2 TO 2 INCHES LESS THAN TOTAL WALL THICKNESS. FURNISH FACTORY FABRICATED CORNER AND TEE SECTIONS FOR CORNERS AND WALL INTERSECTIONS.
8. DESIGN AND PROVIDE TEMPORARY BRACING OF MASONRY WALLS DURING CONSTRUCTION. BRACING SHALL REMAIN IN PLACE UNTIL PERMANENT SUPPORTING ELEMENTS OF THE STRUCTURE HAVE BEEN CONSTRUCTED. BRACING SHALL FULLY CONFORM TO ALL OSHA REQUIREMENTS.
9. GALVANIZED ADJUSTABLE WIRE TIES SHALL BE FURNISHED AND INSTALLED AT 16 INCHES ON CENTER MAXIMUM, EACH WAY, FOR ALL CAVITY WALLS AND AS INDICATED. MASONRY TIES SHALL BE FURNISHED AND INSTALLED TO STEEL FRAMING TO SUPPORT ALL MASONRY CONSTRUCTION, TIES TO BE AT 16 INCHES ON CENTER MAXIMUM. SEE SPECIFICATIONS FOR MATERIAL REQUIREMENTS.
10. GROUT ALL CELLS OF MASONRY UNITS FOR THE FIRST TWO COURSES ABOVE ALL FOUNDATION WALLS AND SLABS.
11. PROVIDE CORNER BARS WHERE HORIZONTAL REINFORCING MEETS AT A CORNER OR INTERSECTION.
12. PROVIDE REINFORCING BARS AROUND ALL MASONRY OPENINGS. SEE TYPICAL MASONRY DETAILS.
13. ALL MASONRY COURSING SHOWN IN SECTION AND ELEVATION IS SCHEMATIC. MASONRY MAY NEED TO BE CUT AS REQUIRED.
14. CONDUITS, PIPES, AND SLEEVES IN MASONRY SHALL BE NO CLOSER THAN 3 DIAMETERS ON CENTER. ALUMINUM SHALL NOT BE USED.

WOOD FRAMING NOTES:

1. WOOD CONSTRUCTION SHALL CONFORM TO THE LATEST EDITION OF THE NATIONAL FOREST PRODUCTS ASSOCIATION'S (NFPA) NATIONAL DESIGN SPECIFICATIONS (NDS) AND CHAPTER 23 OF THE BUILDING CODE OF NYS, LATEST EDITION.
2. MINIMUM DESIGN VALUES SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED:
3. WOOD IN CONTACT WITH MASONRY, CONCRETE OR EARTH, OR WITHIN 1'-0" OF GRADE OR EXPOSED TO THE EXTERIOR SHALL BE PRESSURE PRESERVATIVE TREATED.
4. FRAMING ANCHORS AND MISCELLANEOUS METAL DEVICES FOR ALL FRAMING SHALL BE GALVANIZED STEEL OF AT LEAST 16 GAGE THICKNESS (G90 FOR INTERIOR APPLICATION, G185 OR STAINLESS STEEL FOR EXTERIOR). INSTALL IN STRICT ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. USE FASTENERS AND FASTENING METHODS RECOMMENDED BY THE MANUFACTURER. EXTERIOR EXPOSED ANCHORS AND ANCHORS IN CONTACT WITH PRESSURE TREATED WOOD TO BE STAINLESS OR GALVANIZED (G185).
5. BUILT-UP FRAMING MEMBERS SHALL BE FASTENED IN ACCORDANCE WITH NDS STANDARDS UNLESS OTHERWISE NOTED.
6. NOTCHES, COPES, AND HOLES IN WOOD MEMBERS ARE NOT PERMITTED UNLESS SPECIFICALLY DETAILED. NOTCHES, COPES, AND HOLES IN PRE-ENGINEERED MEMBERS SHALL BE IN ACCORDANCE AND APPROVED BY THE MANUFACTURER.
7. ROOF TRUSSES, INCLUDING DESIGN, FRAMING CONNECTORS, BRACING ERECTION AND QUALITY SHALL CONFORM TO THE SPECIFICATIONS AND RECOMMENDATIONS OF NFPA AND THE TRUSS PLATE INSTITUTE (TPI).
8. SHEATHING SHALL BE RATED AS FOLLOWS (CHECK THAT IT MEETS DESIGN LOADS)
   a. WALL: APA RATED 24" O.C. EXPOSURE I (7/16" MIN. THICKNESS)
   b. FLOOR: APA RATED 24/16, EXPOSURE I (3/4" MIN. THICKNESS)
   c. ROOF: APA RATED 48/24, EXPOSURE I (5/8" MIN. THICKNESS)
9. SHEATHING SHALL BE CONTINUOUS OVER TWO OR MORE SUPPORTS. FLOOR AND ROOF SHEATHING SHALL BE ORIENTED WITH THE STRENGTH AXIS PERPENDICULAR TO THE SUPPORTS. WALL SHEATHING CAN BE ORIENTED PERPENDICULAR OR PARALLEL
10. WALL SHEATHING SHALL HAVE 2X BLOCKING OR FRAMING MEMBERS BEHIND ALL PANEL EDGES.
11. UNLESS NOTED OTHERWISE, THE MINIMUM FASTENING FOR SHEATHING SHALL BE AS FOLLOWS:
   a. WALL: 8d COMMON NAILS @ 6" O.C. (EDGE) & 12" O.C. (FIELD)
   b. FLOOR: GLUED AND 10d COMMON NAILS @ 6" O.C. (PANEL EDGES) AND 12" O.C. (FIELD)
   c. ROOF: 10d COMMON NAILS@ 6" O.C. (PANEL EDGES) AND 12" O.C. (FIELD)
   d. GWB: #6 - 1 ¼" SCREWS AT 8" (EDGE) AND 12" (FIELD).
12. WOOD CONNECTORS: SIMPSON STRONG-TIE CONNECTORS USED AS BASIS OF DESIGN. USP STRUCTURAL CONNECTORS OF EQUAL STRENGTH ARE ACCEPTABLE.
13. BOLTS THROUGH WOOD MEMBERS SHALL BE ASTM A307.

GENERAL DEMOLITION NOTES:

1. DEMOLISH AND REMOVE EXISTING CONSTRUCTION ONLY TO THE EXTENT REQUIRED BY NEW CONSTRUCTION AND AS INDICATED. USE METHODS REQUIRED TO COMPLETE THE WORK WITHIN LIMITATIONS OF GOVERNING REGULATIONS AND AS FOLLOWS:
   a. PROCEED WITH SELECTIVE DEMOLITION SYSTEMATICALLY, FROM HIGHER TO LOWER LEVEL. COMPLETE SELECTIVE DEMOLITION OPERATIONS ABOVE EACH FLOOR OR TIER BEFORE DISTURBING SUPPORTING MEMBERS ON THE NEXT LOWER LEVEL.
   b. NEATLY CUT OPENINGS AND HOLES PLUMB, SQUARE, AND TRUE TO DIMENSIONS REQUIRED. USE CUTTING METHODS LEAST LIKELY TO DAMAGE CONSTRUCTION TO REMAIN OR ADJOINING CONSTRUCTION. USE HAND TOOLS OR SMALL POWER TOOLS DESIGNED FOR SAWING OR GRINDING, NOT HAMMERING AND CHOPPING, TO MINIMIZE DISTURBANCE OF ADJACENT SURFACES. TEMPORARILY COVER OPENINGS TO MAINTAIN A WATERTIGHT CONDITION UNTIL PERMANENT CONSTRUCTION IS COMPLETE.
   c. CUT OR DRILL FROM THE EXPOSED OR FINISHED SIDE INTO CONCEALED SURFACES. AVOID MARRING EXISTING FINISHED SURFACES.
   d. NO FLAME CUTTING.
   e. REMOVE DECAYED, VERMIN-INFESTED, OR OTHERWISE DANGEROUS OR UNSUITABLE NON-HAZARDOUS MATERIALS. PROMPTLY DISPOSE OF OFF-SITE.
   f. ASBESTOS CONTAINING MATERIAL (ACM) / HAZARDOUS MATERIALS:
      .... i. NO ACM SURVEY HAS BEEN PERFORMED FOR THIS PROJECT
   g. REMOVE STRUCTURAL FRAMING MEMBERS AND LOWER TO GROUND BY METHOD SUITABLE TO AVOID FREE FALL AND TO PREVENT GROUND IMPACT OR DUST GENERATION.
   h. LOCATE SELECTIVE DEMOLITION EQUIPMENT AND REMOVE DEBRIS AND MATERIALS SO AS NOT TO IMPOSE EXCESSIVE LOADS ON SUPPORTING WALLS, FLOORS, OR FRAMING.
   i. DISPOSE OF DEBRIS OFF-SITE PROMPTLY AT CONTRACTOR'S EXPENSE AND IN ACCORDANCE WITH ALL APPLICABLE REGULATIONS AND CODES.
2. BUILDING ELEMENTS TO REMAIN: DO NOT DEMOLISH BUILDING ELEMENTS BEYOND LIMITS INDICATED.
3. EXISTING ITEMS TO REMAIN: PROTECT CONSTRUCTION INDICATED TO REMAIN AGAINST DAMAGE AND SOILING DURING SELECTIVE DEMOLITION. WHEN PERMITTED BY ARCHITECT, ITEMS MAY BE REMOVED TO A SUITABLE, PROTECTED STORAGE LOCATION DURING SELECTIVE DEMOLITION[AND CLEANED] AND REINSTALLED IN THEIR ORIGINAL LOCATIONS AFTER SELECTIVE DEMOLITION OPERATIONS ARE COMPLETE. COMPLY WITH INSTALLATION REQUIREMENTS FOR NEW MATERIALS AND EQUIPMENT. PROVIDE CONNECTIONS, SUPPORTS, AND MISCELLANEOUS MATERIALS NECESSARY TO MAKE ITEM FUNCTIONAL FOR USE INDICATED.
4. SELECTIVE DEMOLITION PROCEDURES FOR SPECIFIC MATERIALS:
   a. REINFORCED CONCRETE: DEMOLISH IN SMALL SECTIONS. SAW CUT CONCRETE TO A DEPTH OF AT LEAST 3/4 INCH AT JUNCTURES WITH CONSTRUCTION TO REMAIN. DISLODGE CONCRETE FROM REINFORCEMENT AT PERIMETER OF AREAS BEING DEMOLISHED, CUT REINFORCEMENT, AND THEN REMOVE REMAINDER OF CONCRETE INDICATED FOR SELECTIVE DEMOLITION USING MAXIMUM 15-LB CHIPPING HAMMER. NEATLY TRIM OPENINGS TO DIMENSIONS INDICATED.
   b. CONCRETE SLABS-ON-GRADE: SAW-CUT PERIMETER OF AREA TO BE DEMOLISHED, THEN BREAK UP AND REMOVE.
   c. WOOD: SAWCUT CLEANLY, LEVEL, PLUMB, AND SQUARE TO DIMENSIONS REQUIRED FOR RECONSTRUCTION.
   DISPOSAL OF DEMOLISHED MATERIALS
   d. GENERAL: EXCEPT FOR ITEMS OR MATERIALS INDICATED TO BE RECYCLED, REUSED, SALVAGED, REINSTALLED, OR OTHERWISE INDICATED TO REMAIN OWNER'S PROPERTY, REMOVE DEMOLISHED MATERIALS FROM PROJECT SITE AND LEGALLY DISPOSE OF THEM IN AN EPA-APPROVED LANDFILL.
5. DO NOT ALLOW DEMOLISHED MATERIALS TO ACCUMULATE ON-SITE.
6. REMOVE AND TRANSPORT DEBRIS IN A MANNER THAT WILL PREVENT SPILLAGE ON ADJACENT SURFACES AND AREAS.
7. REMOVE DEBRIS FROM ELEVATED PORTIONS OF BUILDING BY CHUTE, HOIST, OR OTHER DEVICE THAT WILL CONVEY DEBRIS TO GRADE LEVEL IN A CONTROLLED DESCENT.
8. BURNING: DO NOT BURN DEMOLISHED MATERIALS.

8a. COMPLY WITH REQUIREMENTS SPECIFIED IN DIVISION 01 SECTION "CONSTRUCTION WASTE MANAGEMENT AND DISPOSAL."

9. DISPOSAL:
   TRANSPORT DEMOLISHED MATERIALS OFF OWNER'S PROPERTY AND LEGALLY DISPOSE OF THEM.
10. CLEANING:
    CLEAN ADJACENT STRUCTURES AND IMPROVEMENTS OF DUST, DIRT, AND DEBRIS CAUSED BY ALL DEMOLITION OPERATIONS. RETURN ADJACENT AREAS TO CONDITION EXISTING BEFORE DEMOLITION OPERATIONS BEGAN.

STRUCTURAL DESIGN TABLE - IBC 2015
(IN ACCORDANCE WITH APPLICABLE BUILDING CODE)

BUILDING DATA:			
LOCATION		195 SPRINGBROOK AVE, CLAYTON, NC 27520	
BUILDING OCCUPANCY RISK CATEGORY		III	IBC 2015 TABLE 1604.5
APPLICABLE BUILDING CODE		NORTH CAROLINA STATE BUILDING CODE, LATEST EDITION	
FLOOR LIVE LOAD:			
LOBBY	LL1	100 PSF	IBC 2015 TABLE 1607.1
CORRIDORS (FIRST FLOOR)	LL2	100 PSF	
OFFICES	LL3	40 PSF	
MECHANICAL	LL4	150 PSF	
ROOF LIVE LOAD:			
ROOF	LLr	20 PSF	IBC 2015 TABLE 1607.1
SNOW LOAD:			
SNOW LOAD IMPORTANCE FACTOR	Is	1.0	ASCE 7-10 TABLE 1.5-2
GROUND SNOW LOAD	Pg	15 PSF	IBC 2015 FIGURE 1608.2
SNOW EXPOSURE FACTOR	Ce	1.0	ASCE 7-10 TABLE 7-2
THERMAL FACTOR	Ct	1.0	ASCE 7-10 TABLE 7-3
DRIFTING SNOW		AS REQ. PER ASCE 7-10	ASCE 7-10 SECTION 7.7
WIND LOAD (MAIN WIND-FORCE RESISTING SYSTEM):			
ANALYSIS PROCEDURE		DIRECTIONAL PROCEDURE	ASCE 7-10 CHAPTER 27
ULTIMATE DESIGN WIND SPEED (3-SECOND GUST)	Vult	122 mph	ASCE 7-10 SECTION 26.5
WIND DIRECTIONALITY FACTOR	Kd	0.85	ASCE 7-10 SECTION 26.6
EXPOSURE CATEGORY		C	ASCE 7-10 SECTION 26.7
TOPOGRAPHIC FACTOR	Kzt	1.00	ASCE 7-10 SECTION 26.8
GUST-EFFECT FACTOR	G	0.85	ASCE 7-10 SECTION 26.9
ENCLOSURE CLASSIFICATION		ENCLOSED	ASCE 7-10 SECTION 26.10
INTERNAL PRESSURE COEFFICIENT	GCpi	+0.18/-0.18	ASCE 7-10 SECTION 26.11
VELOCITY PRESSURE	q	16 PSF	ASCE 7-10 SECTION 27.3.2
MINIMUM WALL WIND PRESSURE	Pmin	16 PSF	ASCE 7-10 SECTION 27.4.7
MINIMUM ROOF WIND PRESSURE	Pmin	8 PSF	ASCE 7-10 SECTION 27.4.7
NOTES		WIND LOADS ARE CALCULATED FROM THESE PARAMETERS FOR EACH SURFACE OF THE MAIN WIND-FORCE RESISTING SYSTEM.	
WIND LOAD (COMPONENTS & CLADDING):			
BASIC WIND SPEED (3-SECOND GUST)	V	122 mph	ASCE 7-10 SECTION 26.5
EXPOSURE CATEGORY		C	ASCE 7-10 SECTION 26.7
TOPOGRAPHIC FACTOR	Kzt	1.00	ASCE 7-10 SECTION 26.8
ENCLOSURE CLASSIFICATION		ENCLOSED	ASCE 7-10 SECTION 26.10
EFFECTIVE WIND AREA	Aeff	10 SQFT	ASCE 7-10 FIGURE 30.5-1
MINIMUM DESIGN WIND PRESSURE	Pmin	+/- 16 PSF	ASCE 7-10 SECTION 30.2.2
NOTES	1.	EFFECTIVE AREA ABOVE USED AS BASIS FOR "WORST CASE" PRESSURE CALCULATIONS. THE EFFECTIVE AREA FOR EACH INDIVIDUAL COMPONENT SHALL BE CALCULATED AND PRESSURE VALUES ADJUSTED ACCORDINGLY.	
	2.	INCREASED WIND PRESSURES AT EDGES, OVERHANGS, AND OTHER SURFACES ARE AS DERIVED IN ASCE 7-10 "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES".	
EARTHQUAKE LOAD:			
SEISMIC - FORCE RESISTING SYSTEM		BEARING WALL SYSTEM W/ LIGHT FRAMED (WOOD) WALLS SHEATHED WITH SHEAR PANELS	ASCE 7-10 TABLE 12.2-1
SOIL SITE CLASSIFICATION		D	ASCE 7-10 SECTION 20.3
SPECTRAL RESPONSE ACCELERATION AT 0.2 SEC	Ss	0.158g	ASCE 7-10 FIGURE 22-1
SPECTRAL RESPONSE ACCELERATION AT 1.0 SEC	S1	0.077g	ASCE 7-10 SECTION 11.4.1
SEISMIC IMPORTANCE FACTOR	Ie	1.25	ASCE 7-10 TABLE 1.5-2
DESIGN SPECTRAL RESPONSE COEFFICIENT	SDS	0.124g	ASCE 7-10 SECTION 11.4.4
DESIGN SPECTRAL RESPONSE COEFFICIENT	SD1	0.000g	ASCE 7-10 SECTION 11.4.4
SEISMIC DESIGN CATEGORY		B	ASCE 7-10 TABLE 11.6-(1&2)
ANALYSIS PROCEDURE		EQUIV. LATERAL FORCE	ASCE 7-10 SECTION 12.8
SEISMIC RESPONSE COEFFICIENT	Cs	0.0238	ASCE 7-10 SECTION 12.8.1.1
SEISMIC BASE SHEAR - AREA B	V	5.5K	ASCE 7-10 SECTION 12.8.1
RESPONSE MODIFICATION FACTOR	R	6.5	ASCE 7-10 TABLE 12.2-1
SEISMIC BASE SHEAR - AREA A	V	4K	ASCE 7-10 SECTION 12.8.1

ROOF DECK SCHEDULE

MARK	TYPE	FASTENER SIZE AND SPACING		COMMENTS
		AT BOUNDARIES AND PANEL EDGES	INTERMEDIATE	
RD-1	5/8" EXTERIOR GRADE PLYWOOD STRUCTURAL 1 RATED SHEATHING	10D NAILS @ 6" O.C.	10D NAILS @ 12" O.C.	PROVIDE BLOCKING AT ALL PANEL EDGES

SLAB-ON-GRADE SCHEDULE

MARK	TYPE	SLAB THICKNESS	SLAB REINFORCEMENT	COMMENTS
S.O.G. 1	SLAB-ON-GRADE	4"	FIBER REINFORCEMENT - SEE GENERAL NOTES	SEE SPECIFICATIONS & ARCH FOR FINISH

MASONRY WALL SCHEDULE

MARK	THICKNESS	WALL REINFORCEMENT			COMMENTS
		HORIZONTAL	VERTICAL	BOND BEAM REINF. & SPACING	
M8	7 5/8"	JOINT REINFORCEMENT - SEE SPECS	#5 BARS @ 48" O.C.	(1) #5 BAR CONT.	

WOOD WALL SCHEDULE

MARK	THICKNESS	SHEATHING GRADE	FRAMING SIZE & SPACING			BASE OF WALL CONNECTIONS		COMMENTS
			STUDS	CHORDS	TOP & BOTTOM PLATE	BOTTOM PLATE ANCHORS	CHORD HOLDOWN ANCHOR	
W4	3 1/2"	TYPE X	2X4 STUDS @ 16" O.C.	(2) 2X4	(2) 2X4 TOP PLATE 2X4 PT SILL PLATE	1/2" DIA. ANCHORS @ 4' - 0" O.C.	SIMPSON STRONG-TIE HDU8-SDS2.5	HOLDOWN ANCHORS ONLY REQUIRED AT SHEARWALLS - SEE PLAN
W6	5 1/2"	VARIES	2X6 STUDS @ 16" O.C.	(2) 2X6	(2) 2X6 TOP PLATE 2X6 PT SILL PLATE	1/2" DIA. ANCHORS @ 4' - 0" O.C.	SIMPSON STRONG-TIE HDU8-SDS2.5	HOLDOWN ANCHORS ONLY REQUIRED AT SHEARWALLS - SEE PLAN

WALL FOOTING SCHEDULE

MARK	WIDTH	THICKNESS	FOOTING REINFORCEMENT		COMMENTS
			LONGITUDINAL	TRANSVERSE	
WF2	2' - 0"	1' - 0"	(3) #4 BARS CONT.	#4 BARS @ 12" O.C.	

STUDIO WALES
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NORTH CAROLINA PROFESSIONAL SEAL 040156 ENGINEER DANIEL R. HILL
02/18/2025

LABELLA ASSOCIATES P.C. REGISTERED ARCHITECTURAL CORPORATION CERT. NO. 52904 NORTH CAROLINA CHARLOTTE, N. C.

SPRINGBROOK NURSING AND REHABILITATION CENTER ADDITION

195 SPRINGBROOK AVE, CLAYTON, NC

CONSTRUCTION DOCUMENTS
FOR CONSTRUCTION

Revisions

No.	Description	Date

date: 02/06/2025
commission: NH-3138

sheet title:
GENERAL NOTES

sheet number :
S001

STATEMENT OF SPECIAL INSPECTIONS

LOCATION	195 SPRINGBROOK AVE. CLAYTON, NC 27520
OWNER	HILL CO LTD.
DESIGN PROFESSIONAL IN CHARGE	DANIEL R. HILL, P.E.

This statement of Special Inspections is submitted as a condition for permit issuance in accordance with the Special Inspection and Structural Testing requirements of the applicable building code. It includes a schedule of Special Inspection services applicable to this project as well as the name of the Special Inspection coordinator and the identity of other approved agencies to be retained for conducting these inspections and tests. This Statement of Special Inspections encompasses the following disciplines: STRUCTURAL. The Special Inspection Coordinator shall keep records of all inspections and shall furnish inspection reports to the Building Official and the Registered Design Professional in Responsible Charge (RDP). Discovered discrepancies shall be brought to the immediate attention of the contractor for correction. If such discrepancies are not corrected, the discrepancies shall be brought to the attention of the Building Official and the RDP. The Special Inspection program does not relieve the contractor of his or her responsibility for quality assurance.

Interim reports shall be submitted to the Building Official and the RDP.

A Final Report of Special Inspections documenting completion of all required Special Inspections, testing, and correction of any discrepancies noted in the inspections shall be submitted by the special Inspection Coordinator prior to issuance of a Certificate of Use and Occupancy.

Job site safety and means and methods of construction are solely the responsibility of the contractor.

Interim reports shall be submitted monthly.

In accordance with the applicable building code, the Observations and Inspections listed in the Schedule of Special Inspections are required.

SCHEDULE OF INSPECTION AND TESTING AGENCIES

SPECIAL INSPECTION AGENCIES	FIRM	ADDRESS	TELEPHONE No.
Special Inspection Coordinator	TBD	TBD	(###) ###-####
Inspector	TBD	TBD	(###) ###-####

Note: The inspectors and testing agencies shall be engaged by the Owner or the Owner's Agent in accordance with the applicable building code, and not by the Contractor or Subcontractor whose work is to be inspected or tested. An approved agency shall be objective, competent and independent from the contractor responsible for the work being inspected. The agency shall also disclose to the building official and the registered design professional in responsible charge possible conflicts of interest so that objectivity can be confirmed.

STATEMENT OF CONTRACTORS RESPONSIBILITY

In accordance with the applicable building code, each contractor responsible for the construction of a main wind or seismic force-resisting system, designated seismic system or a wind or seismic force-resisting component listed in the statement of special inspections above shall submit a written statement of responsibility to the building official and the owner or the owner's authorized agent prior to the commencement of work on the system or component. The contractor's statement of responsibility shall contain acknowledgement of awareness of the special requirements contained in the statement of special inspections.

QUALIFICATIONS OF INSPECTORS AND TESTING TECHNICIANS

The qualifications of all personnel performing Special Inspection and testing activities are subject to the approval of the Building Official. The credentials of all Inspectors and testing technicians shall be provided.

Key for Minimum Qualifications of Inspection Agents:

When the Registered Design Professional in Responsible Charge deems it appropriate that the individual performing a stipulated test of inspection have a specific certification or license as indicated below, such designation shall appear below the Agency Number on the Schedule.

PE/SE	Structural Engineer - a licensed PE specializing in the design of building structures
PE/GE	Geotechnical Engineer - a licensed PE specializing in soil mechanics and foundations
EIT	Engineer - In - Training - a graduate engineer who as passed the Fundamentals of Engineering examination
AMERICAN CONCRETE INSTITUTE (ACI) CERTIFICATION	
ACI-CFTT	Concrete Field Testing Technician - Grade 1
ACI-CCSI	Concrete Construction Special Inspector
ACI-LTT	Laboratory Testing Technician - Grade 1&2
ACI-STT	Strength Testing Technician
AMERICAN WELDING SOCIETY (AWS) CERTIFICATION	
AWS-CWI	Certified Welding Inspector
AWS/AISC-SSI	Certified Structural Steel Inspector
INTERNATIONAL CODE COUNCIL (ICC) CERTIFICATION	
ICC-SMSI	Structural Masonry Special Inspector
ICC-SWSI	Structural Steel and Welding Special Inspector
ICC-SFSI	Spray-Applied Fireproofing Special Inspector
ICC-PCSI	Prestressed Concrete Special Inspector
ICC-RCSI	Reinforced Concrete Special Inspector
NATIONAL INSTITUTE FOR CERTIFICATION IN ENGINEERING TECHNOLOGIES (NICET)	
NICET-CT	Concrete Technician - Levels I, II, III, & IV
NICET-ST	Soil Technicians - Levels I, II, III & IV
NICET-GET	Geotechnical Engineering Technician - Levels I, II, III & IV

REFERENCES

CODE/STANDARD	TITLE
ACI 301	Standard Specifications for Structural Concrete.
ACI 318	Building Code Requirements for Structural Concrete
ACI 530.1/ASCE 6/TMS 602	Specifications for Masonry Structures
AISC 360	Specifications for Structural Steel Buildings
ASTM A6	Specifications for General Requirements for Rolled Steel Plates, Shapes, Sheet Piling, and Bars for Structural Use.
ASTM A568	Specifications for Steel Sheet, Carbon and High Strength, Low-Alloy, Hot-Rolled and Cold Rolled.
ASTM C31	Practice for Making and Curing Concrete Test Specimens in the Field
ASTM C94	Specifications for Ready-Mixed Concrete
ASTM C109	Test Methods for Compressive Strength of Hydraulic Cement Mortars (Using 2 in. or 50 mm Cube Specimens)
ASTM C138	Test Method for Unit Weight, Yield and Air Content (Gravimetric) of Concrete
ASTM C143	Test Method for Slump of Hydraulic Cement Concrete.
ASTM C172	Practice for Sampling Freshly Mixed Concrete
ASTM C173	Test Method for Air Content of Freshly Mixed Concrete by the Volumetric Method
ASTM C231	Test Method for Air Content of Freshly Mixed Concrete by the Pressure Method
ASTM C567	Test Method for Unit Weight of Structural Lightweight Concrete
ASTM C1090	Test Method for Temperature of Freshly Mixed Portland Cement Concrete
ASTM C1064	Test Method for Measuring Changes in Height of Cylindrical Specimens from Hydraulic Cement Grout
ASTM C1314	Test Method for Constructing and Testing Masonry Prisms Used to Determine Compliance with Specified Compressive Strength of Masonry
ASTM E605	Standard Test Methods for Thickness and Density of Sprayed Fire-Resistive Material Applied to Structural Members
ASTM E736	Standard Test Method for Cohesion/Adhesion of Sprayed Fire-Resistive Materials Applied to Structural Members
ASTM E2174	Standard Practice for On-Site Inspection of Installed Firestops
ASTM E2393	Standard Practice for On-Site Inspection of Installed Fire Resistive Joint Systems and Perimeter Fire Barriers
AWCI 12-B	Standard Practice for the Testing and Inspection of Field Applied Thin Film Intumescent Fire-Resistive Materials
AWS D1.1	Structural Welding Code - Steel.
APPLICABLE BUILDING CODE	SEE STRUCTURAL DESIGN CRITERIA CHART AND GENERAL NOTES.
RCSC	Specification for Structural Joints Using High Strength Bolts.

SCHEDULE OF STRUCTURAL SPECIAL INSPECTIONS

THE FOLLOWING TABLES COMPRISES THE STRUCTURAL SPECIAL INSPECTION REQUIREMENTS FOR THIS PROJECT IN ACCORDANCE WITH CHAPTER 17 OF THE 2015 INTERNATIONAL BUILDING CODE. REFER TO THE PROJECT SPECIFICATIONS FOR REQUIRED QUALIFICATIONS OF ALL PERSONNEL PERFORMING SPECIAL INSPECTION ACTIVITIES AND ADDITIONAL TESTING INFORMATION.

EARTHWORK - REQUIREMENTS FOR SPECIAL INSPECTION & TESTING

AREAS OF INSPECTION & TESTING	FREQUENCY OF INSPECTION OR TESTING	REFERENCE STANDARD	IBC REFERENCE
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	PERIODIC	-	1705.6
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	PERIODIC		
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS	PERIODIC		
4. VERIFY USE OF PROPER MATERIALS, DENSITIES, AND LIFT THICKNESS DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	CONTINUOUS		
5. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	PERIODIC		

CAST-IN-PLACE CONCRETE - REQUIREMENTS FOR SPECIAL INSPECTION & TESTING

AREAS OF INSPECTION & TESTING	FREQUENCY OF INSPECTION OR TESTING	REFERENCE STANDARD	IBC REFERENCE
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	PERIODIC	ACI 318 CH. 20, 25.2, 25.3, 26.6.1 - 26.6.3	1908.4
2. REINFORCING BAR WELDING: A. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706; B. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16" C. INSPECT ALL OTHER WELDS.	 PERIODIC PERIODIC CONTINUOUS	 AWS D1.4 ACI 318: 26.6.4	 -
3. INSPECT ANCHORS CAST IN CONCRETE	PERIODIC	ACI 318: 17.8.2	-
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS. A. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS. B. MECHANICAL ANCHORS AND ADHESIVE ANCHORS.	 CONTINUOUS PERIODIC	 ACI 318: 17.8.2.4 ACI 318: 17.8.2	 -
5. VERIFY USE OF REQUIRED DESIGN MIX.	PERIODIC	ACI 318: CH. 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	CONTINUOUS	ASTM C172 ASTM C31 ACI 318: 26.4, 26.12	1908.10
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	CONTINUOUS	ACI 318: 26.5	1908.6, 1908.7, 1908.8
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	PERIODIC	ACI 318: 26.5.3 - 26.5.5	1908.9
9. INSPECT PRESTRESSED CONCRETE FOR: A. APPLICATION OF PRESTRESSING FORCES B. GROUTING OF BONDED PRESTRESSING TENDONS.	 CONTINUOUS CONTINUOUS	ACI 318: 26.10	-
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.	PERIODIC	ACI 318: CH. 26.8	-
11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	PERIODIC	ACI 318: 26.11.2	-
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	PERIODIC	ACI 318: 26.11.2 (b)	-

MASONRY CONSTRUCTION - REQUIREMENTS FOR LEVEL B SPECIAL INSPECTION & TESTING

AREAS OF INSPECTION & TESTING	FREQUENCY OF INSPECTION OR TESTING	REFERENCE STANDARD	IBC REFERENCE
1. VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS	PERIODIC	-	1705.4
2. AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ITEMS ARE IN COMPLIANCE: A. PROPORTIONS OF SITE-PREPARED MORTAR. B. CONSTRUCTION OF MORTAR JOINTS. C. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES. D. LOCATION OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS, AND ANCHORAGES. E. PRESTRESSING TECHNIQUE. F. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY.	 PERIODIC PERIODIC PERIODIC PERIODIC PERIODIC PERIODIC	-	
3. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE: A. GROUT SPACE B. GRADE, TYPE, SIZE OF REINFORCEMENT, ANCHOR BOLTS, PRESTRESSING TENDONS AND ANCHORAGES. C. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES. D. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS. E. CONSTRUCTION OF MORTAR JOINTS.	 PERIODIC PERIODIC PERIODIC PERIODIC PERIODIC	 SEC. 6.1 SEC. 6.1, 6.2.1, 6.2.6, 6.2.7	
4. VERIFY DURING CONSTRUCTION: A. SIZE AND LOCATION OF STRUCTURAL ELEMENTS. B. TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGES OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION. C. WELDING OF REINFORCEMENT. D. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLE WEATHER (TEMPERATURES BELOW 40) OR HOT WEATHER (TEMPERATURES ABOVE 90). E. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE. F. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE G. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS.	 PERIODIC PERIODIC CONTINUOUS PERIODIC CONTINUOUS CONTINUOUS PERIODIC	 SEC. 1.2.1(E), 6.1.4.3, 8.2.1 SEC. 8.1.6.7.2, 9.3.3.4(C), 11.3.3.4(B)	
5. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS.	PERIODIC		

WOOD CONSTRUCTION - REQUIREMENTS FOR SPECIAL INSPECTION & TESTING

AREAS OF INSPECTION & TESTING	FREQUENCY OF INSPECTION OR TESTING	REFERENCE STANDARD	IBC REFERENCE
1. FIELD GLUING OPERATIONS OF ELEMENTS OF THE SEISMIC FORCE-RESISTING SYSTEM.	REQUIRED FOR SEISMIC CATEGORY C, D, E OR F.	NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION	1705.12.2
2. NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF ELEMENTS OF THE SEISMIC FORCE-RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES, SHEAR PANELS AND HOLD-DOWNS.	PERIODIC		

SCHEDULE OF SPECIAL INSPECTIONS

SPECIAL INSPECTION AS REQUIRED BY THE NORTH CAROLINA STATE BUILDING CODE LATEST EDITION:

PERIODIC SPECIAL INSPECTIONS:

1. STEEL - SEE SPECIFICATION 05120
2. STEEL JOISTS AND JOIST GIRDERS - SEE SPECIFICATION 05210
3. STEEL DECK - SEE SPECIFICATION 05300
4. CONCRETE - SEE SPECIFICATION 03310 AND 03312
5. MASONRY - SEE SPECIFICATION 04200
6. PRECAST - SEE SPECIFICATION 03410
7. SEISMIC - INSPECTIONS DURING THE ERECTION AND FASTENING OF EXTERIOR CLADDING, INTERIOR AND EXTERIOR NON-LOAD BEARING WALLS, AND VENEER.

CONTINUOUS SPECIAL INSPECTIONS:

1. CONCRETE - SEE SPECIFICATION 03310 AND 03312
2. MASONRY - SEE SPECIFICATION 04200
3. POST-INSTALLED ANCHORS - SEE SPECIFICATION 05090

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NORTH CAROLINA PROFESSIONAL SEAL 040156 ENGINEER DANIEL R. HILL
02/18/2025

LABELLA ASSOCIATES P.C. REGISTERED ARCHITECTURE CORPORATION NORTH CAROLINA CHARLOTTE, N.C. CERT. NO. 52904

SPRINGBROOK NURSING AND REHABILITATION CENTER ADDITION

195 SPRINGBROOK AVE, CLAYTON, NC

CONSTRUCTION DOCUMENTS

FOR CONSTRUCTION

Revisions

No.	Description	Date

date: 02/06/2025

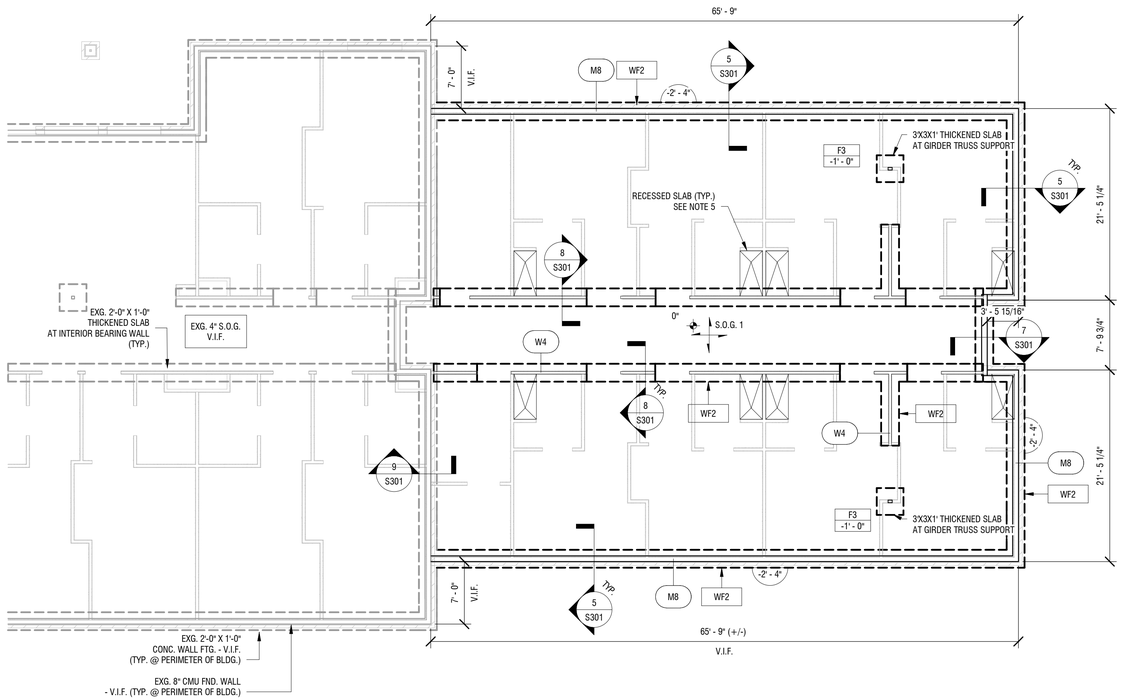
commission: NH-3138

sheet title:
SPECIAL INSPECTIONS

sheet number :
S004

FOUNDATION LEGEND

1.	P# | F# / #' - #" | -#' - #"	P# - INDICATES PIER TYPE (SEE PIER SCHEDULE) F# - INDICATES COLUMN FOOTING TYPE (SEE FOOTING SCHEDULE) [-#' - #"] - BELOW COLUMN FOOTING TYPE INDICATES BOTTOM OF FOOTING ELEVATION WITH RESPECT TO DATUM ELEVATION = 0' - 0". [-#' - #"] - BELOW PIER TYPE INDICATES TOP OF PIER ELEVATION WITH RESPECT TO DATUM ELEVATION = 0' - 0".
2.	W#	## - INDICATES WALL TYPE (SEE FOUNDATION WALL &/OR WALL SCHEDULE)
3.	WF# | #'-#"	WF# - INDICATES WALL FOOTING TYPE (SEE WALL FOOTING SCHEDULE) #'-#" - BOTTOM OF FOOTING ELEV. FOR WALL FOOTING W/ RESPECT TO DATUM ELEVATION = 0' - 0".
4.	#'-#"	#'-#" - BOTTOM OF FOOTING ELEV. FOR WALL FOOTING W/ RESPECT TO DATUM ELEVATION = 0' - 0".
5.	[##'-##"]	TOP OF WALL ELEVATION



1 / S100 FOUNDATION PLAN - AREA A

1/8" = 1'-0"

FOUNDATION PLAN NOTES:

1. BOTTOM OF FOOTING ELEVATIONS ARE REFERENCED FROM FINISHED FLOOR ELEVATION (DATUM ELEV. 0' - 0") AND ARE NOTED ON PLAN.
2. SECTIONS INDICATED ON PLAN ARE TYPICAL FOR SIMILAR CONDITIONS.
3. CENTER ISOLATED FOOTINGS UNDER COLUMNS AND/OR AT COLUMN LINE INTERSECTIONS. U.O.N..
4. DO NOT SCALE DRAWINGS. SEE ARCHITECTURAL FLOOR PLANS FOR DIMENSIONS NOT INDICATED ON STRUCTURAL DRAWINGS.
5. SEE DETAIL 1/S301 FOR RECESSED SLAB DETAIL.
6. SEE DETAIL 2/S301 FOR CONTROL JOINT SPACING.
7. PROVIDE (4) 2X6 AT EXTERIOR WALLS, (5) 2X4 AT INTERIOR BEARING WALLS (MIN.) AT ALL GIRDER TRUSS BEARING POINTS WITH SIMPSON HTT4 AT STUD BASE.
8. ALL EXTERIOR STUDS SHALL BE 2X6 SPF NO. 2 STUDS AT 16" O.C. AND ALL INTERIOR STUDS SHALL BE 2X4 SPF NO. 2 STUDS AT 16" O.C.

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02/18/2025

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CERT. NO. 52904

SPRINGBROOK NURSING AND REHABILITATION CENTER ADDITION

195 SPRINGBROOK AVE, CLAYTON, NC

CONSTRUCTION DOCUMENTS

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Revisions

No.	Description	Date

date: 02/06/2025
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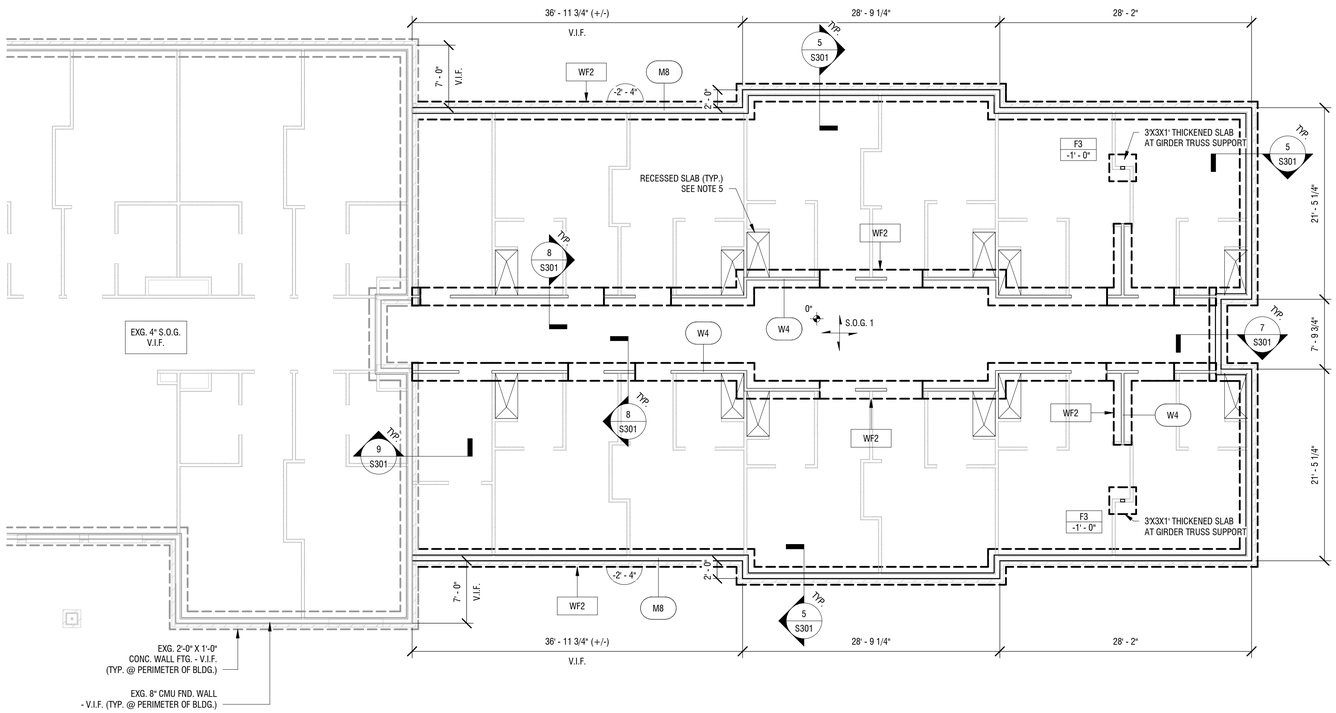
sheet title:
FOUNDATION PLAN - AREA A

sheet number :

S100

FOUNDATION LEGEND

1.	P# / #'-#" | F# / -#"-#"	P# - INDICATES PIER TYPE (SEE PIER SCHEDULE) F# - INDICATES COLUMN FOOTING TYPE (SEE FOOTING SCHEDULE) [-#"-#"] - BELOW COLUMN FOOTING TYPE INDICATES BOTTOM OF FOOTING ELEVATION WITH RESPECT TO DATUM ELEVATION = 0' - 0". [-#"-#"] - BELOW PIER TYPE INDICATES TOP OF PIER ELEVATION WITH RESPECT TO DATUM ELEVATION = 0' - 0".
2.	W#	## - INDICATES WALL TYPE (SEE FOUNDATION WALL &/OR WALL SCHEDULE)
3.	WF# | #'-#"	WF# - INDICATES WALL FOOTING TYPE (SEE WALL FOOTING SCHEDULE) #'-#" - BOTTOM OF FOOTING ELEV. FOR WALL FOOTING W/ RESPECT TO DATUM ELEVATION = 0' - 0".
4.	#'-#"	#'-#" - BOTTOM OF FOOTING ELEV. FOR WALL FOOTING W/ RESPECT TO DATUM ELEVATION = 0' - 0".
5.	[##'-##"]	TOP OF WALL ELEVATION



1 / S101 FOUNDATION PLAN - AREA B

1/8" = 1'-0"

FOUNDATION PLAN NOTES:

1. BOTTOM OF FOOTING ELEVATIONS ARE REFERENCED FROM FINISHED FLOOR ELEVATION (DATUM ELEV. 0' - 0") AND ARE NOTED ON PLAN.
2. SECTIONS INDICATED ON PLAN ARE TYPICAL FOR SIMILAR CONDITIONS.
3. CENTER ISOLATED FOOTINGS UNDER COLUMNS AND/OR AT COLUMN LINE INTERSECTIONS. U.O.N..
4. DO NOT SCALE DRAWINGS. SEE ARCHITECTURAL FLOOR PLANS FOR DIMENSIONS NOT INDICATED ON STRUCTURAL DRAWINGS.
5. SEE DETAIL 1/S301 FOR RECESSED SLAB DETAIL.
6. SEE DETAIL 2/S301 FOR CONTROL JOINT SPACING.
7. PROVIDE (4) 2X6 AT EXTERIOR WALLS, (5) 2X4 AT INTERIOR BEARING WALLS (MIN.) AT ALL GIRDER TRUSS BEARING POINTS WITH SIMPSON HTT4 AT STUD BASE.
8. ALL EXTERIOR STUDS SHALL BE 2X6 SPF NO. 2 STUDS AT 16" O.C. AND ALL INTERIOR STUDS SHALL BE 2X4 SPF NO. 2 STUDS AT 16" O.C.

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CONSTRUCTION DOCUMENTS

FOR CONSTRUCTION

Revisions

No.	Description	Date

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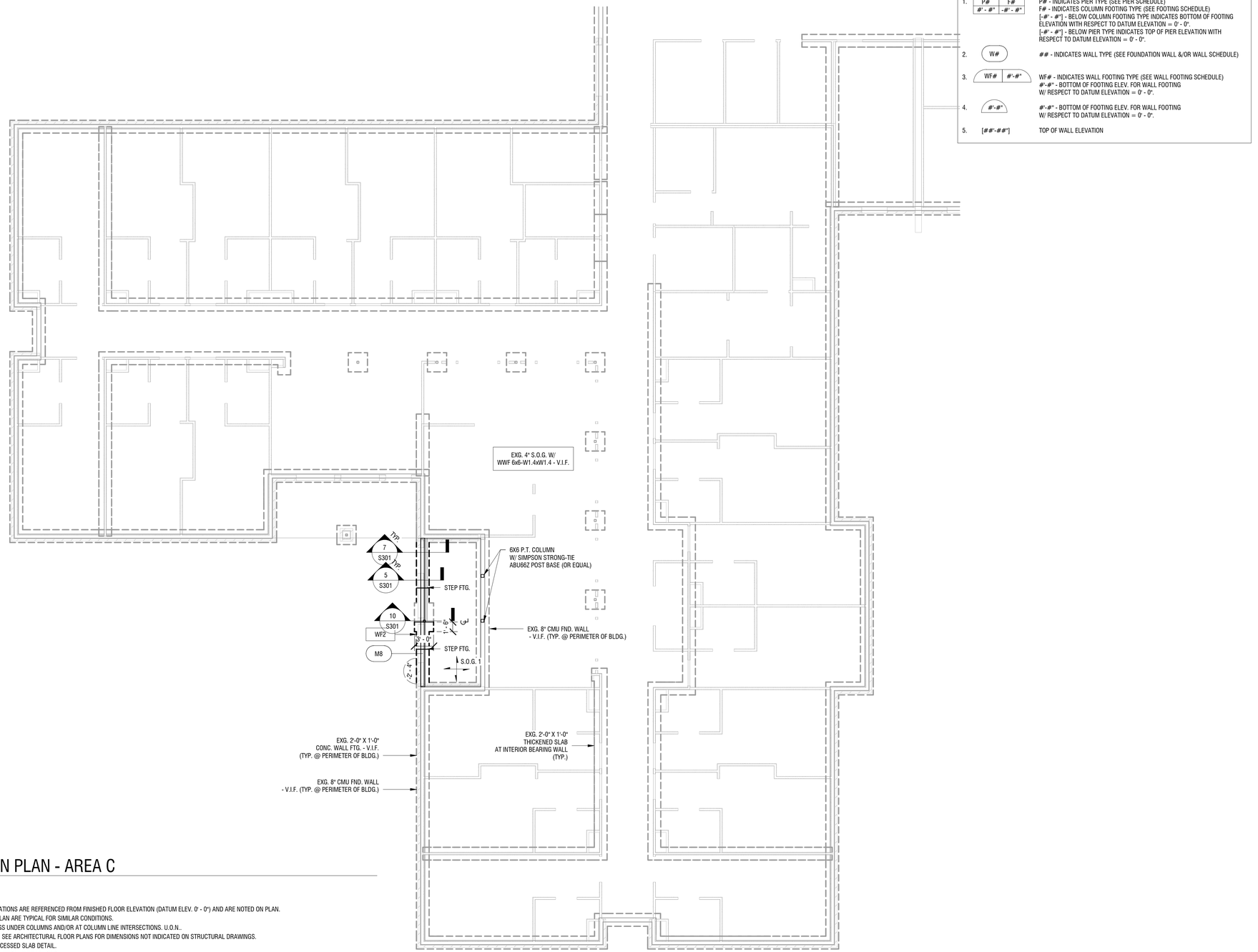
FOUNDATION PLAN - AREA B

sheet number :

S101



FOUNDATION PLAN - AREA C

1 / S102 — 1/8" = 1'-0"

FOUNDATION PLAN NOTES:

1. BOTTOM OF FOOTING ELEVATIONS ARE REFERENCED FROM FINISHED FLOOR ELEVATION (DATUM ELEV. 0' - 0") AND ARE NOTED ON PLAN.
2. SECTIONS INDICATED ON PLAN ARE TYPICAL FOR SIMILAR CONDITIONS.
3. CENTER ISOLATED FOOTINGS UNDER COLUMNS AND/OR AT COLUMN LINE INTERSECTIONS. U.O.N..
4. DO NOT SCALE DRAWINGS. SEE ARCHITECTURAL FLOOR PLANS FOR DIMENSIONS NOT INDICATED ON STRUCTURAL DRAWINGS.
5. SEE DETAIL 1/S301 FOR RECESSED SLAB DETAIL.
6. SEE DETAIL 2/S301 FOR CONTROL JOINT SPACING.
7. PROVIDE (4) 2X6 AT EXTERIOR WALLS, (5) 2X4 AT INTERIOR BEARING WALLS (MIN.) AT ALL GIRDER TRUSS BEARING POINTS WITH SIMPSON HTT4 AT STUD BASE.
8. ALL EXTERIOR STUDS SHALL BE 2X6 SPF NO. 2 STUDS AT 16" O.C. AND ALL INTERIOR STUDS SHALL BE 2X4 SPF NO. 2 STUDS AT 16" O.C.

FOUNDATION LEGEND

1.	P# | F# / #' - #" | -#' - #"	P# - INDICATES PIER TYPE (SEE PIER SCHEDULE) F# - INDICATES COLUMN FOOTING TYPE (SEE FOOTING SCHEDULE) [-#' - #"] - BELOW COLUMN FOOTING TYPE INDICATES BOTTOM OF FOOTING ELEVATION WITH RESPECT TO DATUM ELEVATION = 0' - 0". [-#' - #"] - BELOW PIER TYPE INDICATES TOP OF PIER ELEVATION WITH RESPECT TO DATUM ELEVATION = 0' - 0".
2.	W#	## - INDICATES WALL TYPE (SEE FOUNDATION WALL &/OR WALL SCHEDULE)
3.	WF# | #'-#"	WF# - INDICATES WALL FOOTING TYPE (SEE WALL FOOTING SCHEDULE) #'-#" - BOTTOM OF FOOTING ELEV. FOR WALL FOOTING W/ RESPECT TO DATUM ELEVATION = 0' - 0".
4.	#'-#"	#'-#" - BOTTOM OF FOOTING ELEV. FOR WALL FOOTING W/ RESPECT TO DATUM ELEVATION = 0' - 0".
5.	[##'-##"]	TOP OF WALL ELEVATION

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02/18/2025

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CONSTRUCTION DOCUMENTS

FOR CONSTRUCTION

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No.	Description	Date

date: 02/06/2025

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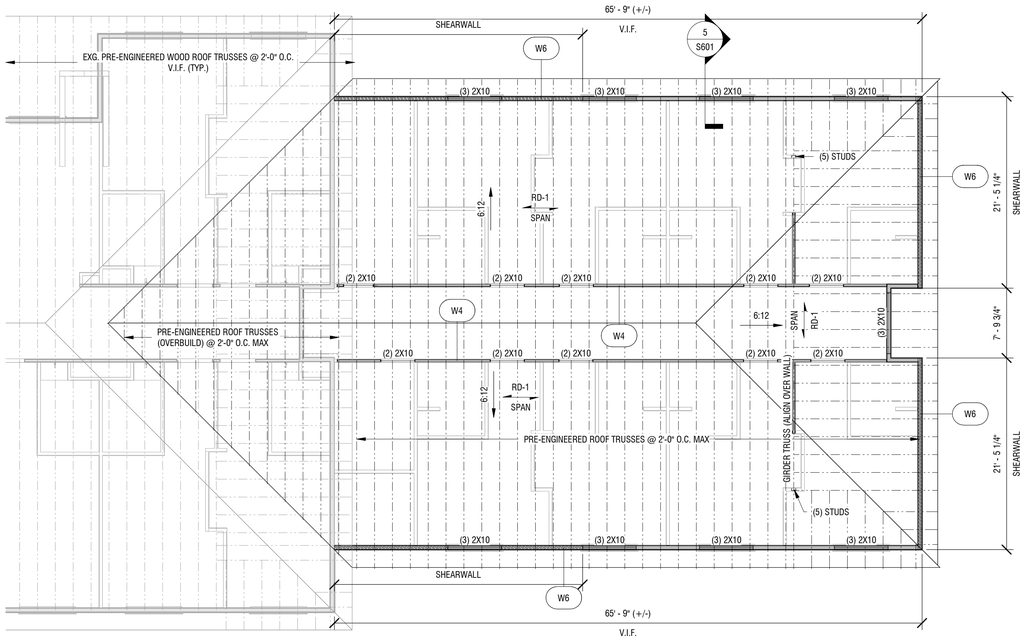
FOUNDATION PLAN - AREA C

sheet number :
S102

FRAMING LEGEND		
1.	S# / SPAN	ROOF DECK: ARROWS INDICATE SPAN DIRECTION # = DECK MARK (SEE ROOF DECK SCHEDULE)
2.	W#	WALL MARK: SEE WALL SCHEDULE
3.	F.D.	F.D. = FLOOR DRAIN (SEE MECH. & ARCH.)
4.	C.O.	C.O. = CLEAN OUT (SEE MECH. & ARCH.)
5.	R.D.	R.D. = ROOF DRAIN (SEE MECH. & ARCH.)
6.		INDICATES SHEAR WALL LOCATION
7.		INDICATES LOAD BEARING WALL LOCATION



1 S200 ROOF FRAMING PLAN - AREA A

1/8" = 1'-0"

ROOF FRAMING PLAN NOTES:

1. ALL TRUSS SPACING IS AT 2'-0" O.C. UNLESS NOTED OTHERWISE. SPACE TRUSSES AT ATTIC ACCESS DOORS TO ALLOW FOR PROPER INSTALLATION. VERIFY ATTIC ACCESS LOCATIONS WITH ARCHITECTURAL DRAWINGS.
2. TRUSS FABRICATOR SHALL VERIFY ALL DIMENSIONS AND LAYOUTS AND COORDINATE WITH BEARING WALL AND BEAM LOCATIONS.
3. DESIGN ROOF TRUSSES FOR ADDITIONAL MECHANICAL, SPRINKLER AND ARCHITECTURAL LOADS AS REQUIRED.
4. PROVIDE L5X3 1/2X5/16 (LLV) BRICK LINTEL FOR ALL OPENINGS 6'-6" OR LESS.
5. ROOF SHEATHING SHALL BE CONTINUOUS BENEATH TRUSS OVERBUILD. PROVIDE ATTACHMENT OF OVERBUILD FRAMING TO ROOF SHEATHING AND TRUSSES BELOW ACCORDING TO TRUSS MANUFACTURER.
6. PROVIDE (4) 2X6 AT EXTERIOR WALL AND (5) 2X4 AT INTERIOR WALL BELOW ALL GIRDER TRUSS BEARING POINTS.
7. VERIFY LOCATION AND EXTENT FOR THE ROOF TOP WALKWAY AND PLATFORM. SEE MECH. DRAWINGS FOR LOCATIONS.

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02/18/2025

LABELLA ASSOCIATES P.C. REGISTERED ARCHITECTURAL CORPORATION NORTH CAROLINA CHARLOTTE, N.C. CERT. NO. 52904

SPRINGBROOK NURSING AND REHABILITATION CENTER ADDITION

195 SPRINGBROOK AVE, CLAYTON, NC

CONSTRUCTION DOCUMENTS

FOR CONSTRUCTION

Revisions

No.	Description	Date

date: 02/06/2025

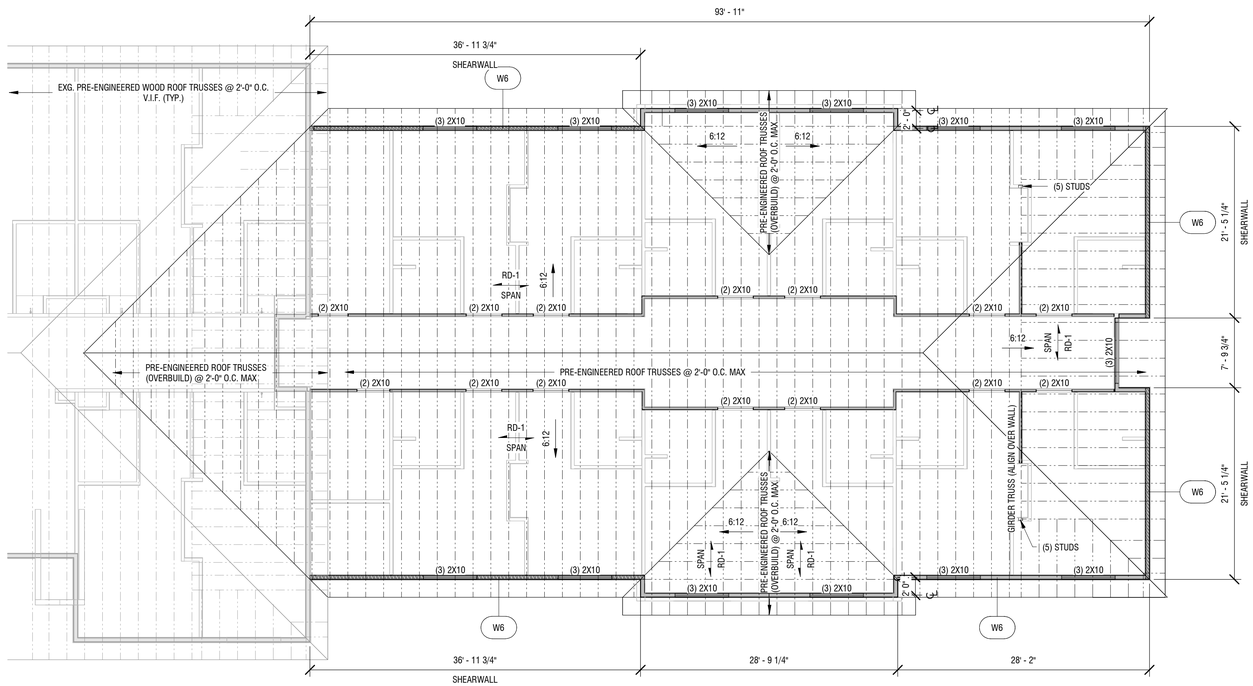
commission: NH-3138

sheet title:
ROOF FRAMING PLAN - AREA A

sheet number :

S200

FRAMING LEGEND		
1.	S# / SPAN	ROOF DECK: ARROWS INDICATE SPAN DIRECTION # = DECK MARK (SEE ROOF DECK SCHEDULE)
2.	W#	WALL MARK: SEE WALL SCHEDULE
3.	F.D.	F.D. = FLOOR DRAIN (SEE MECH. & ARCH.)
4.	C.O.	C.O. = CLEAN OUT (SEE MECH. & ARCH.)
5.	R.D.	R.D. = ROOF DRAIN (SEE MECH. & ARCH.)
6.		INDICATES SHEAR WALL LOCATION
7.		INDICATES LOAD BEARING WALL LOCATION



1 S201

ROOF FRAMING PLAN - AREA B

1/8" = 1'-0"

ROOF FRAMING PLAN NOTES:

1. ALL TRUSS SPACING IS AT 2'-0" O.C. UNLESS NOTED OTHERWISE. SPACE TRUSSES AT ATTIC ACCESS DOORS TO ALLOW FOR PROPER INSTALLATION. VERIFY ATTIC ACCESS LOCATIONS WITH ARCHITECTURAL DRAWINGS.
2. TRUSS FABRICATOR SHALL VERIFY ALL DIMENSIONS AND LAYOUTS AND COORDINATE WITH BEARING WALL AND BEAM LOCATIONS.
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6. PROVIDE (4) 2X6 AT EXTERIOR WALL AND (5) 2X4 AT INTERIOR WALL BELOW ALL GIRDER TRUSS BEARING POINTS.
7. VERIFY LOCATION AND EXTENT FOR THE ROOF TOP WALKWAY AND PLATFORM. SEE MECH. DRAWINGS FOR LOCATIONS.

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ROOF FRAMING PLAN - AREA B

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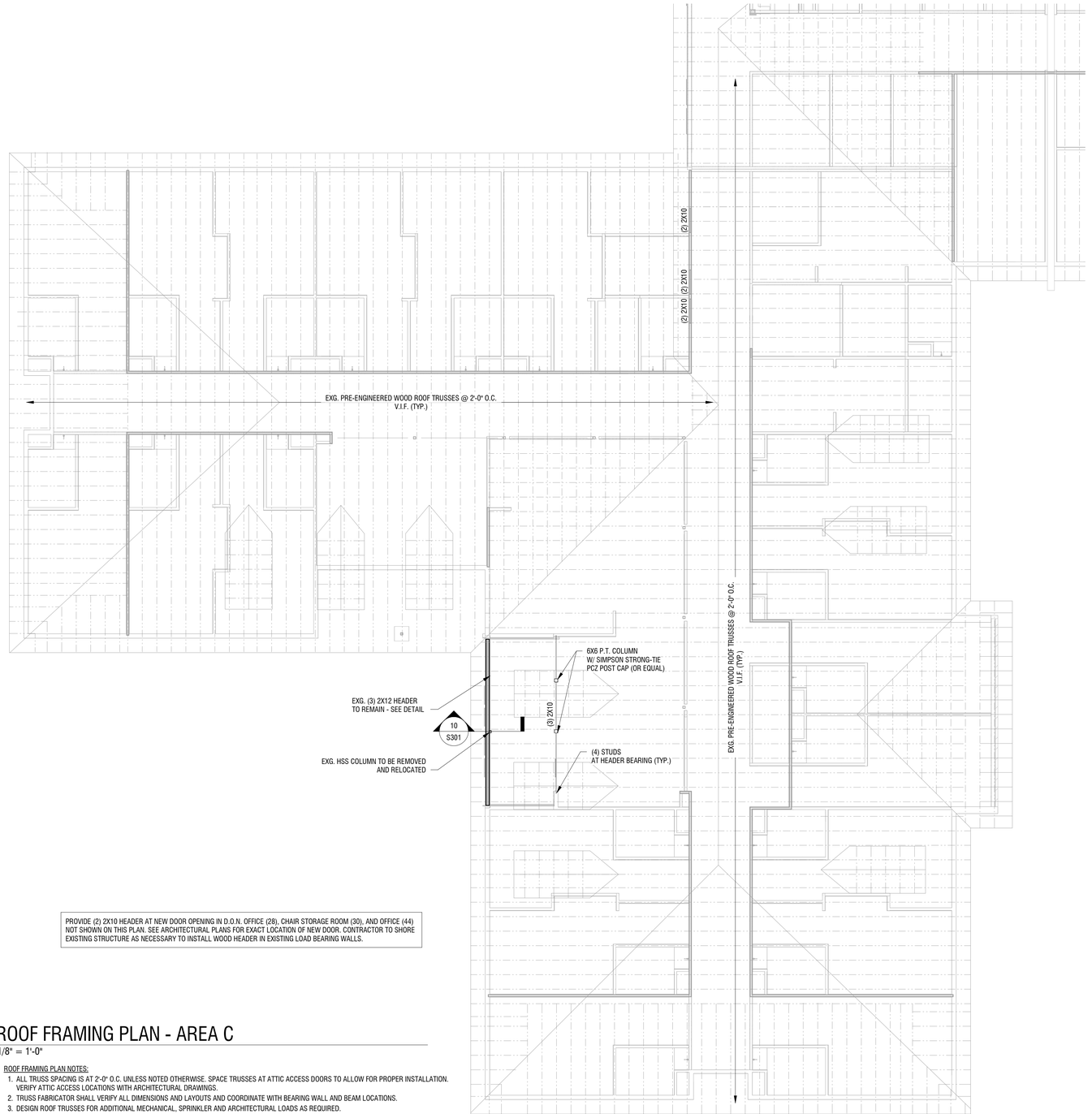
S201



FRAMING LEGEND		
1.	S# SPAN	ROOF DECK: ARROWS INDICATE SPAN DIRECTION # = DECK MARK (SEE ROOF DECK SCHEDULE)
2.	W#	WALL MARK: SEE WALL SCHEDULE
3.	F.D.	F.D. = FLOOR DRAIN (SEE MECH. & ARCH.)
4.	C.O.	C.O. = CLEAN OUT (SEE MECH. & ARCH.)
5.	R.D.	R.D. = ROOF DRAIN (SEE MECH. & ARCH.)
6.		INDICATES SHEAR WALL LOCATION
7.		INDICATES LOAD BEARING WALL LOCATION

PROVIDE (2) 2X10 HEADER AT NEW DOOR OPENING IN D.O.N. OFFICE (28), CHAIR STORAGE ROOM (30), AND OFFICE (44) NOT SHOWN ON THIS PLAN. SEE ARCHITECTURAL PLANS FOR EXACT LOCATION OF NEW DOOR. CONTRACTOR TO SHORE EXISTING STRUCTURE AS NECESSARY TO INSTALL WOOD HEADER IN EXISTING LOAD BEARING WALLS.

1 / S202 ROOF FRAMING PLAN - AREA C

1/8" = 1'-0"

ROOF FRAMING PLAN NOTES:

1. ALL TRUSS SPACING IS AT 2'-0" O.C. UNLESS NOTED OTHERWISE. SPACE TRUSSES AT ATTIC ACCESS DOORS TO ALLOW FOR PROPER INSTALLATION. VERIFY ATTIC ACCESS LOCATIONS WITH ARCHITECTURAL DRAWINGS.
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7. VERIFY LOCATION AND EXTENT FOR THE ROOF TOP WALKWAY AND PLATFORM. SEE MECH. DRAWINGS FOR LOCATIONS.

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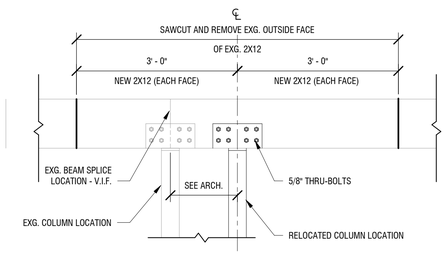
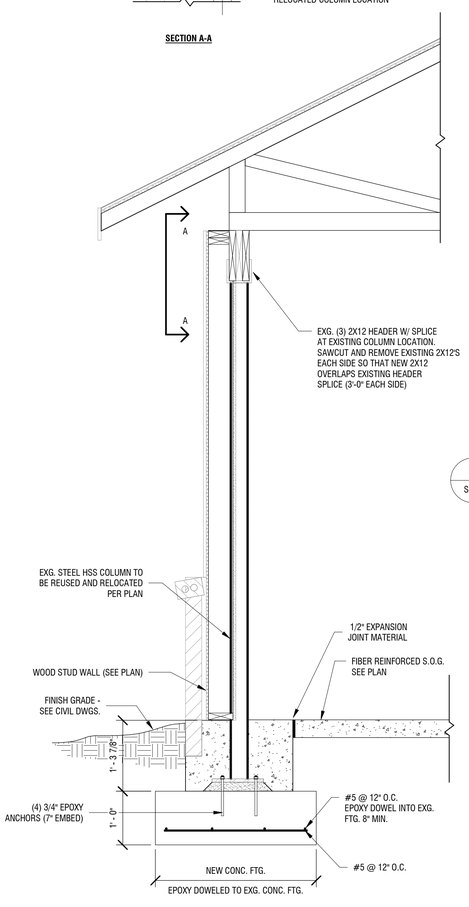
ROOF FRAMING PLAN - AREA C

sheet number :

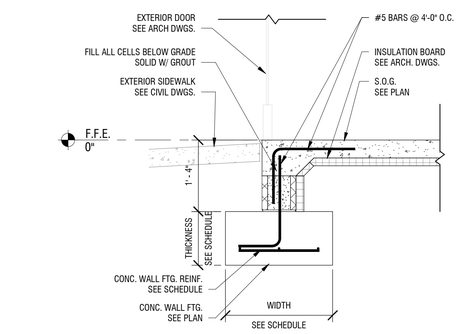
S202



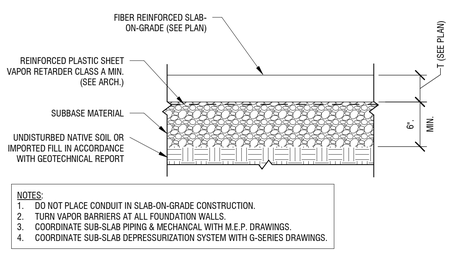
SECTION A-A



10 S301 ENCLOSED BALCONY DETAIL
3/4" = 1'-0"

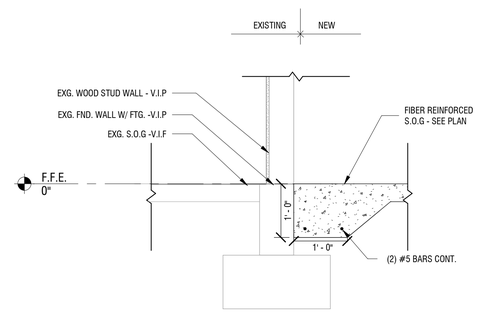


7 S301 EXTERIOR DOOR DETAIL
3/4" = 1'-0"

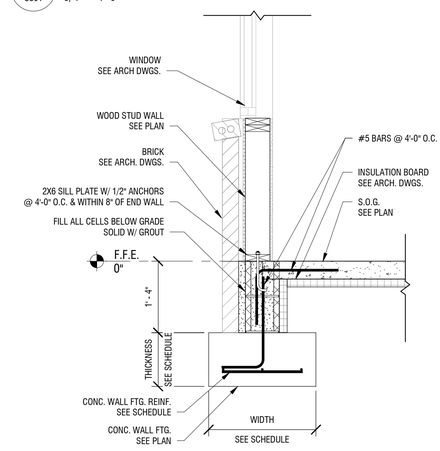


NOTES:
1. DO NOT PLACE CONDUIT IN SLAB-ON-GRADE CONSTRUCTION.
2. TURN VAPOR BARRIERS AT ALL FOUNDATION WALLS.
3. COORDINATE SUB-SLAB PIPING & MECHANCAL WITH M.E.P. DRAWINGS.
4. COORDINATE SUB-SLAB DEPRESSURIZATION SYSTEM WITH G-SERIES DRAWINGS.

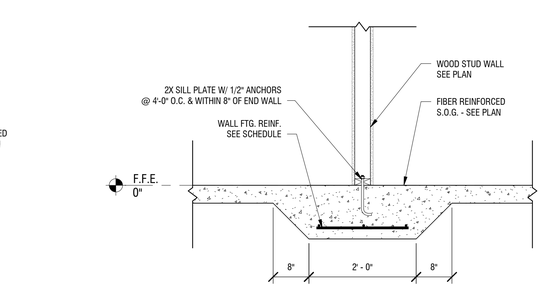
4 S301 TYPICAL SLAB-ON-GRADE WITH VAPOR BARRIER
3/4" = 1'-0"



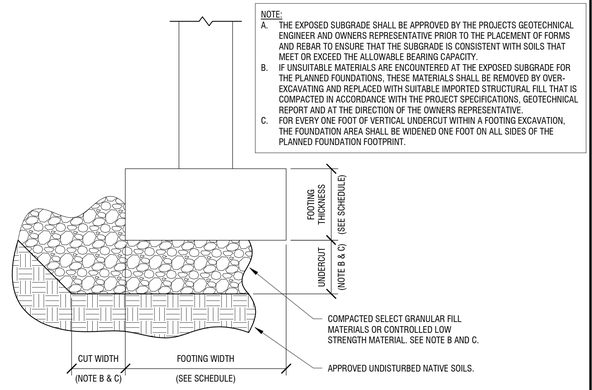
9 S301 NEW/EXISTING SLAB FOUNDATION
3/4" = 1'-0"



5 S301 FOUNDATION WALL DETAIL
3/4" = 1'-0"

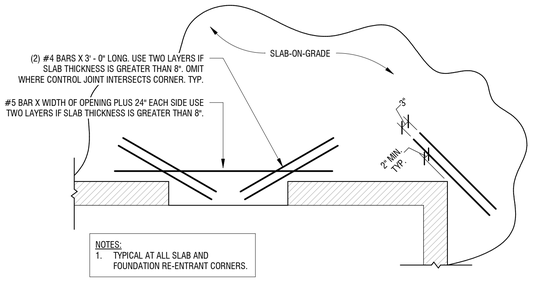


8 S301 INTERIOR LOAD BEARING WALL DETAIL
3/4" = 1'-0"



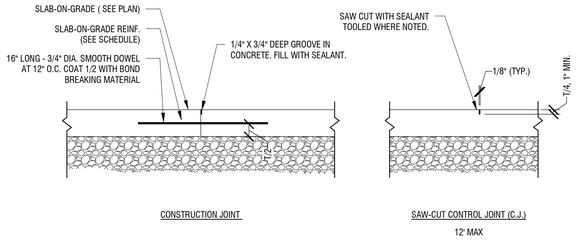
NOTE:
A. THE EXPOSED SUBGRADE SHALL BE APPROVED BY THE PROJECTS GEOTECHNICAL ENGINEER AND OWNERS REPRESENTATIVE PRIOR TO THE PLACEMENT OF FORMS AND REBAR TO ENSURE THAT THE SUBGRADE IS CONSISTENT WITH SOILS THAT MEET OR EXCEED THE ALLOWABLE BEARING CAPACITY.
B. IF UNSUITABLE MATERIALS ARE ENCOUNTERED AT THE EXPOSED SUBGRADE FOR THE PLANNED FOUNDATIONS, THESE MATERIALS SHALL BE REMOVED BY OVER-EXCAVATING AND REPLACED WITH SUITABLE IMPORTED STRUCTURAL FILL THAT IS COMPACTED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, GEOTECHNICAL REPORT AND AT THE DIRECTION OF THE OWNERS REPRESENTATIVE.
C. FOR EVERY ONE FOOT OF VERTICAL UNDERCUT WITHIN A FOOTING EXCAVATION, THE FOUNDATION AREA SHALL BE WIDENED ONE FOOT ON ALL SIDES OF THE PLANNED FOUNDATION FOOTPRINT.

6 S301 TYPICAL FOUNDATION SOIL BEARING
3/4" = 1'-0"

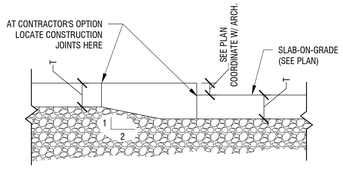


NOTES:
1. TYPICAL AT ALL SLAB AND FOUNDATION RE-ENTRANT CORNERS.

3 S301 TYPICAL SLAB-ON-GRADE RE-ENTRANT CORNER
1/2" = 1'-0"



2 S301 TYPICAL SLAB-ON-GRADE JOINT
3/4" = 1'-0"



1 S301 TYPICAL RECESSED SLAB DETAIL
1" = 1'-0"

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SPRINGBROOK NURSING AND REHABILITATION CENTER ADDITION
195 SPRINGBROOK AVE, CLAYTON, NC

CONSTRUCTION DOCUMENTS
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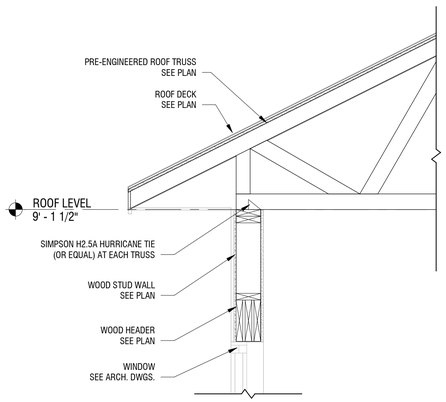
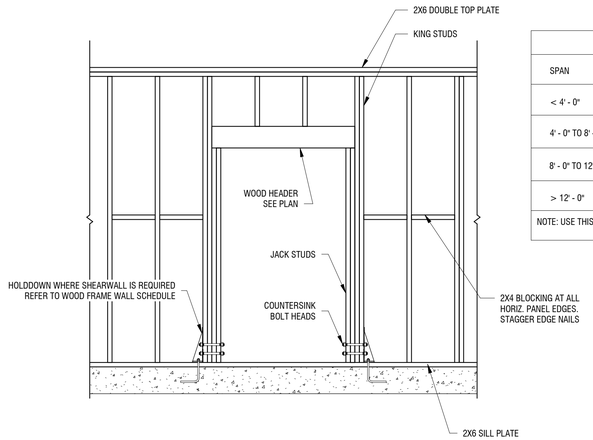
TYPICAL SLAB-ON-GRADE & FOUNDATION DETAILS

sheet number:
S301



5 S601 ROOF FRAMING DETAIL

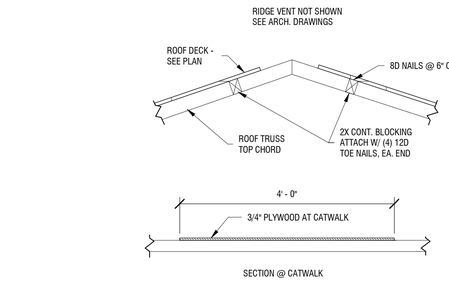
3/4" = 1'-0"



JACK AND KING STUD SCHEDULE		
SPAN	NO. OF JACK STUDS	NO. OF KING STUDS
< 4' - 0"	2	2
4' - 0" TO 8' - 0"	2	2
8' - 0" TO 12' - 0"	2	3
> 12' - 0"	NOTIFY ENGINEER	NOTIFY ENGINEER
NOTE: USE THIS SCHEDULE IF NO HEADER IS CALLED OUT ON PLANS		

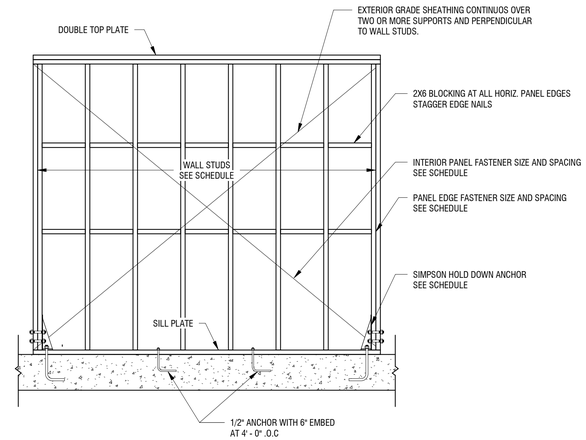
4 S601 TYPICAL WALL OPENING DETAIL

1/2" = 1'-0"



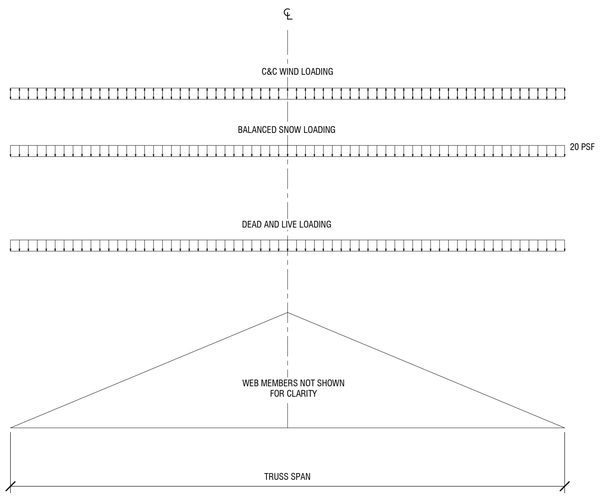
6 S601 SECTION @ RIDGE & CATWALK

3/4" = 1'-0"



3 S601 TYPICAL SHEARWALL ELEVATION

1/2" = 1'-0"



TOP CHORD SUPERIMPOSED DEAD LOAD = 10 PSF
BOTTOM CHORD COLLATERAL DEAD LOAD = 10 PSF
*NOTES
1. TRUSS DESIGN SHALL INCLUDE A 250 LB LOAD PER NFPA TO SUPPORT SPRINKLER LOADS LOCATED ANYWHERE ALONG THE BOTTOM CHORD OF THE TRUSS.
2. TRUSS DESIGN SHALL INCLUDE WEIGHT OF ALL HUNG MECHANICAL EQUIPMENT. COORDINATE LOCATIONS AND LOADING WITH MEP DRAWINGS.

LIVE LOAD = 20 PSF
LIVE LOAD AT CATWALK ONLY= 40 PSF
LIVE LOAD IN ATTICS WITHOUT A STORAGE= 10 PSF
BALANCED SNOW LOAD = SEE TRUSS LOADING DIAGRAMS
UNBALANCED SNOW LOAD = SEE TRUSS LOADING DIAGRAMS
DRIFT SNOW LOAD = SEE TRUSS LOADING DIAGRAMS

WIND C&C LOADING
TRUSS TYPE 1

DOWNWARD WIND PRESSURES
ZONE 1 = 16 PSF
ZONE 2 = 24 PSF
ZONE 3 = 24 PSF

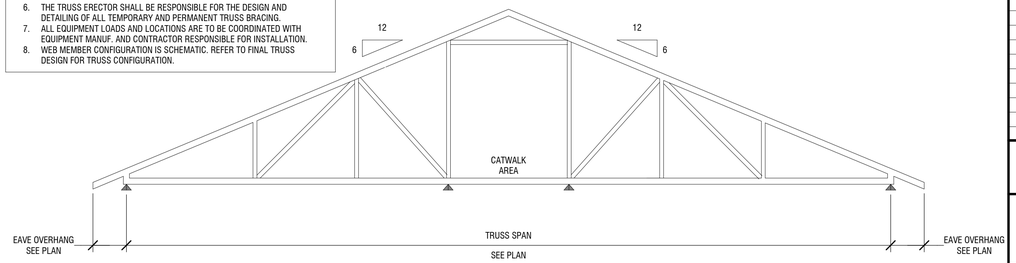
MINIMUM WIND UPLIFT PRESSURES
ZONE 1 = 16 PSF
ZONE 2 = 20 PSF
ZONE 3 = 20 PSF

DIAPHRAMS LOADS
STORY SHEAR IN BOTH MAJOR AND MINOR DIRECTION = 150 PLF

2 S601 TRUSS LOADING DIAGRAMS

1/8" = 1'-0"

NOTES:
1. ROOF TRUSSES SHALL BE DESIGNED AND DETAILED BY PRE-ENGINEERED TRUSS MANUFACTURER.
2. TRUSS MANUFACTURER TO PROVIDE HEEL BLOCKING AND CLIPS CAPBLE OF TRANSFERRING DIAPHRAMS LOADS LISTED ABOVE.
3. LOADS LISTED ABOVE ARE BASED ON SLOPED LOADS. TRUSS MANUFACTURER SHALL ADJUST LOADS IN THE HORIZONTAL PROJECTION BASED ON THE ROOF SLOPE.
4. SEE STRUCTURAL DESIGN TABLE FOR DESIGN INFORMATION.
5. SEE STRUCTURAL AND ARCHITECTURAL PLANS AND SECTIONS FOR REQUIRED TRUSS DIMENSIONS.
6. THE TRUSS ERECTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND DETAILING OF ALL TEMPORARY AND PERMANENT TRUSS BRACING.
7. ALL EQUIPMENT LOADS AND LOCATIONS ARE TO BE COORDINATED WITH EQUIPMENT MANUF. AND CONTRACTOR RESPONSIBLE FOR INSTALLATION.
8. WEB MEMBER CONFIGURATION IS SCHEMATIC. REFER TO FINAL TRUSS DESIGN FOR TRUSS CONFIGURATION.



TRUSS TYPE 1

1 S601 TRUSS PROFILES

3/16" = 1'-0"

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