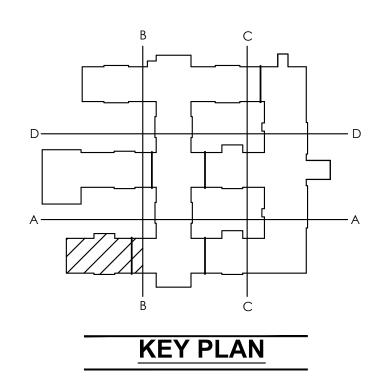
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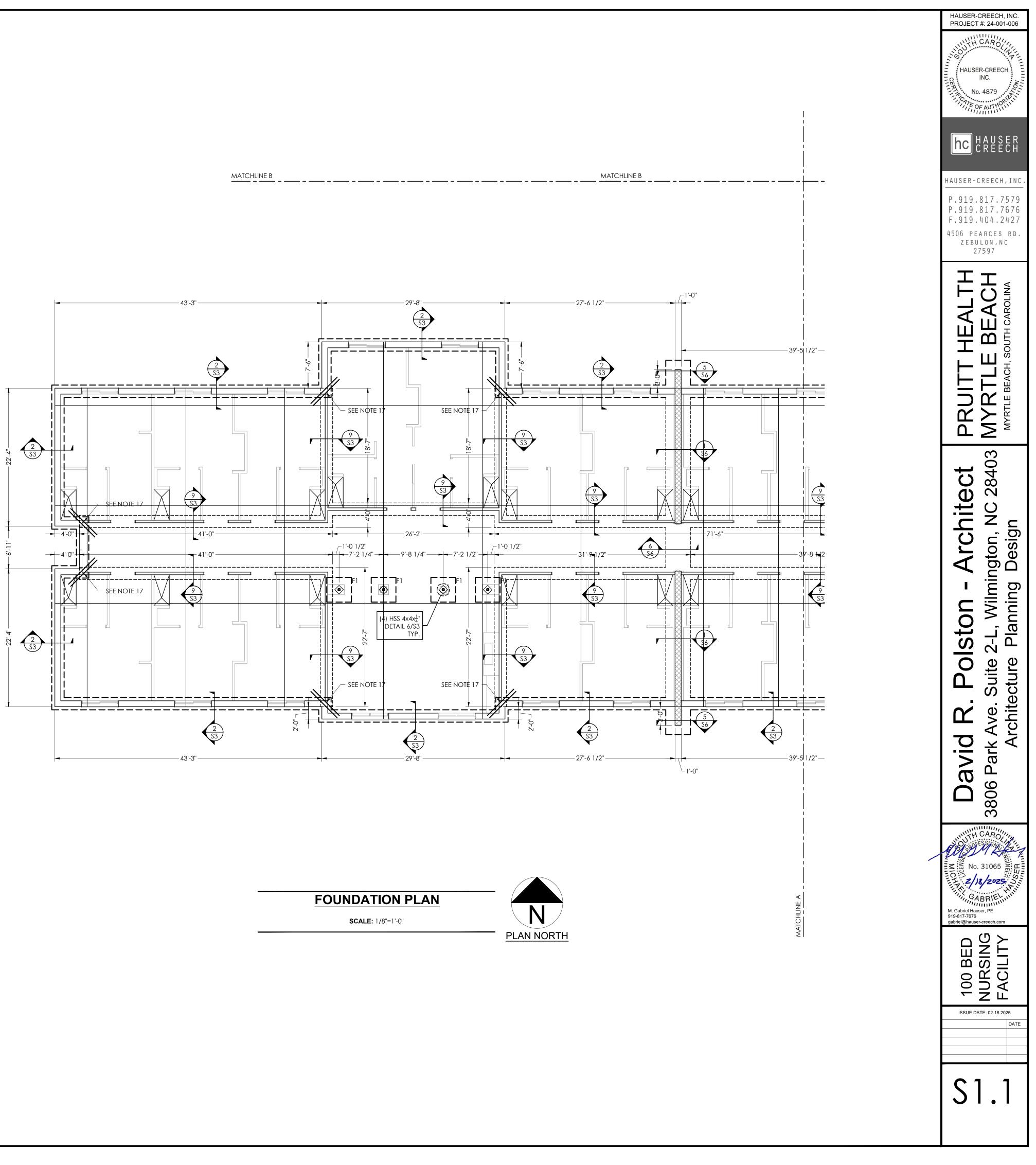
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WRAP ALL EXTERIOR WALLS WITH MINIMUM 7_6 " OSB. PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED JOINTS. EDGE NAIL WITH 8d COMMONS AT 4" O.C. AND FIELD NAIL WITH 8d COMMONS AT 12" O.C.

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F4	6'-0'' X 6'-0'' X 1'-6''	(6) #6s (5'-6" LONG) E.W. T + B
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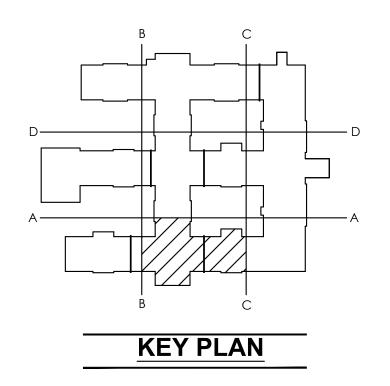
MATCHLINE B

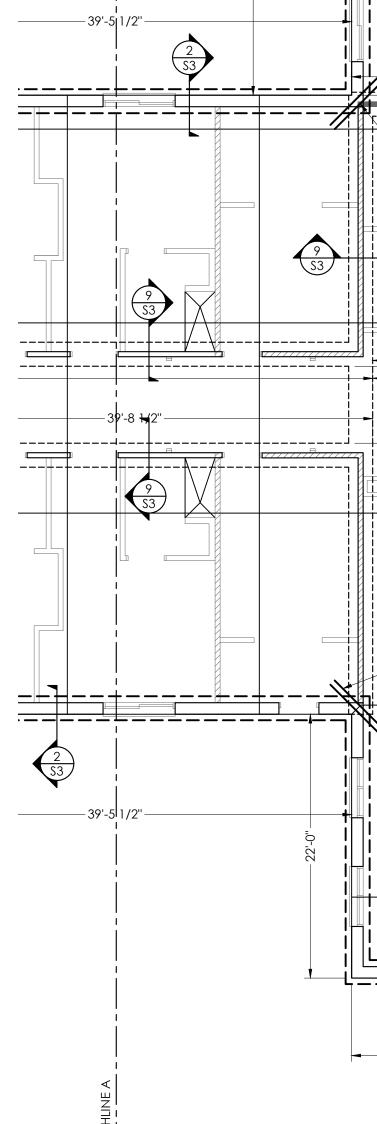


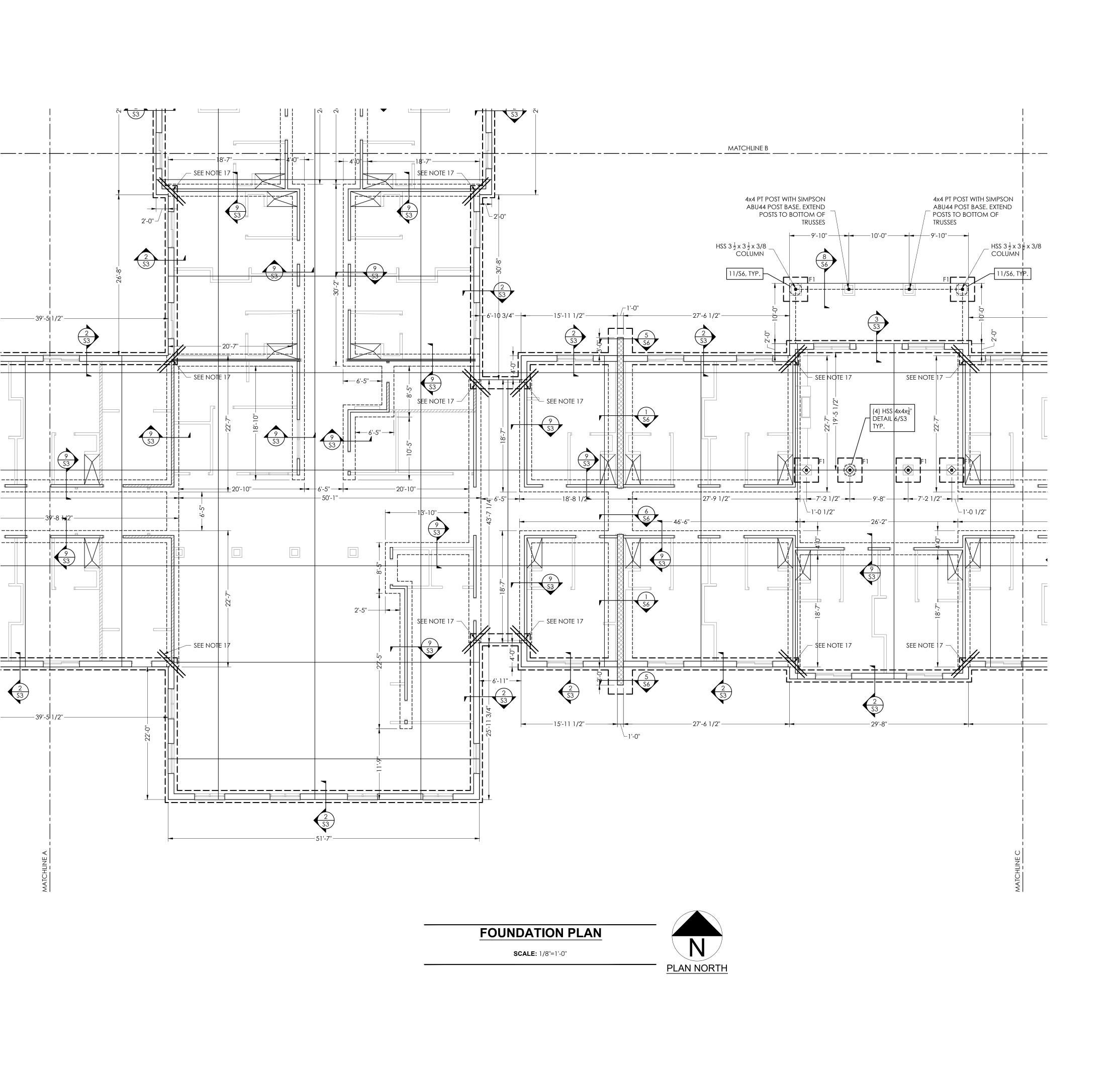
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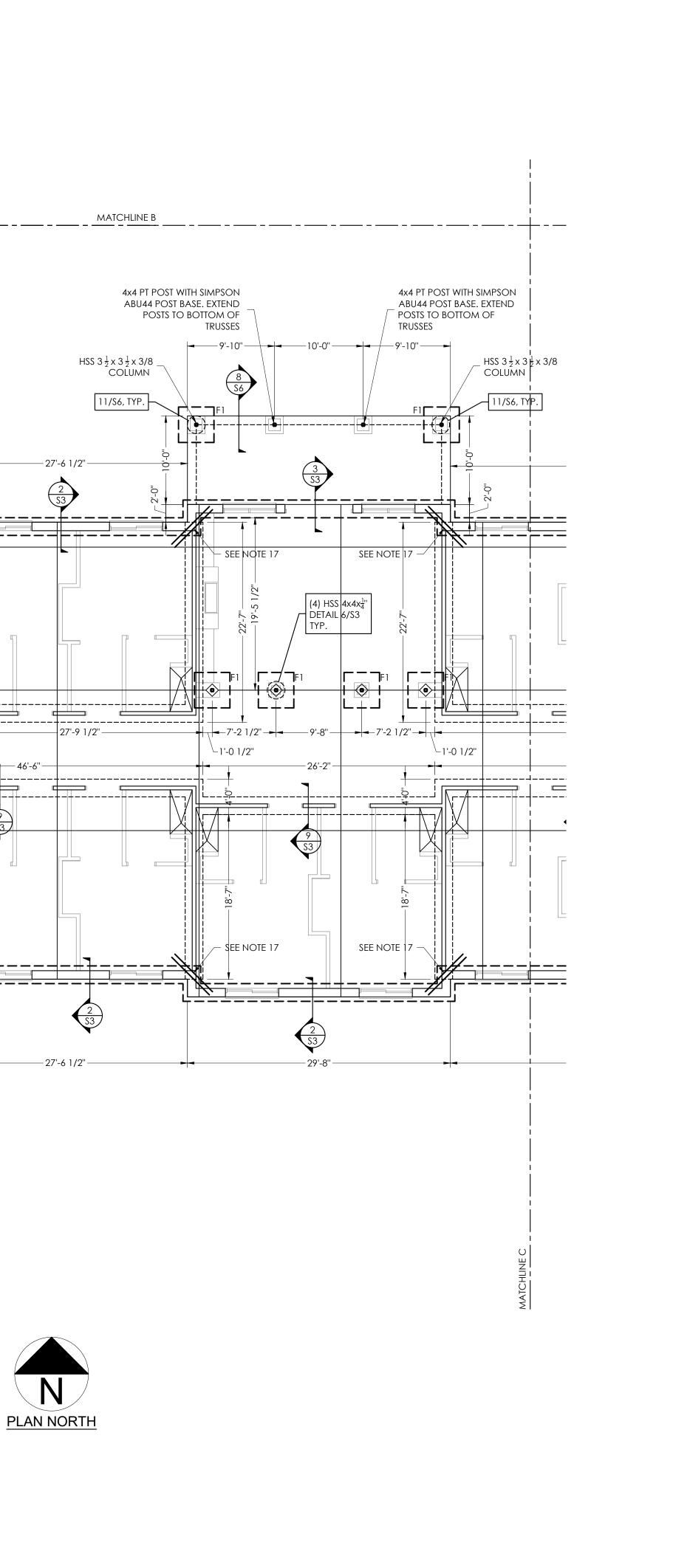
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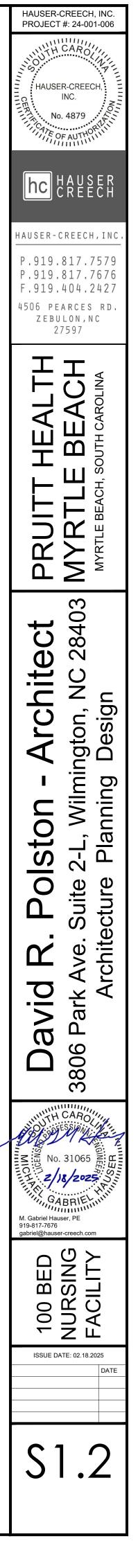






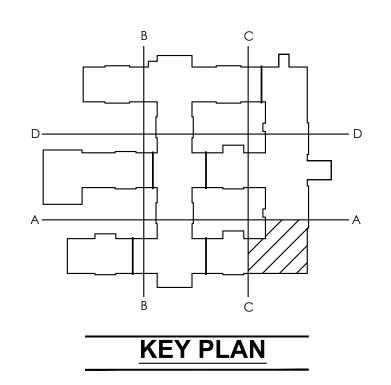


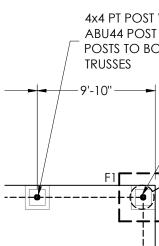


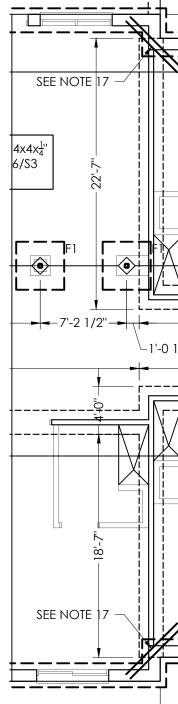


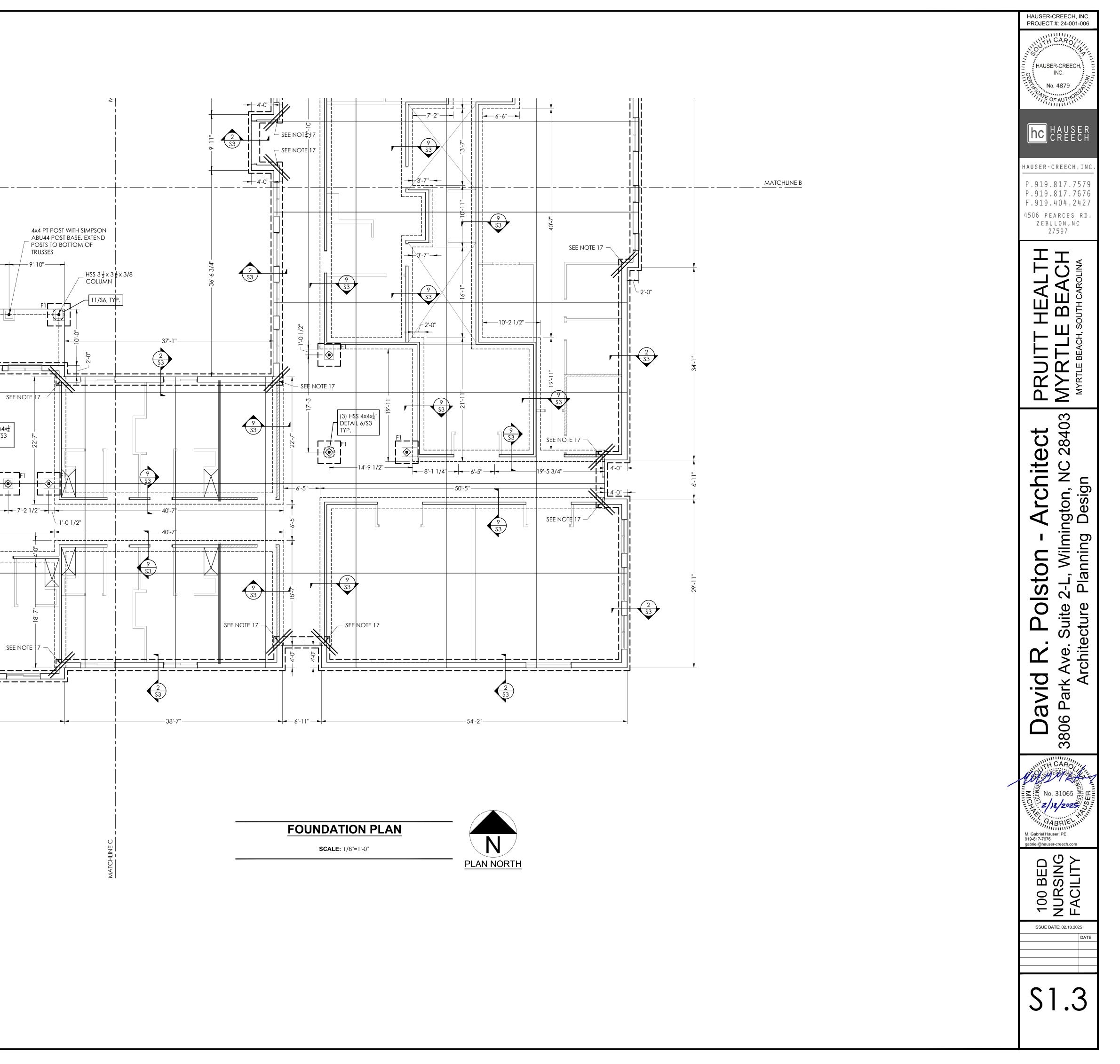
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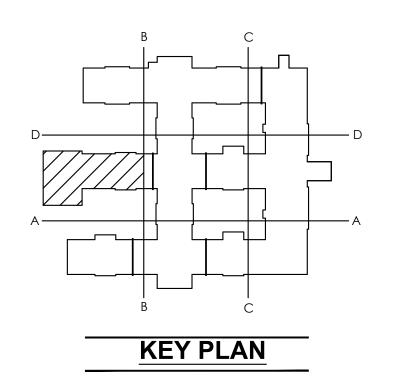


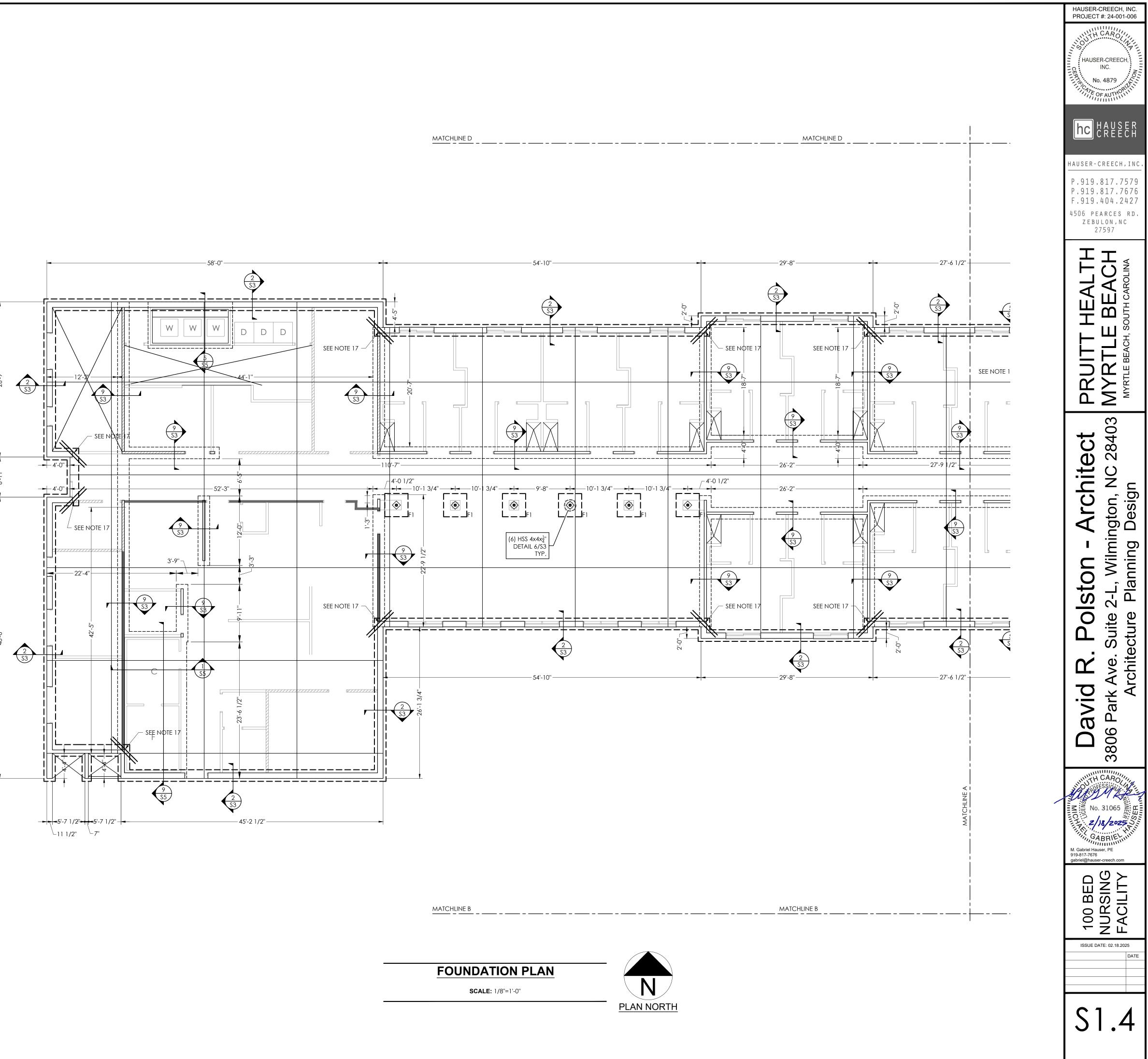


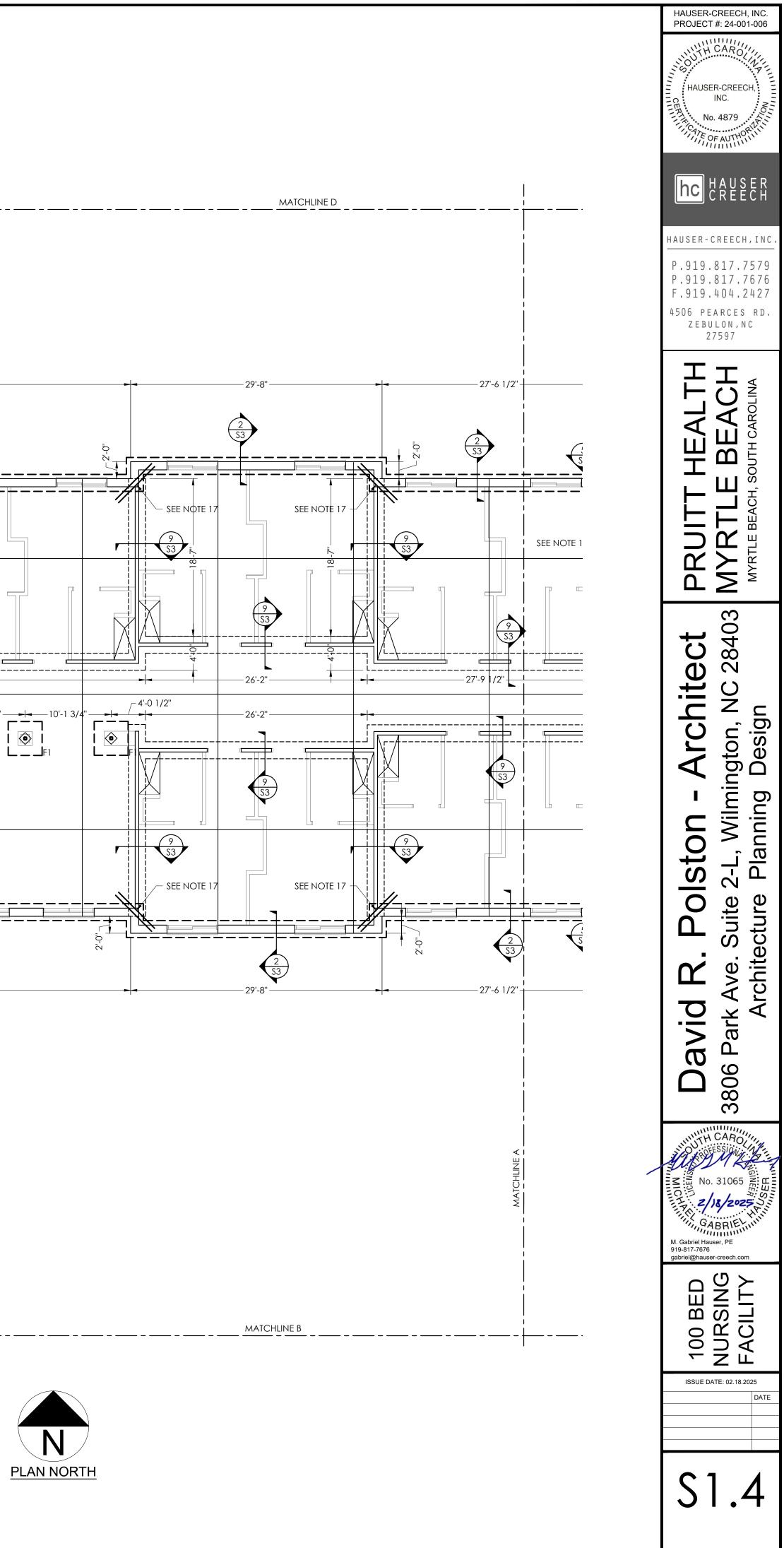
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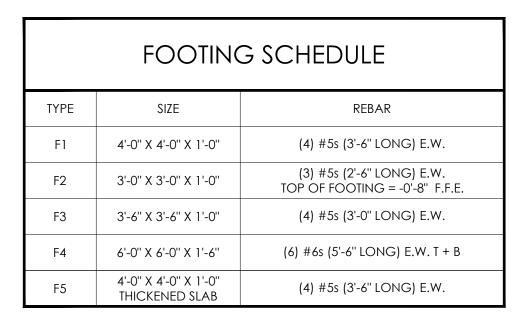


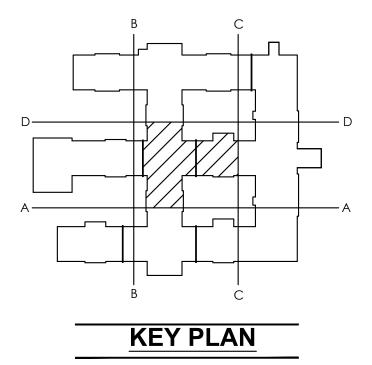


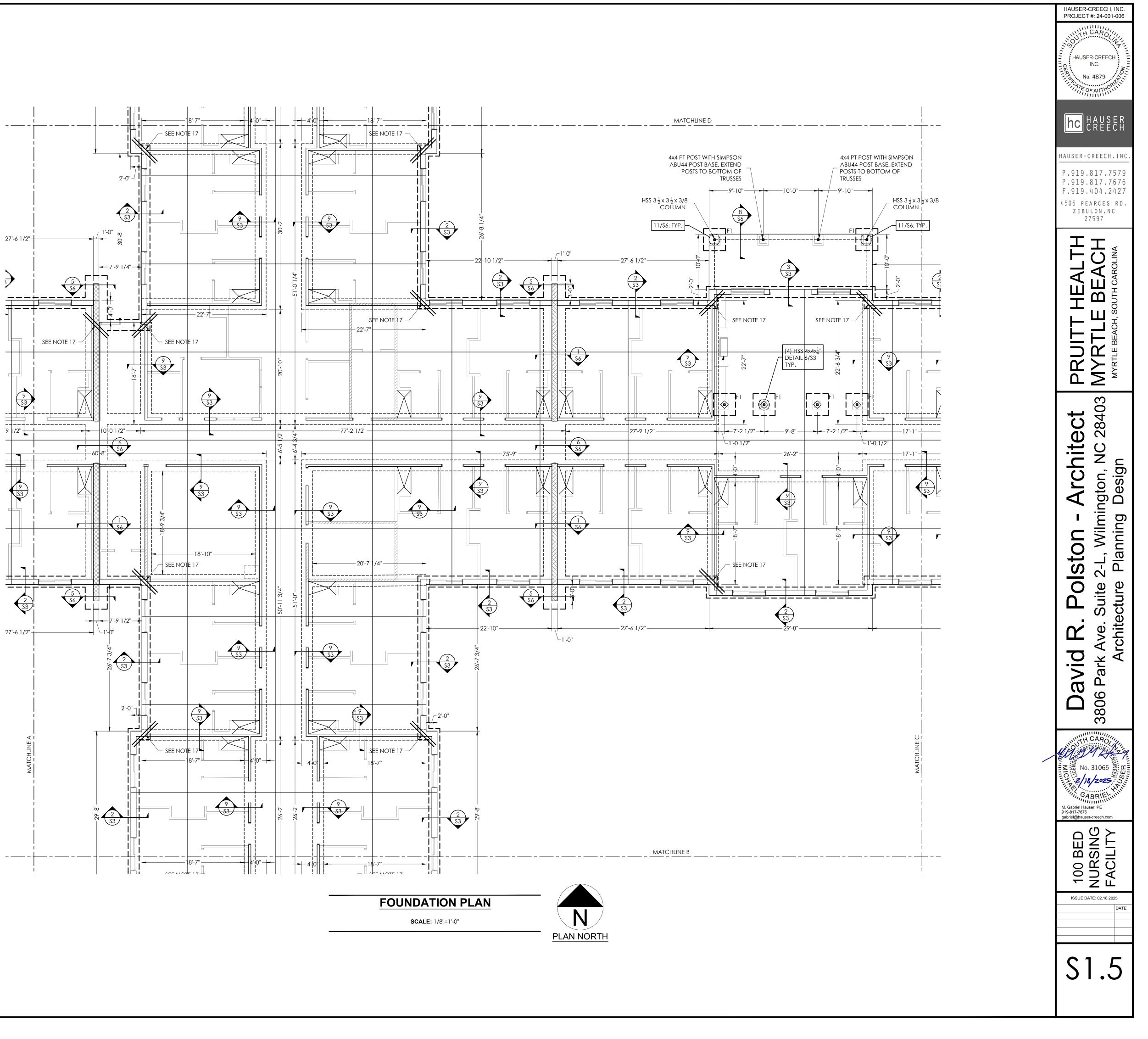


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SIMPSON

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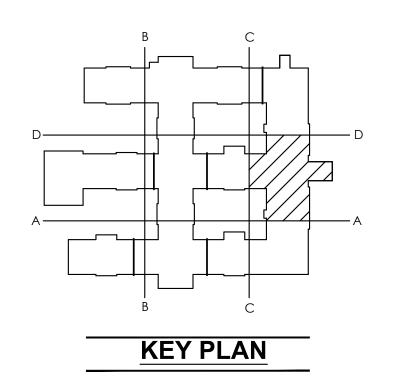
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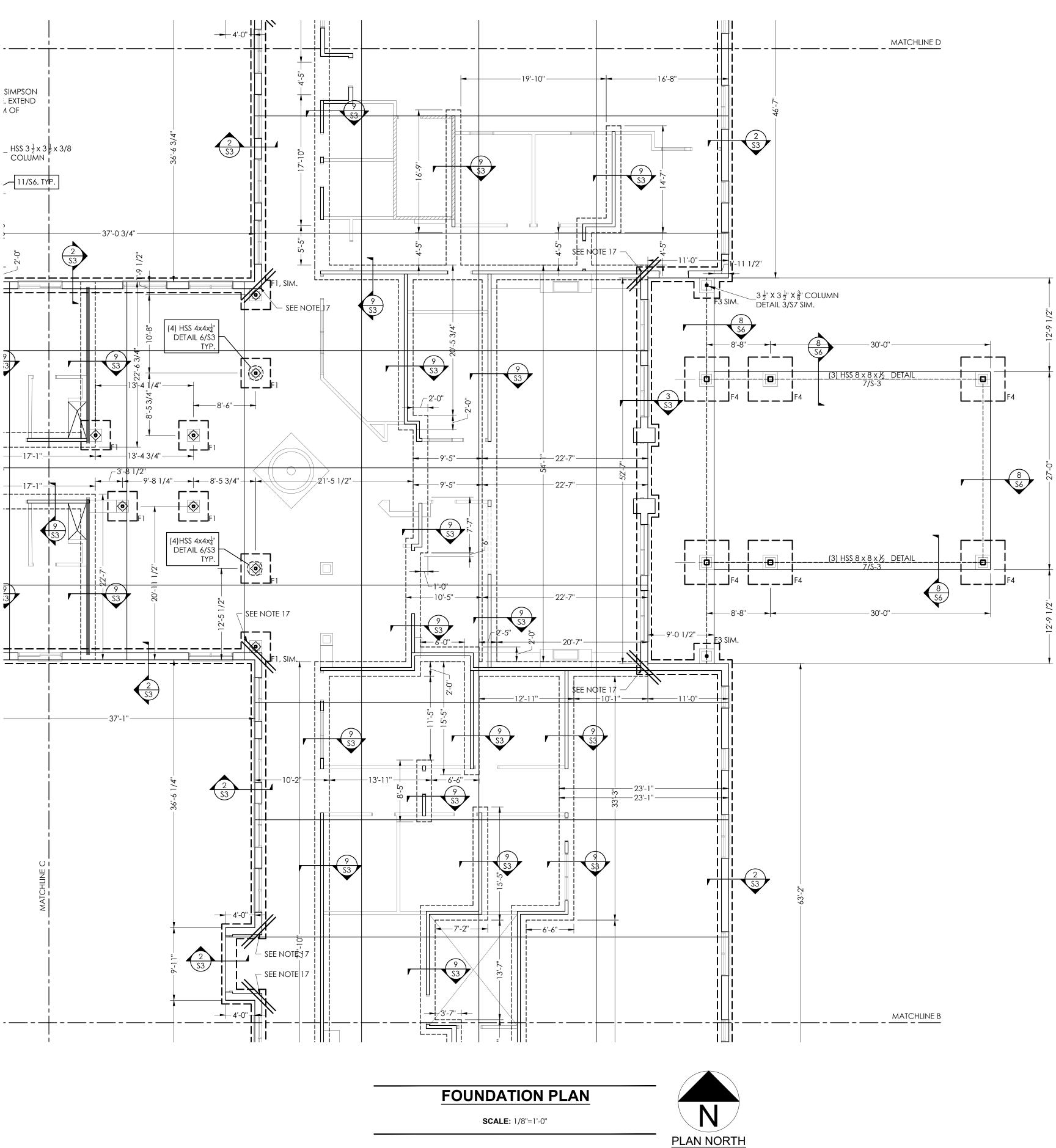
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- SEE MECHANICAL PLANS FOR LOCATIONS AND QUANTITIES. 16. CONTRACTOR MUST PROVIDE AN ENGINEERED SIGN WALL INSTALLATION. SEE THE ATTACH DETAIL 8 ON S9 FOR A GENERAL SUPPORT INSTALLATION. CONTRACTOR MUST VERIFY THE SIGN SIZE, ATTACHMENT, AND LOCATION WITH SIGN SHOP DRAWINGS.
- 17. PROVIDE (2) 6'-0" LONG #5 BARS AT RE-ENTRANT CORNERS, PLACE AT MID-DEPTH OF SLAB. 18. QUICKTIE IS AN ACCEPTABLE ALTERNATIVE TO HOLD DOWNS. SUBMIT SHOP DRAWINGS FOR APPROVAL.

"SHEARWALL" DESIGNATES INTERIOR 2X4 STUDS SHEATHED W/ | MINIMUM $\frac{7}{16}$ " OSB. PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED JOINTS. EDGE NAIL WITH 8d COMMONS AT 4" O.C. AND FIELD NAIL WITH 8d COMMONS AT 12" O.C.

FOOTING SCHEDULE		
TYPE	SIZE	REBAR
F1	4'-0'' X 4'-0'' X 1'-0''	(4) #5s (3'-6" LONG) E.W.
F2	3'-0" X 3'-0" X 1'-0"	(3) #5s (2'-6" LONG) E.W. TOP OF FOOTING = -0'-8" F.F.E.
F3	3'-6'' X 3'-6'' X 1'-0''	(4) #5s (3'-0" LONG) E.W.
F4	6'-0'' X 6'-0'' X 1'-6''	(6) #6s (5'-6" LONG) E.W. T + B
F5	4'-0" X 4'-0" X 1'-0" THICKENED SLAB	(4) #5s (3'-6" LONG) E.W.



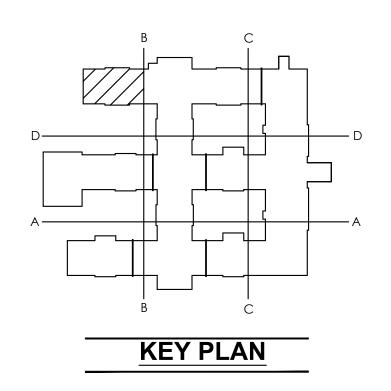


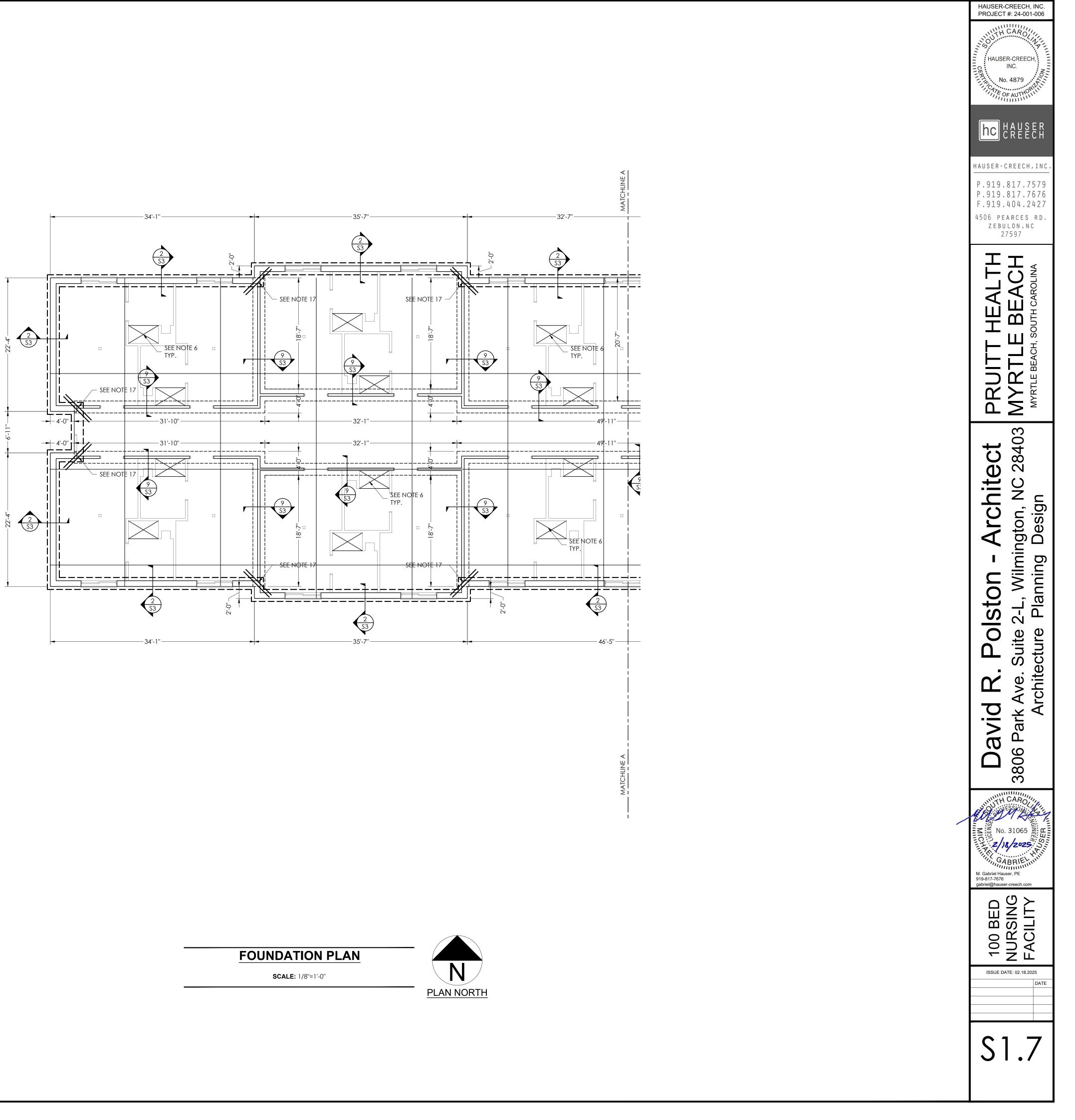
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HAUSER-CREECH, INC. P.919.817.7579 P.919.817.7676 F.919.404.2427 4506 PEARCES RD. ZEBULON, NC 27597
PRUITT HEALTH MYRTLE BEACH MYRTLE BEACH MYRTLE BEACH, SOUTH CAROLINA
David R. Polston - Architect PRUITT HEAL 3806 Park Ave. Suite 2-L, Wilmington, NC 28403 MYRTLE BEA Architecture Planning Design MYRTLE BEA
M. Gabriel Hauser, PE 919-817-7676 gabriel@hauser-creech.com
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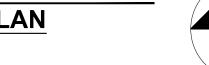
- PROVIDE 4" CONCRETE SLAB ON GRADE REINFORCED W/ WWF 6x6-W1.4xW1.4 OVER 10 MIL POLY VAPOR BARRIER (LAP EDGES 6" MIN.) OVER 4" POROUS BASE.ALL DIMENSIONS REFERENCED TO CENTERLINE OF COLUMNS, FACE OF EXTERIOR VENEER, AND CENTERLINE OF INTERIOR BEARING WALLS. SEE ARCHITECTURAL AND STRUCTURAL SECTIONS TO DETERMINE EDGE OF SLAB. VERIFY DIMENSIONS PRIOR TO CONSTRUCTION.
- TOP OF EXTERIOR FTG. = F.F.E. -1'-4" AND FIN. GRADE -1'-0" (MIN.) 2 SEE ARCH. DWGS. FOR DIMENSIONS NOT SHOWN.
- 4. SEE DETAIL 5/S-3 FOR RECESSED SLAB DETAILS.
- 5. SEE DETAIL 1/S-3 FOR SLAB CONTROL JOINTS (CJ), ALTERNATE LAYOUT PLANS MAY BE SUBMITTED FOR APPROVAL.
- 6. SEE ARCHITECTURAL DRAWINGS. FOR LOCATIONS OF RECESSED AND/OR SLOPED SLAB AREAS. PROVIDE POSITIVE DRAINAGE FROM ALL PERIMETER WALLS TO FLOOR DRAIN. COORDINATE W/ PLUMBING DWGS. SEE DETAIL 5/S-3. 7. LOCATE CONTROL JOINTS UNDERNEATH NON-BEARING WALLS WHERE POSSIBLE.
- 8. PROVIDE (4) 2X6 @ EXT. WALLS, (5) 2X4 @ INT. WALLS BEARING (MIN.) AT ALL GIRDER TRUSSES BEARING POINTS AND SHEARWALL END POSTS W/ SIMPSON HTT4 AT STUD BASE.
- 9. REFER TO ARCHITECTURAL DRAWINGS FOR RATED WALL LOCATIONS. 10. SEE FOOTING SCHEDULE FOR SIZES AND REINFORCING.
- 11. PROVIDE THICKENED SLAB AS REQUIRED BY WASHER MANUFACTURER. CONTRACTOR TO PROVIDE AND INSTALL REBAR FRAME. SEE 5/S-5. 12. ALL EXTERIOR STUDS SHALL BE 2x6 SPF NO. 2 STUDS AT 16" O.C. ALL INTERIOR STUDS AT BEARING
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FOOTING SCHEDULE		
TYPE	SIZE	REBAR
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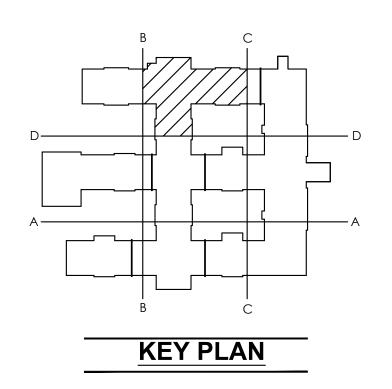


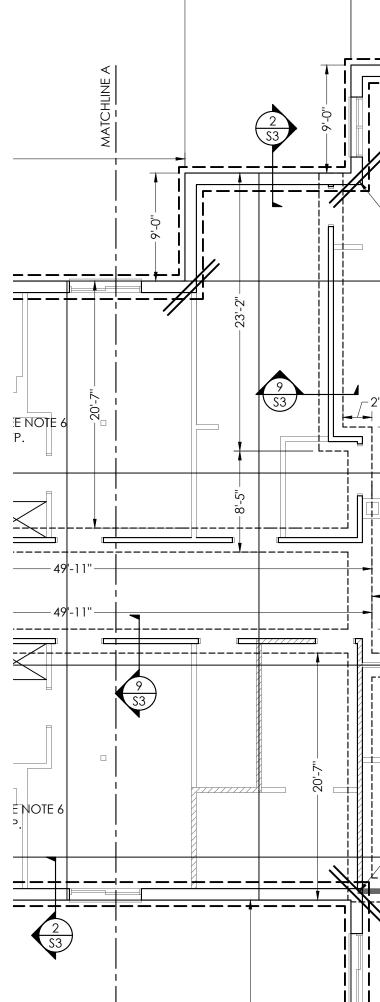
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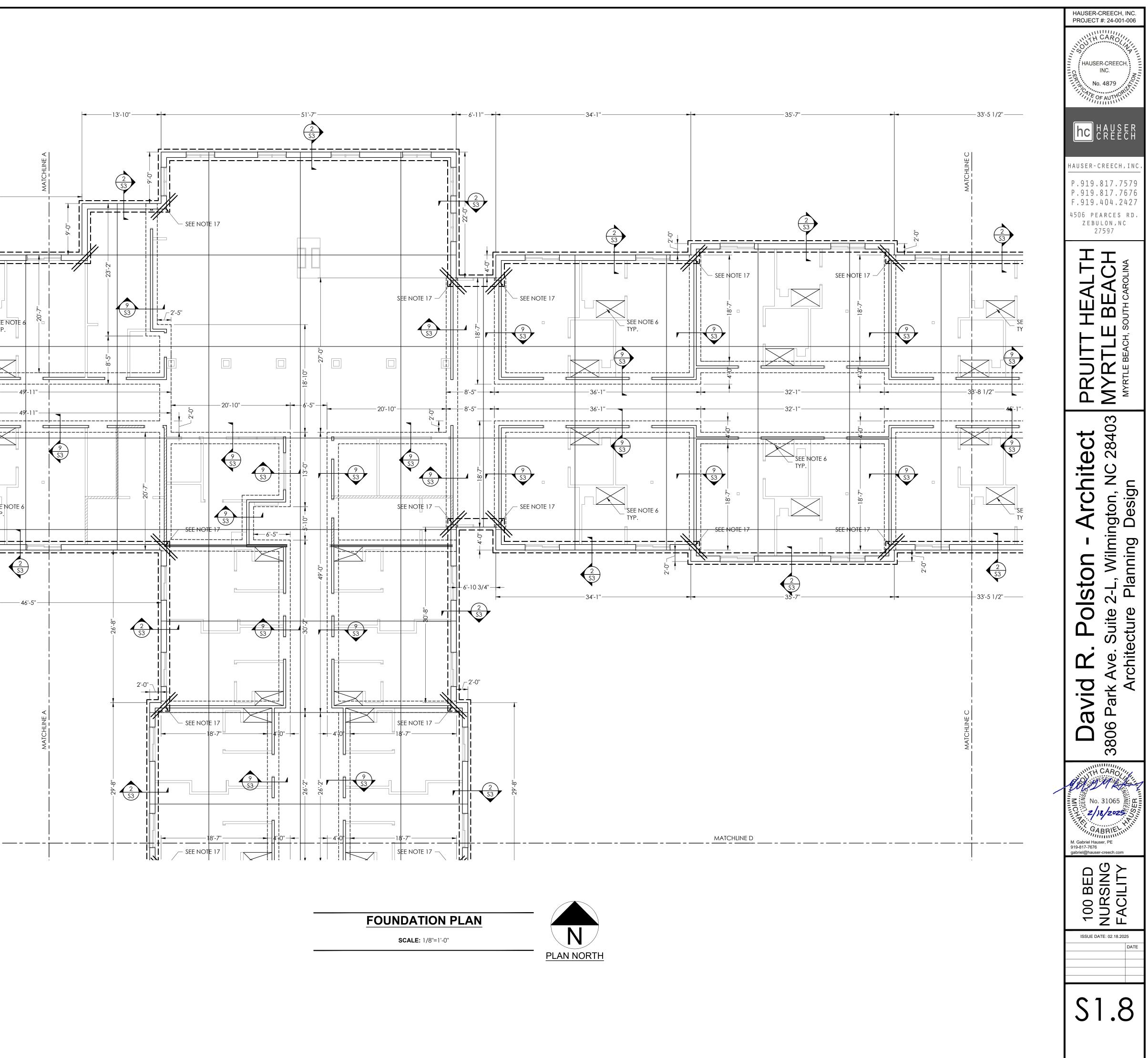
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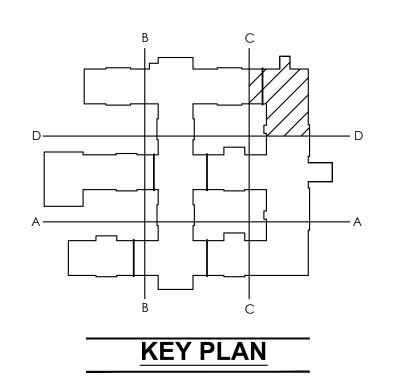


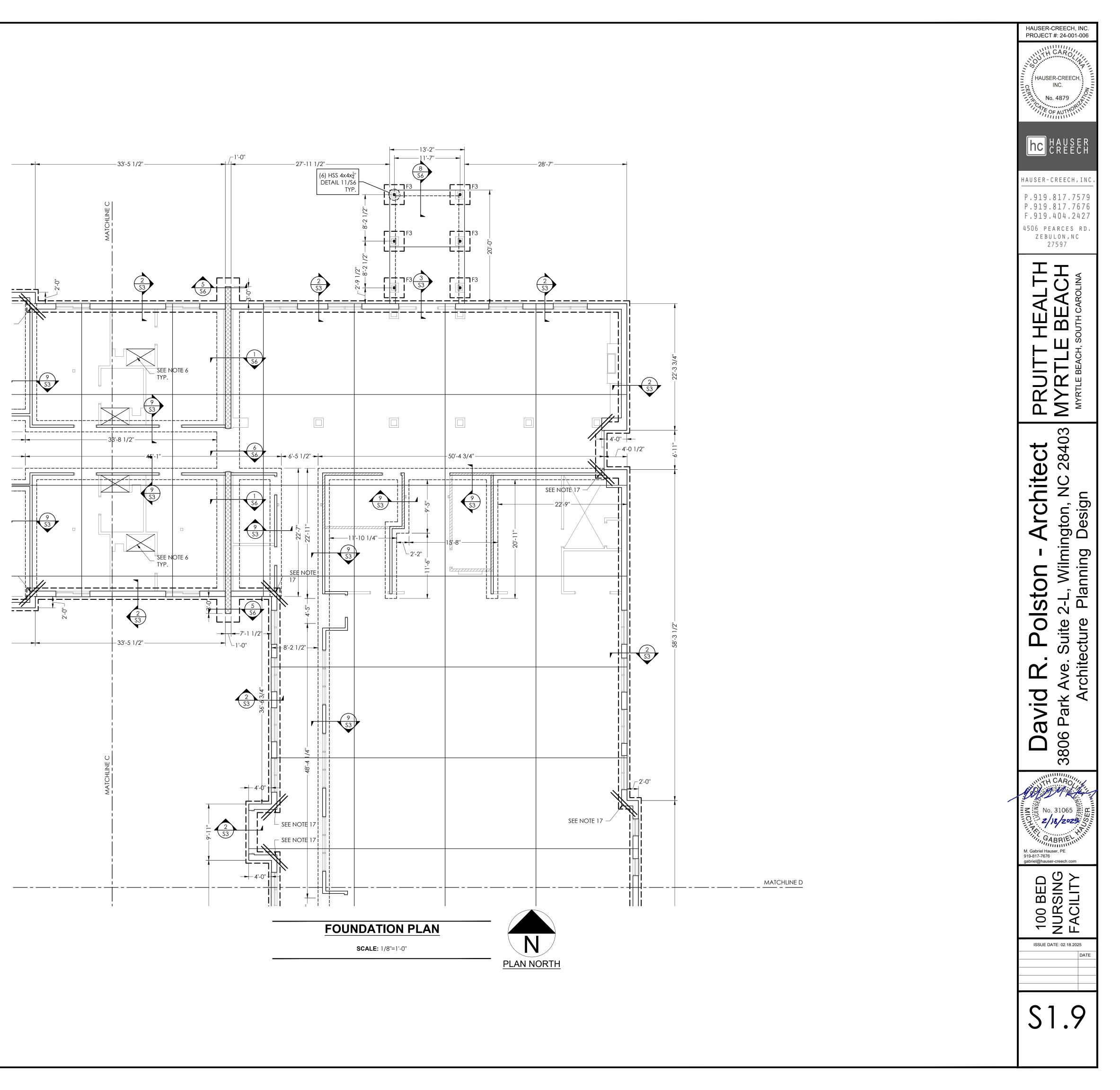


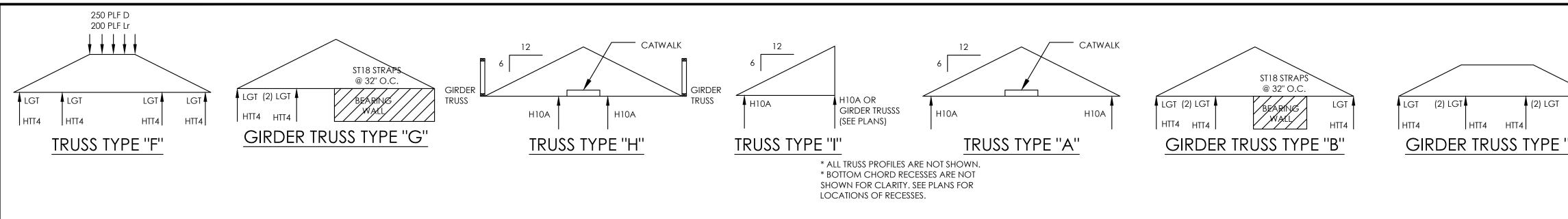
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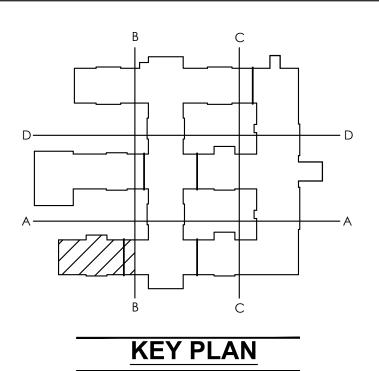




	HEADER AND BEAM SCHEDULE			
TYPE	SIZE	NOTES		
Н1	(2) 2x8	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S5		
H2	(2) 2x10	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S5		
H3	(2) 2x12	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S5		
H4	(3) 2x8	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S5		
H5	(3) 2x10	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S5		
H6	(3) 2x12	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S5		
H7	(2) 1 3/4" x 11 7/8" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0, STRAP EACH FACE OF HEADER TO FACE OF JACK STUD W/ CS16 24" LONG. PROVIDE HTT4 HOLDDOWN AT STUD BASE.		
H8	(3) 1 3/4" x 11 7/8" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0		
H9	(2) 1 3/4" x 14" LVL FLUSH BEAM. BOTTOM OF BEAM FLUSH WITH BOTTOM OF ROOF TRUSSES	Fb=2850 PSI, E=2.0, STRAP ENDS OF BEAM TO STUD GROUP BELOW W/ (2) H6 TIES. PROVIDE HTT4 HOLDDOWN AT STUD BASE.		
H10	(2) 1 3/4" x 9 1/4" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0		
H11	(3) 1 3/4" x 9 1/4" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0		
B1	W8x18 STEEL BEAM. T.O.S = 8'-10 1/2" AFF			
B2	W8x28 STEEL BEAM. T.O.S = 8'-10 1/2'' AFF, U.N.O.			
В3	W8x40 STEEL BEAM. T.O.S = 8'-10 1/2" AFF			
B4	W12x50 STEEL BEAM. T.O.S = 8'-10 1/2" AFF			
B5	W16x57 STEEL BEAM. T.O.S = 15'-0'' AFF			
B6	(2) 8" DEEP BOND BEAMS	PROVIDE (2) #5 CONTINUOUS BARS IN EACH BOND BEAM		

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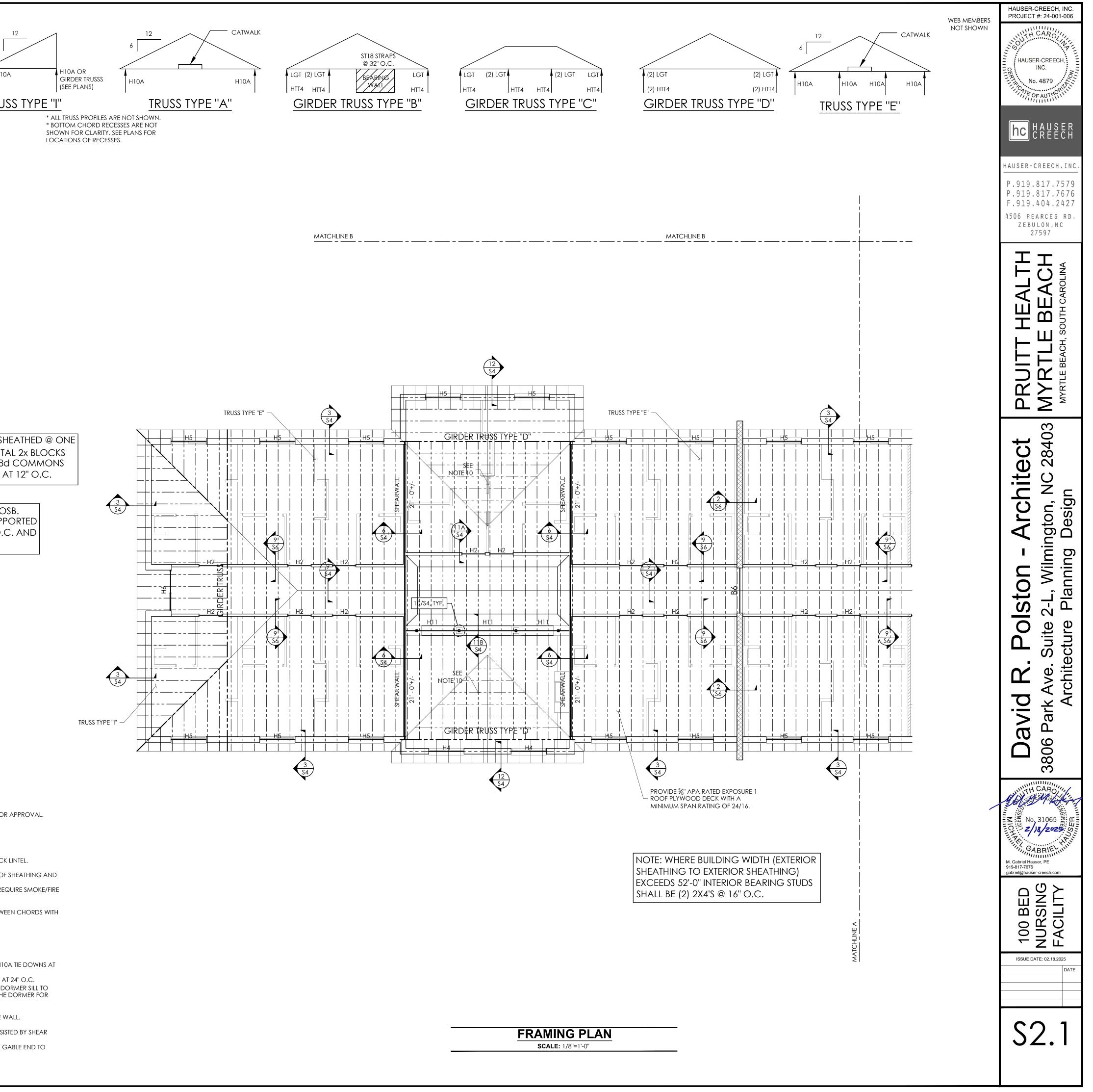
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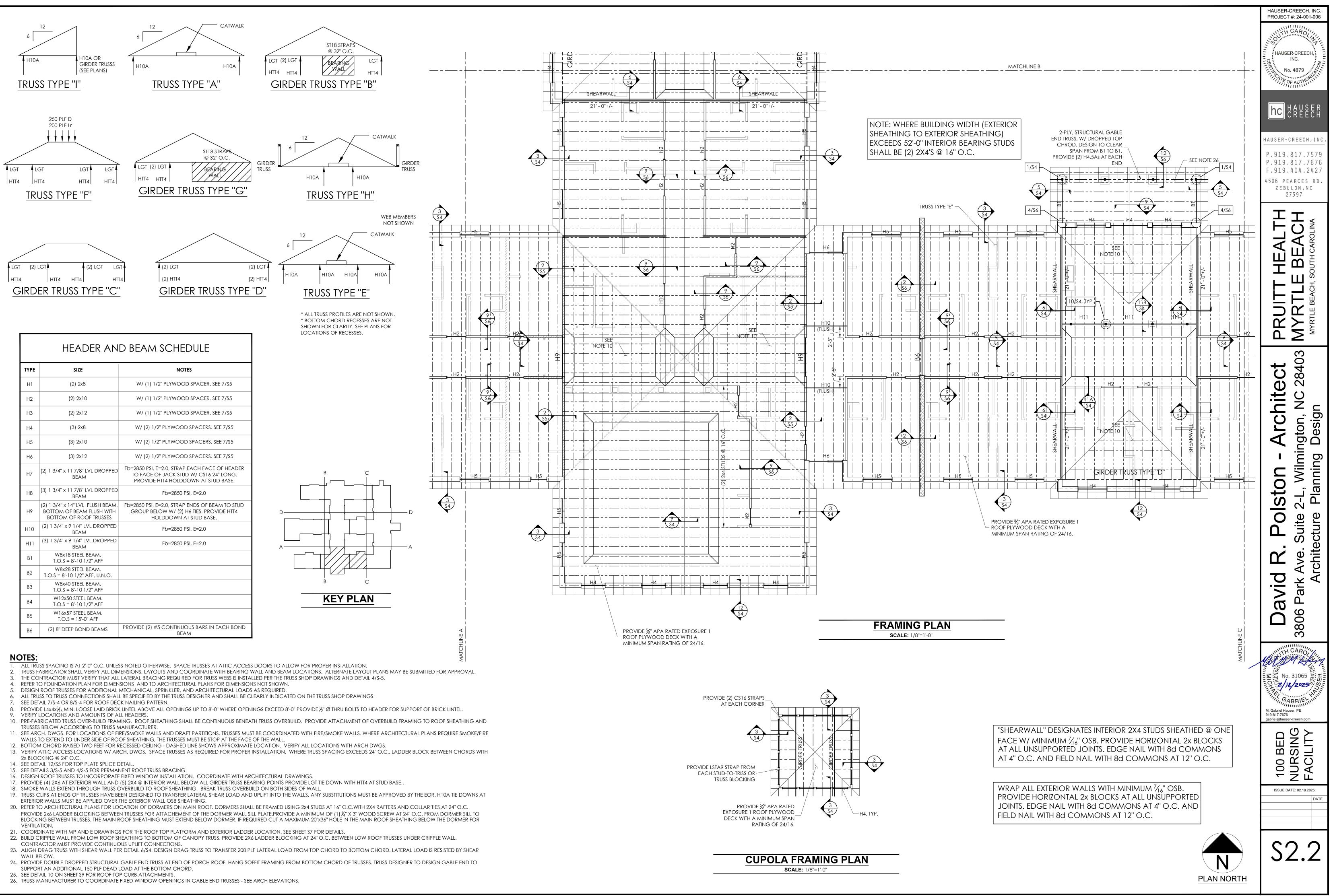


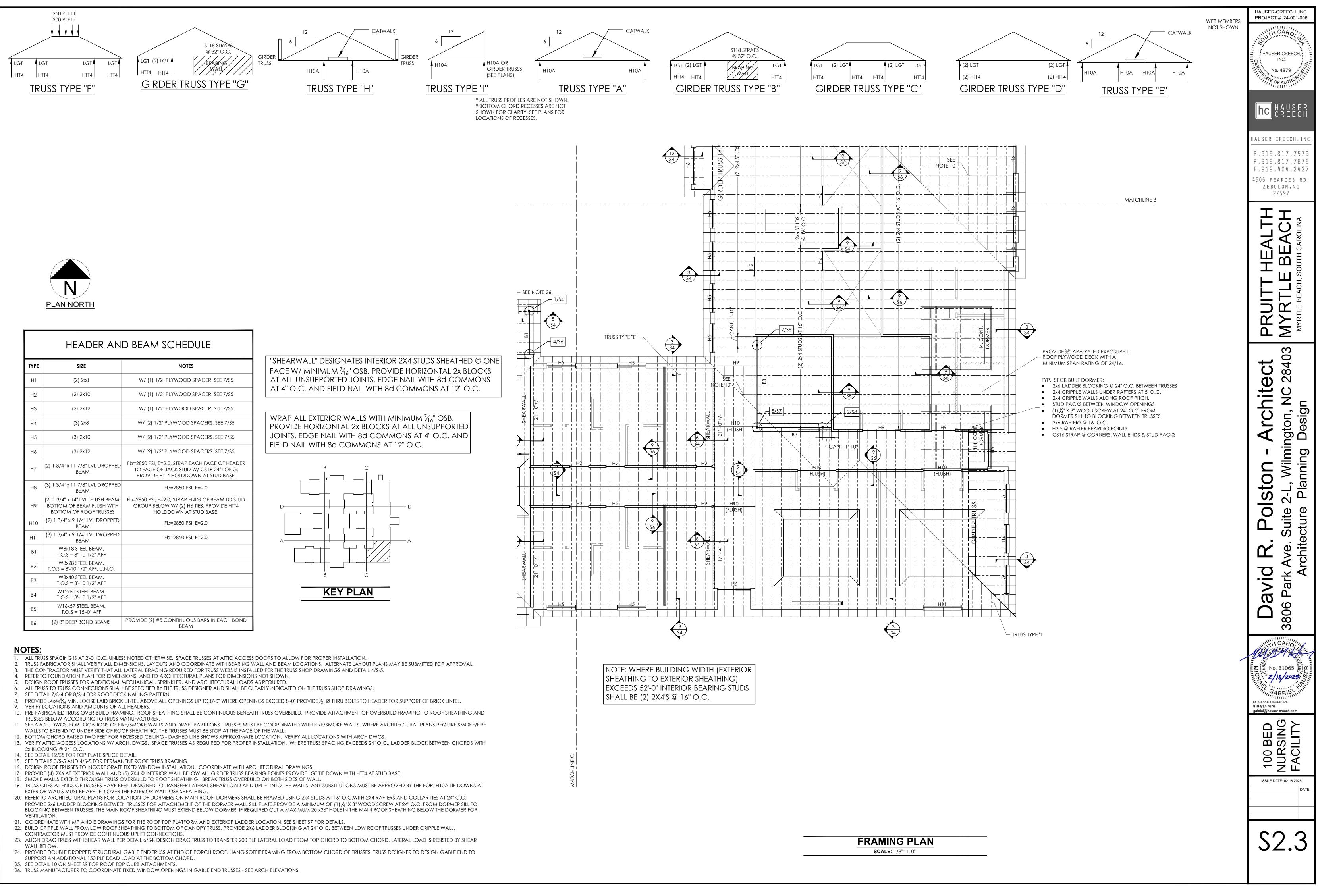
NOTES:

- . ALL TRUSS SPACING IS AT 2'-0" O.C. UNLESS NOTED OTHERWISE. SPACE TRUSSES AT ATTIC ACCESS DOORS TO ALLOW FOR PROPER INSTALLATION.
- TRUSS FABRICATOR SHALL VERIFY ALL DIMENSIONS, LAYOUTS AND COORDINATE WITH BEARING WALL AND BEAM LOCATIONS. ALTERNATE LAYOUT PLANS MAY BE SUBMITTED FOR APPROVAL. 3. THE CONTRACTOR MUST VERIFY THAT ALL LATERAL BRACING REQUIRED FOR TRUSS WEBS IS INSTALLED PER THE TRUSS SHOP DRAWINGS AND DETAIL 4/S-5.
- 4. REFER TO FOUNDATION PLAN FOR DIMENSIONS AND TO ARCHITECTURAL PLANS FOR DIMENSIONS NOT SHOWN. . DESIGN ROOF TRUSSES FOR ADDITIONAL MECHANICAL, SPRINKLER, AND ARCHITECTURAL LOADS AS REQUIRED.
- . ALL TRUSS TO TRUSS CONNECTIONS SHALL BE SPECIFIED BY THE TRUSS DESIGNER AND SHALL BE CLEARLY INDICATED ON THE TRUSS SHOP DRAWINGS.
- 7. SEE DETAIL 7/S-4 OR 8/S-4 FOR ROOF DECK NAILING PATTERN.
- 8. PROVIDE L4x4x³/₄ MIN. LOOSE LAID BRICK LINTEL ABOVE ALL OPENINGS UP TO 8'-0" WHERE OPENINGS EXCEED 8'-0" PROVIDE ½" Ø THRU BOLTS TO HEADER FOR SUPPORT OF BRICK LINTEL. 9. VERIFY LOCATIONS AND AMOUNTS OF ALL HEADERS.
- 10. PRE-FABRICATED TRUSS OVER-BUILD FRAMING. ROOF SHEATHING SHALL BE CONTINUOUS BENEATH TRUSS OVERBUILD. PROVIDE ATTACHMENT OF OVERBUILD FRAMING TO ROOF SHEATHING AND TRUSSES BELOW ACCORDING TO TRUSS MANUFACTURER.
- 11. SEE ARCH. DWGS. FOR LOCATIONS OF FIRE/SMOKE WALLS AND DRAFT PARTITIONS. TRUSSES MUST BE COORDINATED WITH FIRE/SMOKE WALLS. WHERE ARCHITECTURAL PLANS REQUIRE SMOKE/FIRE WALLS TO EXTEND TO UNDER SIDE OF ROOF SHEATHING, THE TRUSSES MUST BE STOP AT THE FACE OF THE WALL.
- 12. BOTTOM CHORD RAISED TWO FEET FOR RECESSED CEILING DASHED LINE SHOWS APPROXIMATE LOCATION. VERIFY ALL LOCATIONS WITH ARCH DWGS. 13. VERIFY ATTIC ACCESS LOCATIONS W/ ARCH. DWGS. SPACE TRUSSES AS REQUIRED FOR PROPER INSTALLATION. WHERE TRUSS SPACING EXCEEDS 24" O.C., LADDER BLOCK BETWEEN CHORDS WITH 2x BLOCKING @ 24" O.C.
- 14. SEE DETAIL 12/S5 FOR TOP PLATE SPLICE DETAIL.
- 15. SEE DETAILS 3/S-5 AND 4/S-5 FOR PERMANENT ROOF TRUSS BRACING.
- 16. DESIGN ROOF TRUSSES TO INCORPORATE FIXED WINDOW INSTALLATION. COORDINATE WITH ARCHITECTURAL DRAWINGS.
- 17. PROVIDE (4) 2X6 AT EXTERIOR WALL AND (5) 2X4 @ INTERIOR WALL BELOW ALL GIRDER TRUSS BEARING POINTS PROVIDE LGT TIE DOWN WITH HTT4 AT STUD BASE ... 18. SMOKE WALLS EXTEND THROUGH TRUSS OVERBUILD TO ROOF SHEATHING. BREAK TRUSS OVERBUILD ON BOTH SIDES OF WALL.
- 19. TRUSS CLIPS AT ENDS OF TRUSSES HAVE BEEN DESIGNED TO TRANSFER LATERAL SHEAR LOAD AND UPLIFT INTO THE WALLS. ANY SUBSTITUTIONS MUST BE APPROVED BY THE EOR. H10A TIE DOWNS AT
- EXTERIOR WALLS MUST BE APPLIED OVER THE EXTERIOR WALL OSB SHEATHING. 20. REFER TO ARCHITECTURAL PLANS FOR LOCATION OF DORMERS ON MAIN ROOF. DORMERS SHALL BE FRAMED USING 2x4 STUDS AT 16" O.C. WITH 2X4 RAFTERS AND COLLAR TIES AT 24" O.C. PROVIDE 2x6 LADDER BLOCKING BETWEEN TRUSSES FOR ATTACHEMENT OF THE DORMER WALL SILL PLATE. PROVIDE A MINIMUM OF (1) 1/4" X 3" WOOD SCREW AT 24" O.C. FROM DORMER SILL TO BLOCKING BETWEEN TRUSSES. THE MAIN ROOF SHEATHING MUST EXTEND BELOW DORMER. IF REQUIRED CUT A MAXIMUM 20"x36" HOLE IN THE MAIN ROOF SHEATHING BELOW THE DORMER FOR VENTILATION.
- 21. COORDINATE WITH MP AND E DRAWINGS FOR THE ROOF TOP PLATFORM AND EXTERIOR LADDER LOCATION. SEE SHEET S7 FOR DETAILS.
- 22. BUILD CRIPPLE WALL FROM LOW ROOF SHEATHING TO BOTTOM OF CANOPY TRUSS, PROVIDE 2X6 LADDER BLOCKING AT 24" O.C. BETWEEN LOW ROOF TRUSSES UNDER CRIPPLE WALL. CONTRACTOR MUST PROVIDE CONTINUOUS UPLIFT CONNECTIONS. 23. ALIGN DRAG TRUSS WITH SHEAR WALL PER DETAIL 6/S4. DESIGN DRAG TRUSS TO TRANSFER 200 PLF LATERAL LOAD FROM TOP CHORD TO BOTTOM CHORD. LATERAL LOAD IS RESISTED BY SHEAR
- WALL BELOW. 24. PROVIDE DOUBLE DROPPED STRUCTURAL GABLE END TRUSS AT END OF PORCH ROOF. HANG SOFFIT FRAMING FROM BOTTOM CHORD OF TRUSSES. TRUSS DESIGNER TO DESIGN GABLE END TO SUPPORT AN ADDITIONAL 150 PLF DEAD LOAD AT THE BOTTOM CHORD.
- 25. SEE DETAIL 10 ON SHEET S9 FOR ROOF TOP CURB ATTACHMENTS. 26. TRUSS MANUFACTURER TO COORDINATE FIXED WINDOW OPENINGS IN GABLE END TRUSSES - SEE ARCH ELEVATIONS.

MATCHLINE B

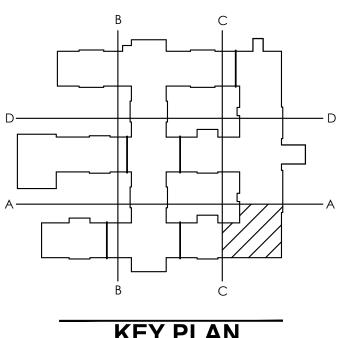


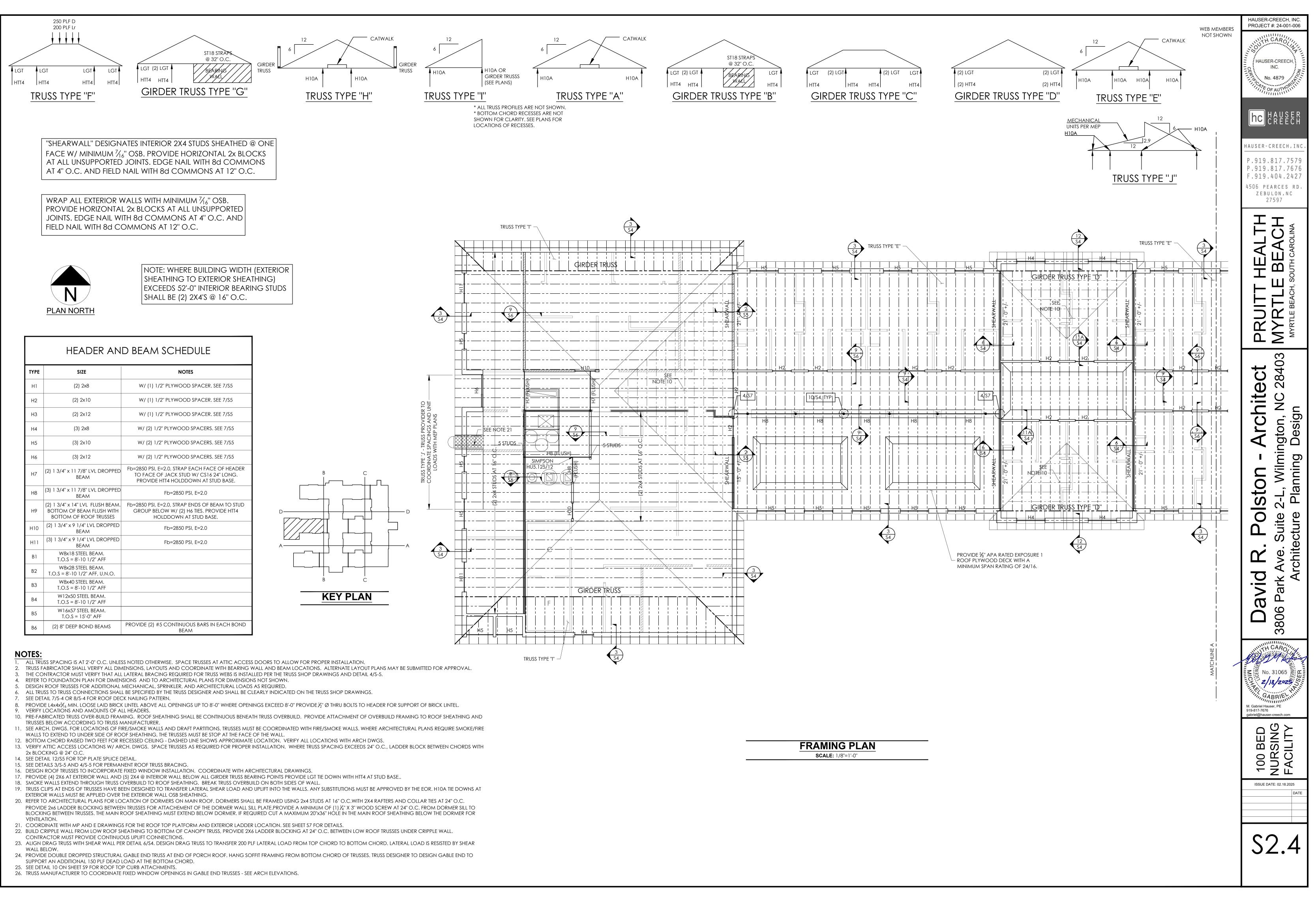


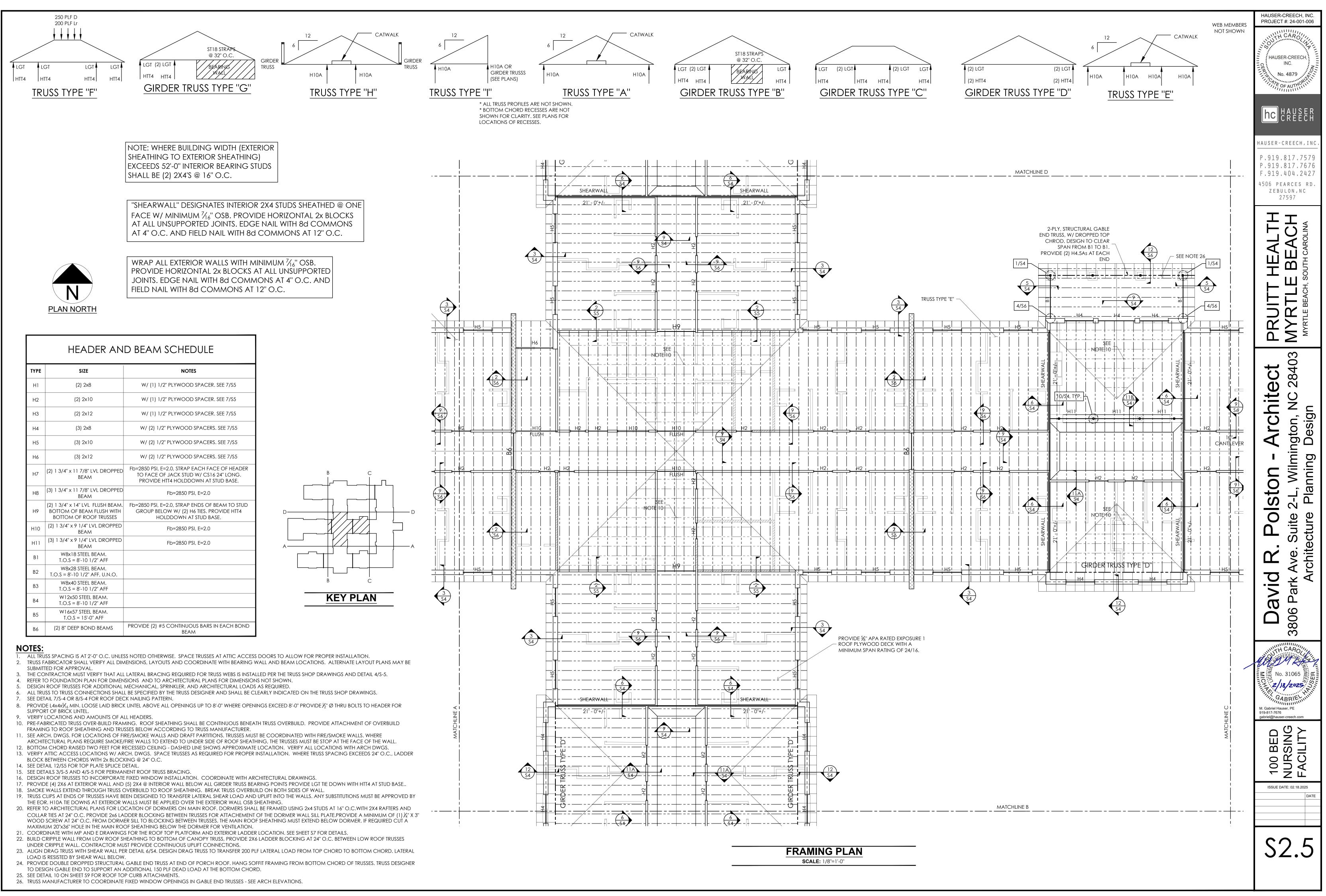




	HEADER AND BEAM SCHEDULE			
TYPE	SIZE	NOTES		
HI	(2) 2x8	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S5		
H2	(2) 2x10	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S5		
H3	(2) 2x12	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S5		
H4	(3) 2x8	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S5		
H5	(3) 2x10	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S5		
H6	(3) 2x12	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S5		
H7	(2) 1 3/4" x 11 7/8" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0, STRAP EACH FACE OF HEADER TO FACE OF JACK STUD W/ CS16 24" LONG. PROVIDE HTT4 HOLDDOWN AT STUD BASE.		
H8	(3) 1 3/4" x 11 7/8" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0		
Н9	(2) 1 3/4" x 14" LVL FLUSH BEAM. BOTTOM OF BEAM FLUSH WITH BOTTOM OF ROOF TRUSSES	Fb=2850 PSI, E=2.0, STRAP ENDS OF BEAM TO STUD GROUP BELOW W/ (2) H6 TIES. PROVIDE HTT4 HOLDDOWN AT STUD BASE.		
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Н11	(3) 1 3/4" x 9 1/4" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0		
B1	W8x18 STEEL BEAM. T.O.S = 8'-10 1/2" AFF			
B2	W8x28 STEEL BEAM. T.O.S = 8'-10 1/2'' AFF, U.N.O.			
В3	W8x40 STEEL BEAM. T.O.S = 8'-10 1/2" AFF			
B4	W12x50 STEEL BEAM. T.O.S = 8'-10 1/2" AFF			
B5	W16x57 STEEL BEAM. T.O.S = 15'-0'' AFF			
B6	(2) 8" DEEP BOND BEAMS	PROVIDE (2) #5 CONTINUOUS BARS IN EACH BOND BEAM		

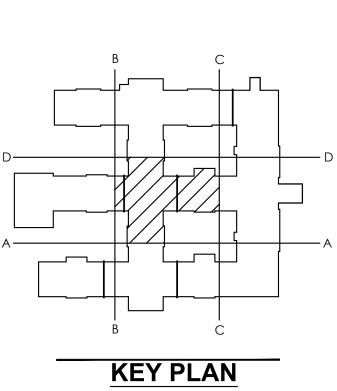


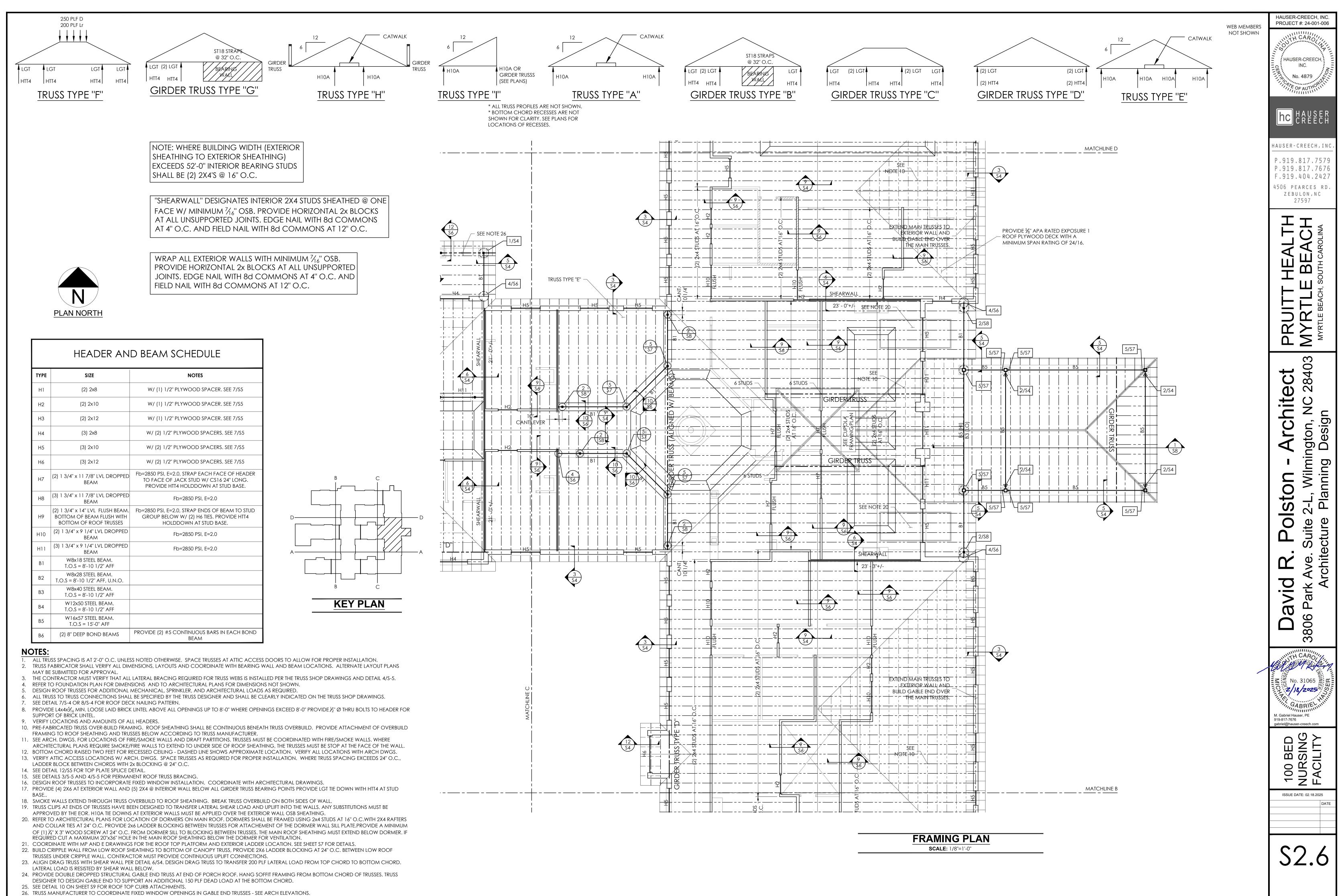


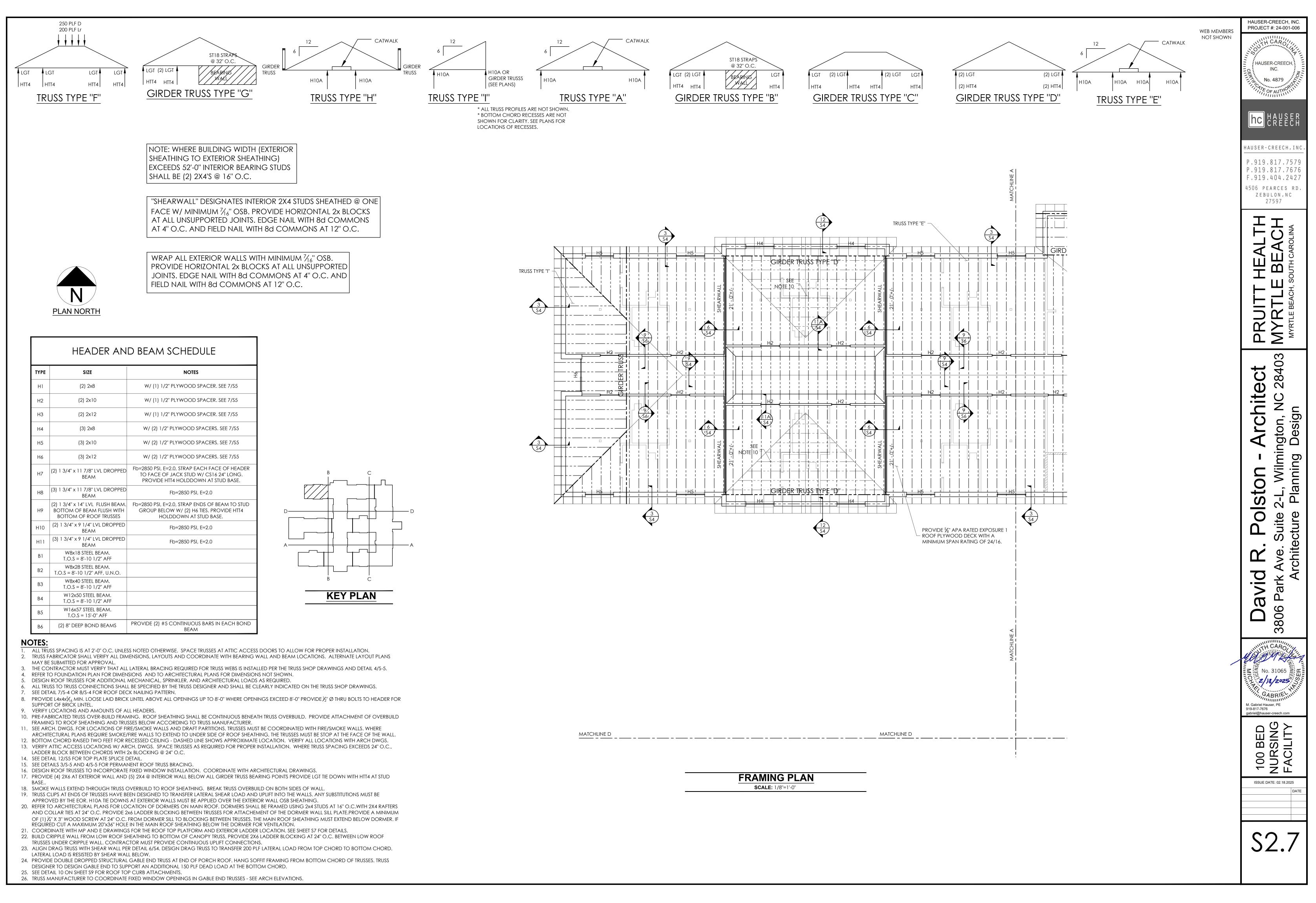




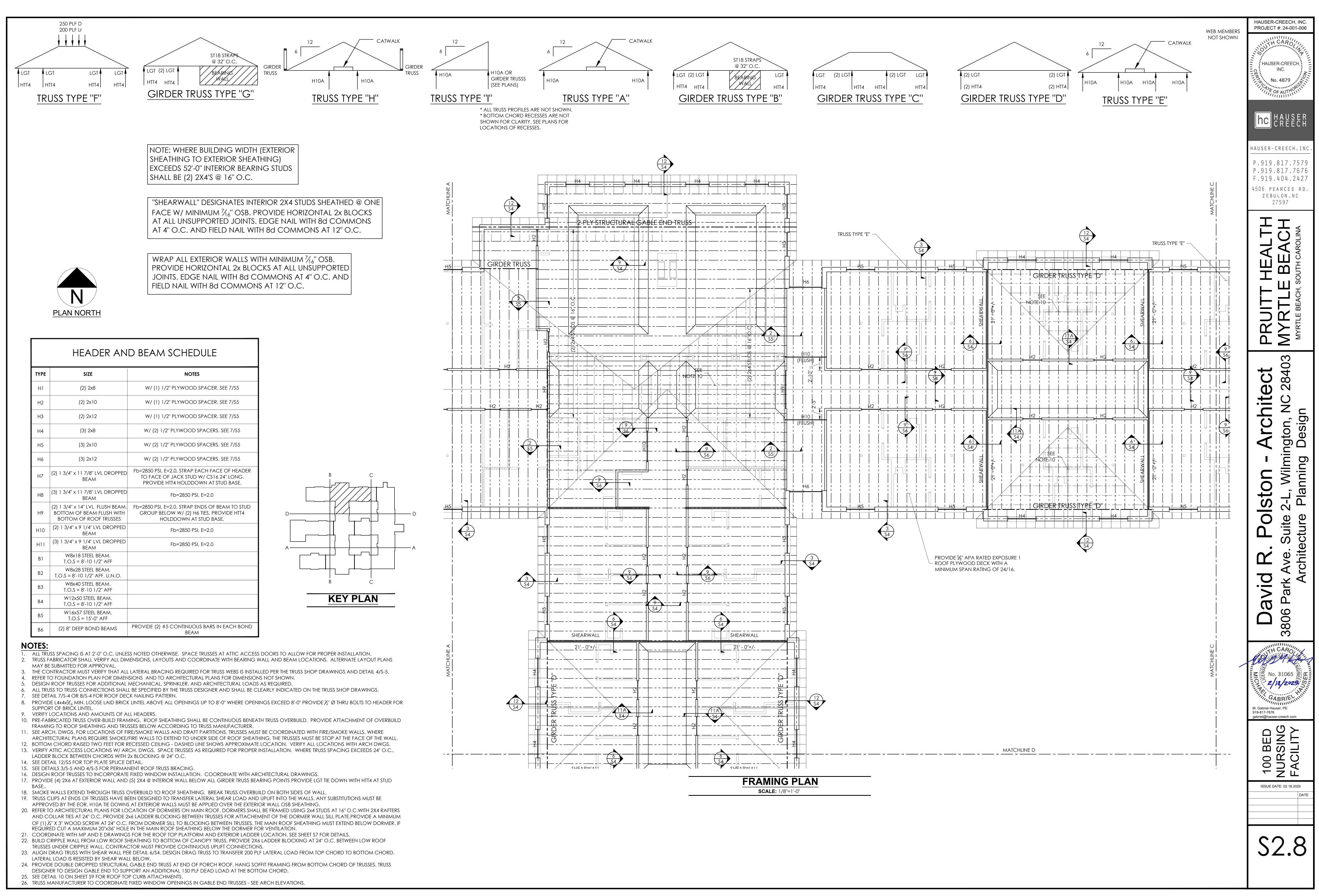
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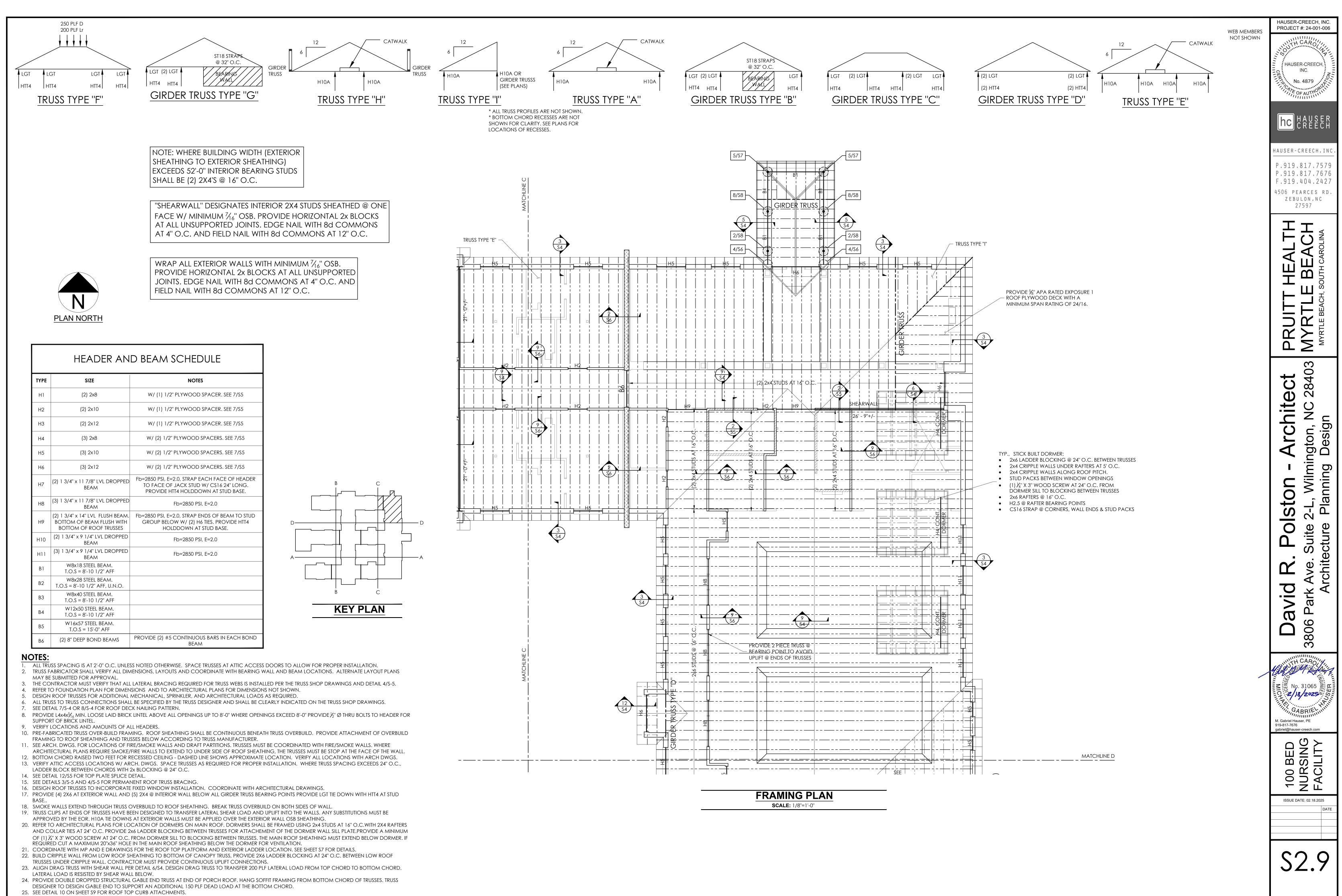


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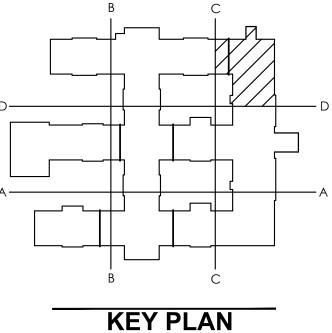


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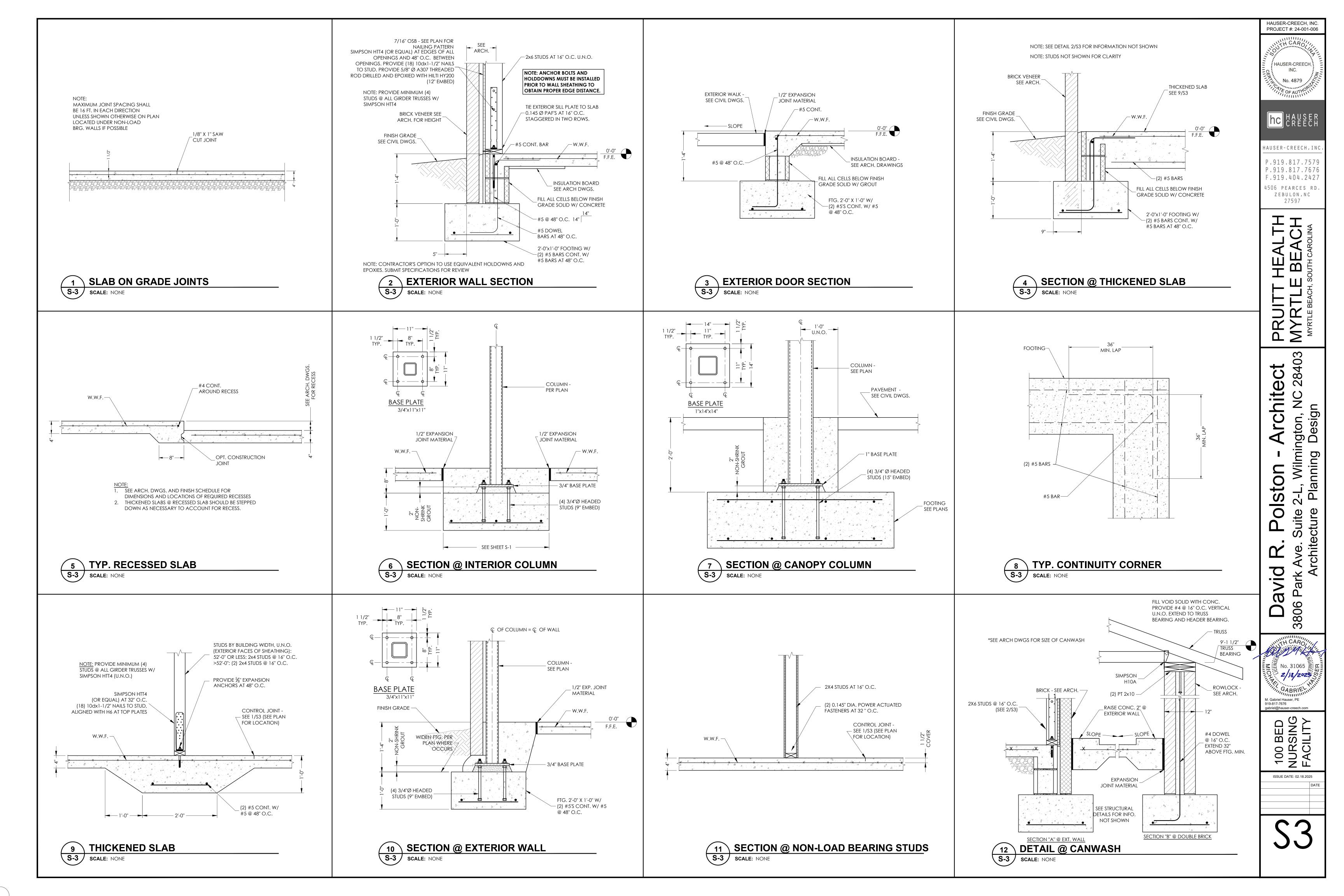


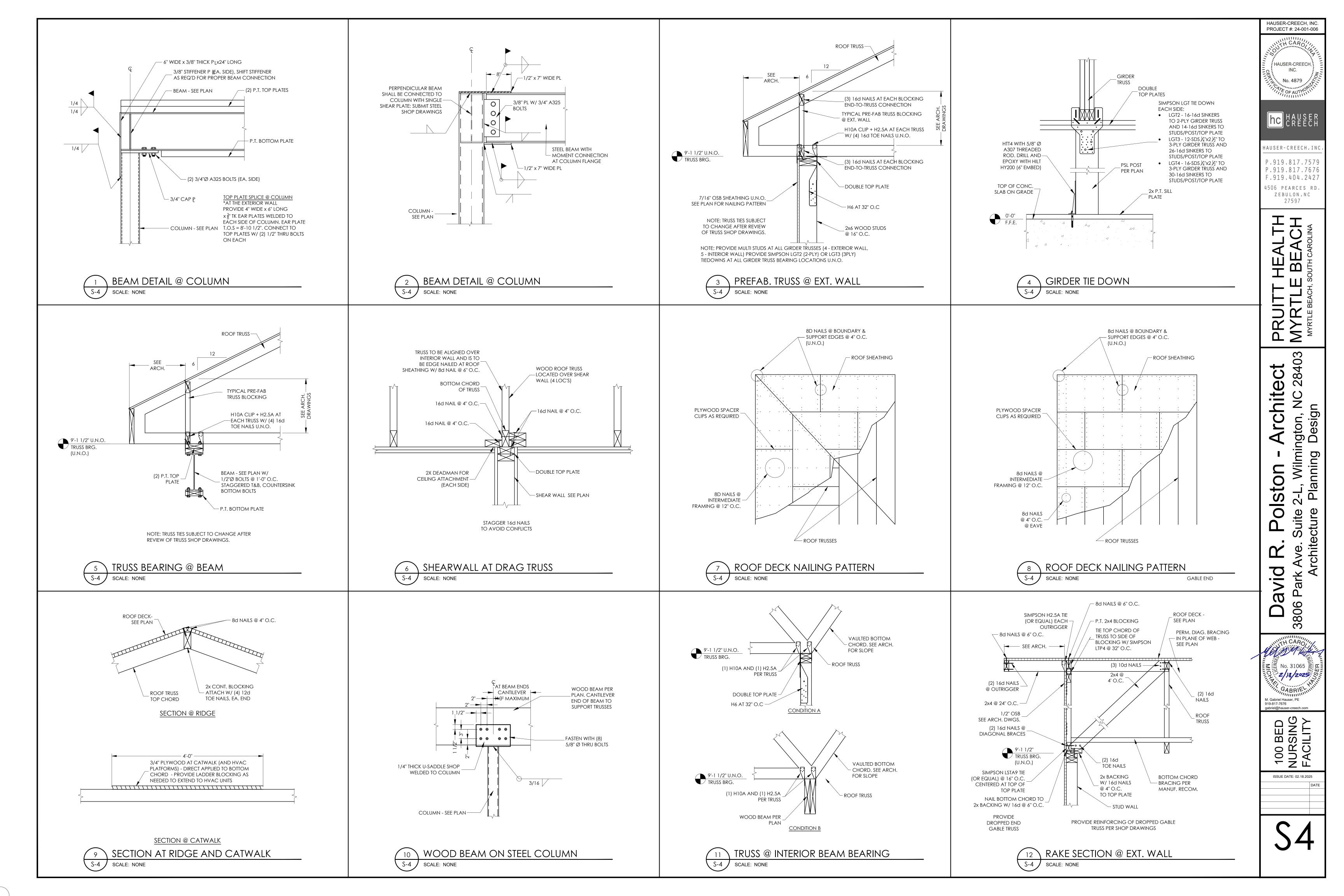


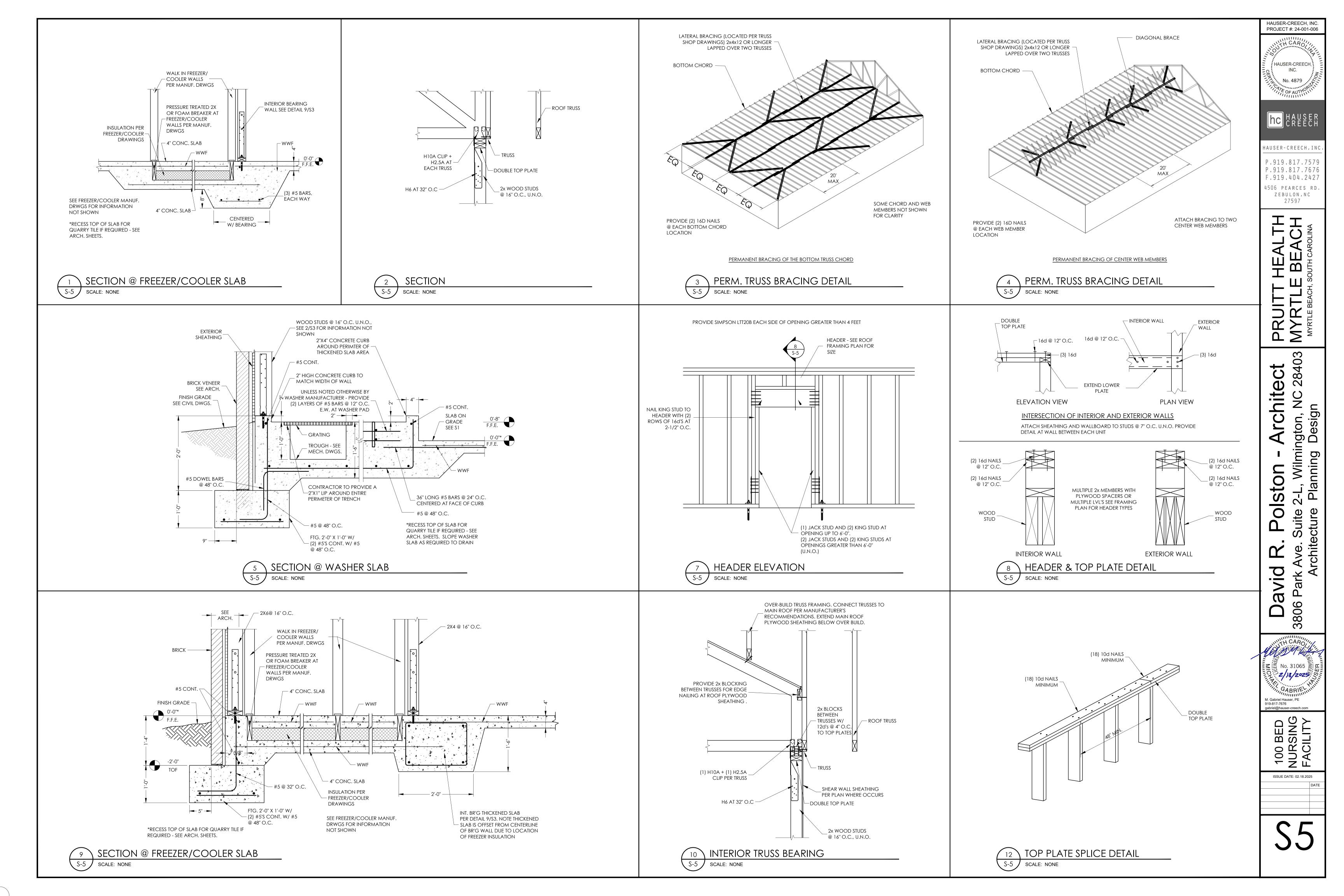
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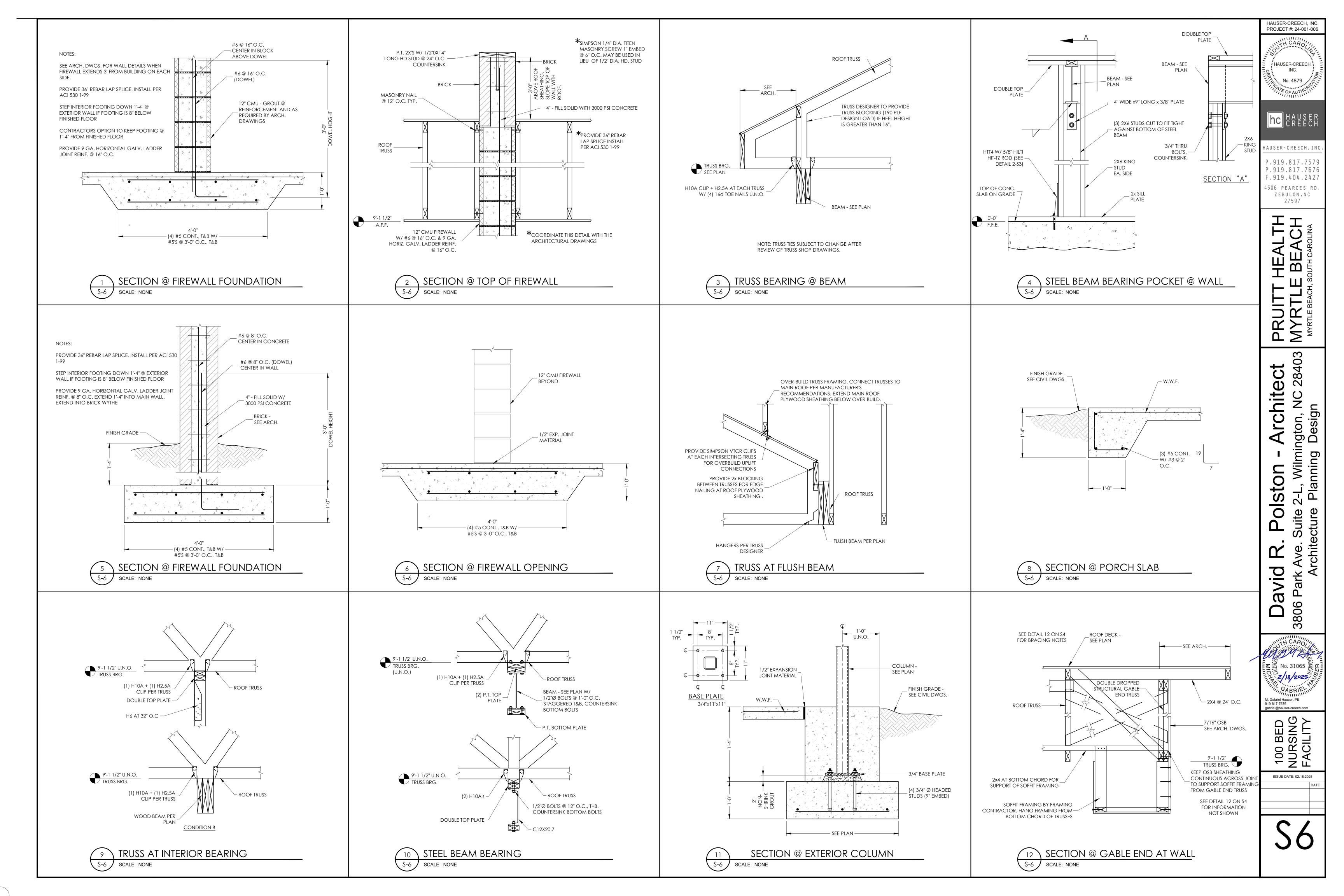


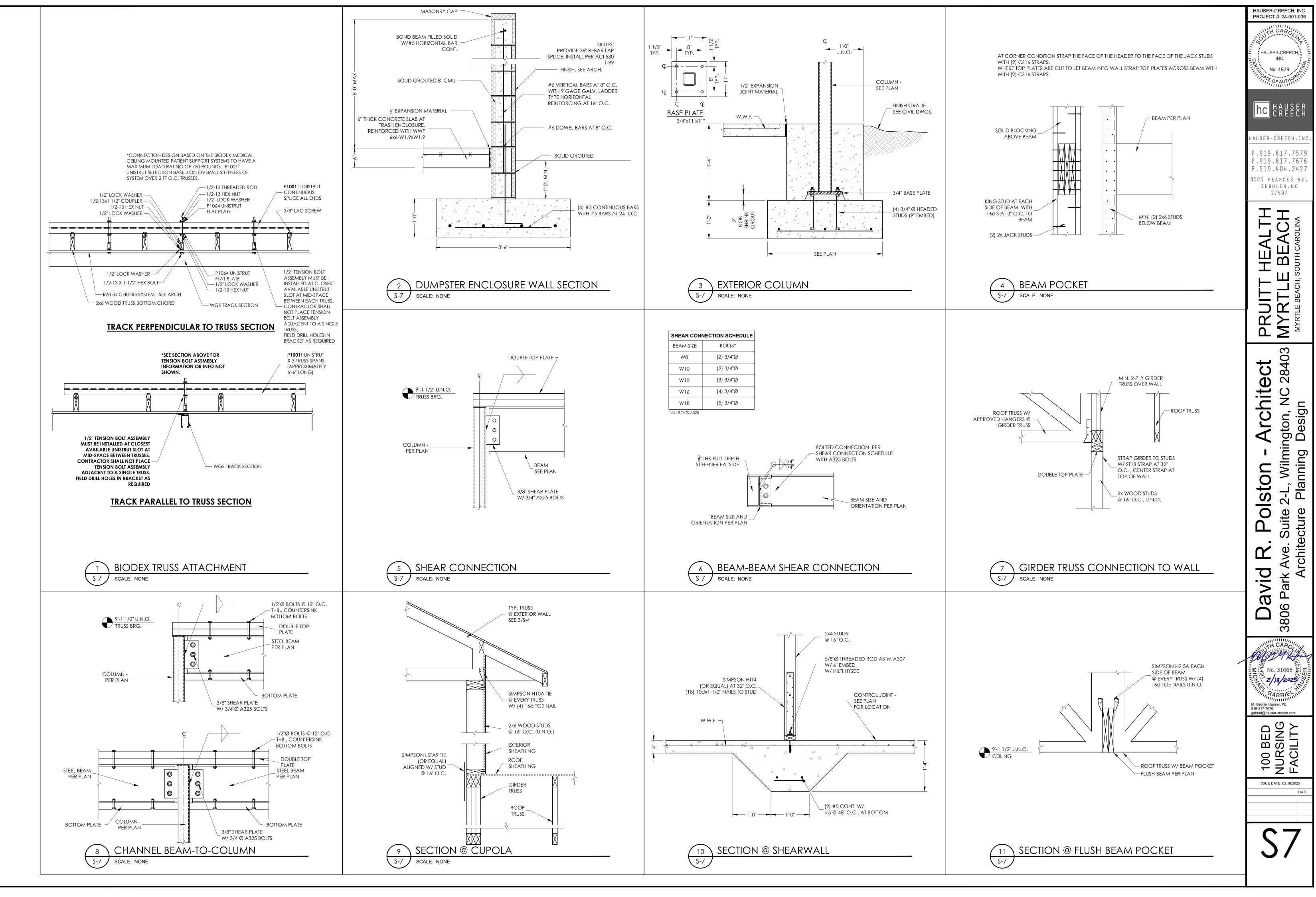
26. TRUSS MANUFACTURER TO COORDINATE FIXED WINDOW OPENINGS IN GABLE END TRUSSES - SEE ARCH ELEVATIONS.

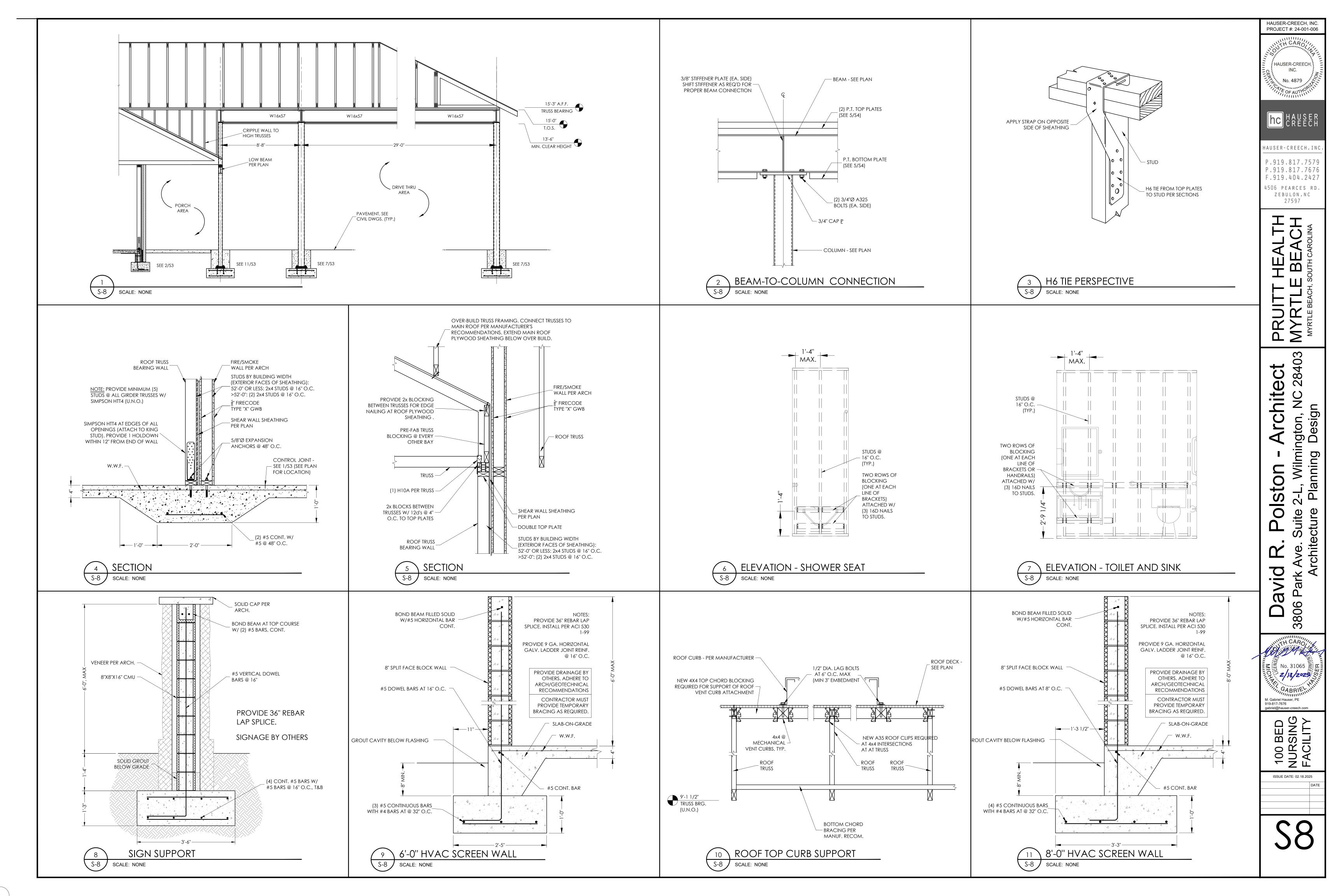


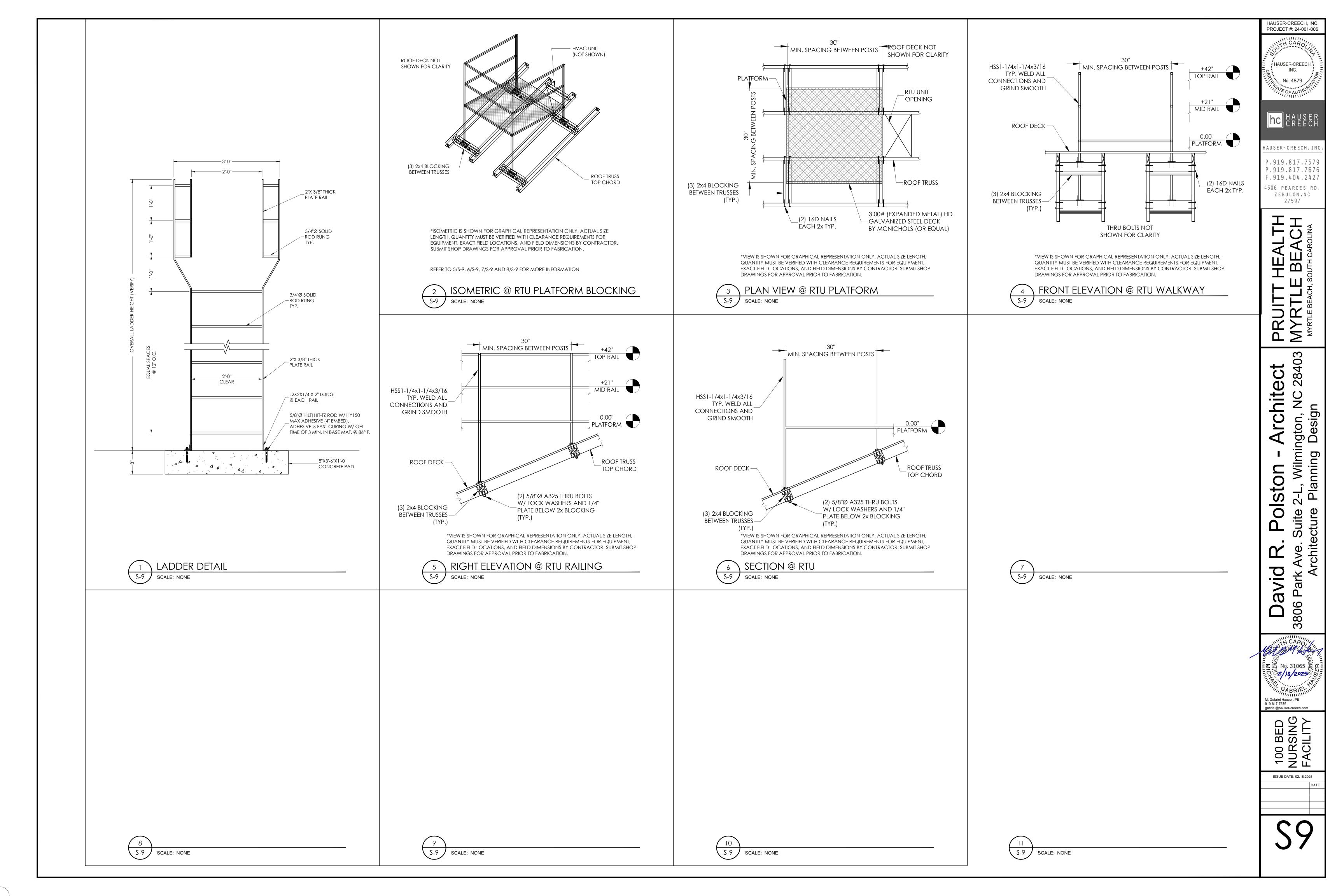












RISK CATEGORY	III (ASCE 7-16)
IMPORTANCE FACTORS:	
l seismic	1.25
l snow	1.10
LIVE LOADS:	
ROOF	20 psf
CATWALK	40 pst
FLOOR	100 psf
SNOW LOAD:	
Pg	5 psf
WIND LOAD:	
Basic Wind Speed	156 MPH
Basic Wind Speed Exposure Category	C
Wind Base Shear (M/W/ERS)	
Vx Vy	71.0 K
Vy	101.3 K
SEISMIC LOAD:	
Spectral Response	
Ss	0.305
\$1	0.112
Sds	0.316
Sdl	0.177
Seismic Design Category	
Seismic Site Class	D (Default)
Structural System	Light framed walls sheathed w/ structural panel
R-Factor	6.5
Analysis Procedure Seismic Base Shear	Equivalent Lateral Force
	1700 /
Vx Vy	170.0 K 192.5 K
SEISMIC ANCHORAGE OF NON-SI	
	on-structural components are required to be
braced against seismic swo	
LATERAL DESIGN CONTROL:	
X-Direction	Wind
Y-Direction	Wind

SCHEDULE OF SPECIAL INSPECTIONS:

Project Name: Pruitt Health Myrtle Beach

Construction divisions which require inspections for this project are as follows:

INSPECTION TASK CONTI OR PE INSP			(P)	SPECIAL INSPECTIONS FIRM	NOTES & SCOPE	
		С	P			
1. VERIFICATION OF SOILS (Table 1704.7)						
Verify materials below shallow Foundatio adequate to achieve the design bearing capacity.			Р	Testing Agency (TA)	Testing Agency shall provide soils report	
Verify excavations are extended to prop depth.	er		Р	Testing Agency (TA)		
Perform Classification and testing of compacted fill materials.			P	Testing Agency (TA)		
Verify use of proper materials, densities and lift thickness during placement and compaction of compacted fill.				Testing Agency (TA)		
Prior to placement of compacted fill, observe sub-grade and verify that site has been prepared properly.			P	Testing Agency (TA)		
2. REINFORCED CONCRETE (Table 1704.4)						
Inspection of reinforcing steel, including prestressing tendons, and placement. AC 318:3.5, 7.1-7.7	21		Р	Testing Agency (TA)	ACI 318: 3.5,7.1-7.7 IBC: 1913.4	
Verifying use of required design mix: ACI Ch. 4, 5.2-5.4	318:		P	Testing Agency (TA)	ACI 318: Ch. 4, 5.2-5.4 IBC: 1904.2.2, 1913.2, 1913.3	
At the time fresh concrete is sampled to fabricate specimens for strength tests, slu air content, and temperature of concrete		С		Testing Agency (TA)	ASTM C 172, C 31 ACI: 318: 5.6, 5.8 IBC: 1913.10	
2. REINFORCED CONCRETE (Table 1704.4)						
InspectT OSB nailing patterns per structur plans. Inspect roof truss and top plate tie holddowns, and anchorage per structure plans	es,		P	Special Inspector (SI)		

STATEMENT OF SPECIAL INSPECTIONS:

Project Name: Pruitt Health Myrtle Beach Building Permit Number:

Project Address: Myrtle Beach, South Carolina

The following information is being submitted in accordance with the Special Inspection provisions of the International Building Code. Attached is the Schedule of Special Inspections (SSI) required for this project.

The Special Inspection program outlined herein does not relieve the Contractor or any other entity of contractual duties, including quality control, quality assurance or safety. The contractor is soley responsible for construction means, methods and job site safety.

Respectfully submitted, The Structural Engineer of Record

SCHEDULE OF SPECIAL INSPECTIONS (Continued):

Project Name: Pruitt Health Myrtle Beach

Construction divisions which require inspections for this project are as follows:

INSPECTION TASK	CONTINUOUS (C) OR PERIODIC (P) INSPECTIONS		SPECIAL INSPECTIONS FIRM	NOTES & SCOPE	
		С	Р		
3. STRUCTURAL STEEL (Table 1704.3)		•			
Material verification of high strength bolts and washers.	, nuts		Р	Special Inspector (SI)	AISC 360, A3.3
nspection of high strength bolting, snug t pints	ight		Р	Special Inspector (SI)	AISC 360, M2.5 IBC 1704.3.3
Material verification of structural steel.			Р	Special Inspector (SI)	Fabricator's bill of materials verification is acceptable.
All field welding.			Р	Special Inspector (SI)	AWS D1.1 IBC 1704.3.1
4. RETAINING WALLS (Table 1704.12)		1	•		·
nspect all retaining walls over 5 feet in he	eight.		P	Testing Agency (TA)	
5. MASONRY		1			
As masonry construction begins, the follow hall be verified to ensure compliance: (A Proportions of site mixed mortar. (B) Construction of mortar joints. (C) Location einforcement and connectors.	x)		P	Testing Agency (TA)	ACI 318: 3.5,7.1-7.7 IBC: 1913.4
The inspection program shall verify: (A) Siz ocation of structural elements. (B) Size, gr ype of reinforcement. (C) Protection of nasonry during cold weather (temperatu below 40 degrees F) or hot weather temperature above 90 degrees F)	rade,		Ρ	Testing Agency (TA)	Sec. 2108.9.2.11, Item 2, Sec. 2104.3, 2104.4, ACI Sec. 1.15.4, 2.1.2, Sec, 1.12, Sec 2.1.8.6, 2.1.8.6.2, ACI 3.3G, Art 2.4,3.4, Art 1.8
Prior to grouting, the following shall be ver o ensure compliance: (A) Grout space is clean. (B) Placement of reinforcement ar connectors. (C) Proportions of site-prepar grout. (D) Construction of mortar joints	nd		Р	Testing Agency (TA)	Sec. 1.12, Art. 3.2D, Art 3.4, Art. 2.6B, Art. 3.3B
Grout Placement shall be verified to ensu compliance with code and construction provisions.	re		P	Testing Agency (TA)	Art. 3.5

REINFORCED CONCRETE:

- 1. ALL CONCRETE WORK SHALL CONFORM TO THE "BUILDING CODE REQUIREMENTS I CONCRETE," (ACI 318, 14)
- 2. REINFORCING STEEL SHALL BE DEFORMED BARS ASTM A-615 (GRADE 60)
- 3. THE COMPRESSIVE STRENGTH AT 28 DAYS OF CAST-IN-PLACE CONCRETE SHALL BE (SEE CIVIL DRAWINGS FOR SITE CONCRETE) KEEP COPY OF CONC. TEST REPORT TIMES.
- 4. LAP SPLICES FOR #5 REINFORCING BARS SHALL BE 24" MIN., U.N.O.

5. CLEAR CONCRETE COVER FOR REINFORCING STEEL: MASONRY WALLS: LOCATE IN CENTER OF WALL (U.N.O.) 2" FORMED EDGES FOOTINGS: 3" CAST AGAINST GROUND SLAB ON GRADE: MID-HEIGHT OF SLAB

6. THE LONGITUDINAL REINFORCING STEEL IN WALLS AND FOOTINGS SHALL AROUND CORNERS. SEE TYPICAL DETAILS.

7. ALL CONCRETE SHALL BE VIBRATED BY MECHANICAL VIBRATORS.

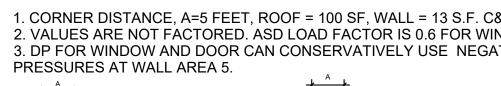
STRUCTURAL STEEL:

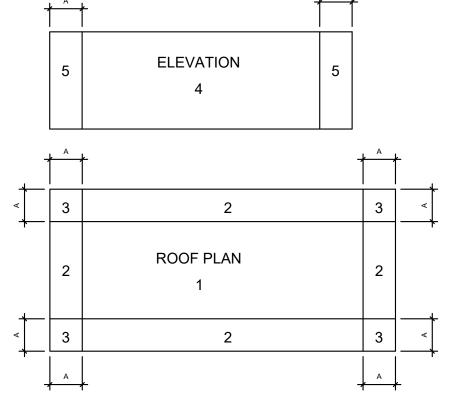
- 1. ALL STRUCTURAL STEEL WORK SHALL CONFORM TO THE A.I.S.C. "STEEL CONSTRU 360-10.
- 2. STRUCTURAL STEEL SHALL BE ASTM A-992.
- 3. STRUCTURAL TUBES SHALL BE ASTM A500, GRADE B.
- 4. STEEL FRAMING CONNECTIONS SHALL BE BOLTED OR WELDED. BOLTS SHALL MINIMUM AND SHALL BE ASTM A-325-N U.N.O., SNUG TIGHT ALL CONNECTIONS.
- 5. ANCHOR BOLTS SHALL BE ASTM F1554 HEADED BOLTS. MINIMUM ANCHOR I LENGTH SHALL BE 12 BOLT DIAMETERS U.N.O. CLEAN ANCHOR BOLTS OF ALL G **BEFORE INSTALLATION.**
- 6. WELDS SHOWN ON THE STRUCTURAL DRAWINGS ARE THE MINIMUM REQ'D FABRICATOR'S DRAWINGS SHALL SHOW WELDS AND THEY SHALL CONF SPECIFICATIONS. ALL WELDING SHALL BE DONE WITH E-70 SERIES ELECTRODES.
- 7. PAINT ALL STRUCTURAL STEEL WITH ONE COAT OF RED OXIDE RUST-INHIBITIVE THICKNESS. THE COMPATABILITY OF PRIMER AND ANY TOP COAT SHALL BE VERI PAINTING IS PERFORMED. TOUCH-UP ALL EXPOSED METAL AFTER FIELD IN STRUCTURAL STEEL WHICH IS EXPOSED TO THE ELEMENTS SHALL RECEIVE TWO CO ENAMEL WHICH IS COMPATIBLE TO THE PRIMED SURFACE.
- 8. THE SHOP DRAWINGS SHALL INCLUDE COMPLETE DETAILS AND SCHEDULES FOR F ASSEMBLY OF STRUCTURAL STEEL MEMBERS. SUBMIT FOUR PRINTS OF REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS CONTRACTOR TO REVIEW AND STAMP DRAWINGS PRIOR TO SUBMISSION TO THE E

WOOD TRUSSES:

- 1. ROOF TRUSSES SHALL BE DESIGNED TO SUPPORT THE DESIGN LOADS INDICATE INFORMATION SECTION.
- 2. IN ADDITION TO THE UNIFORM LOADING SPECIFIED FOR TRUSS DESIGN, THE TRUS INCLUDE ANY CONCENTRATED LOADS CAUSED BY ARCHITECTURAL FEATUR EQUIPMENT OR MATERIALS AND BY SPRINKLER LOADS IN THE TRUSS DESIGN.
- 3. TRUSSES SHALL BE DESIGNED BY A REGISTERED ENGINEER IN THE STATE OF SOUTH SHOP DRAWINGS BEARING THE ENGINEER'S SEAL SHALL BE SUBMITTED FOR APPRO
- 4. TRUSSES SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE STANDARDS OF THE TRUSS PLATE INSTITUTE TPI I-2002.
- 5. LIMIT LL DEFLECTION TO L/360. LIMIT TL DEFLECTION TO L/240 OR 1.25" MAX.

	WIND LOAD SCHEDULE						
COMPONENTS & CLADDING	ROOF WIN	ROOF WIND LOAD					
	ROOF AREA						
	1	2	3				
PRESSURE (PSF)	+22.0	+22.0	+22.0				
SUCTION (PSF)	-45.0	-109.0	-123.0				





		DESIGN INFORMATION:	PROJECT #: 24-001-006
ents for rei	INFORCED	1. ALL CONSTRUCTION SHALL CONFORM TO THE 2021 SOUTH CAROLINA BUILDING CODE, 2021 INTERNATIONAL BUILDING CODE AND ASCE 7-16.	H CAROLINA
LL BE 3000 P.		2. DESIGN LOADS: DEAD AND LIVE LOADS ROOF LOADS	HAUSER-CREECH, III
PORTS ON S		TOP CHORD DEAD15 psf BOTTOM CHORD DEAD5 psf	No. 4879
		TOP CHORD LIVE20 psf BOTTOM CHORD LIVE10 psf (WITHOUT ATTIC STORAGE)	CF AUTHIN
		CATWALK40 psf FLOOR LOADS TOP CHORD DEAD N/A	I HAUSER
		BOTTOM CHORD DEADN/A TOP CHORD LIVE N/A	hc HAUSER CREECH
		BOTTOM CHORD LIVEN/A RISK CATEGORYIII	
IALL BE CON	NTINUOUS	IMPORTANCE FACTORS I seismic1.25 I snow 1.10	HAUSER-CREECH, INC.
		GROUND SNOW LOAD (pg)5 psf DESIGN WIND SPEED156 mph	P.919.817.7579 P.919.817.7676
		SEISMIC DESIGN PARAMETERS	F.919.404.2427 4506 pearces rd.
NSTRUCTION	I MANUAI"	Ss30.5 %g SITE CLASSD (DEFAULT) Sds 0.316	ZEBULON, NC 27597
		Sd10.177 SEISMIC DESIGN CATEGORYC	
		R6.5 Cv0.0608	
IALL BE 3/4" NS.	DIAMETER	 ADDITIONAL LIVE LOADS PRESCRIBED IN ASCE7-16 RELATED TO ROOF ATTICS AND ROOF TRUSSES, INCLUDING LIMITED ACCESS STORAGE IN ATTICS SHALL APPLY TO PRE-FABRICATED TRUSSES, AND SHALL BE CLEARLY IDENTIFIED ON THE TRUSS SHOP DRAWINGS 	
IOR BOLT EM ALL GREASE,		4. THE DESIGN ADEQUACY AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC. IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.	DUTH C
eq'd by de Conform 1		5. FOR LOCATION OF MISCELLANEOUS ITEMS (SUCH AS INSERTS, ETC.) AFFECTING STRUCTURAL WORK, SEE ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS.	ст.
S. IVE PRIMER 2		6. THIS PROJECT CONTAINS A SERIES OF DETAILS CONSIDERED "TYPICAL DETAILS". THESE SHALL APPLY AT ALL SITUATIONS THAT ARE THE SAME OR SIMILAR AS THESE DETAILS. THESE "TYPICAL DETAILS" SHALL APPLY WHETHER OR NOT THEY ARE INDICATED OR CUT AT EACH LOCATION.	
E VERIFIED BE LD INSTALLA O COATS OF	FORE ANY	 VERIFY EXISTING CONDITIONS AND NOTIFY ARCHITECT AND ENGINEER OF ANY CONDITIONS WHICH DO NOT COMPLY WITH PLANS AND SPECIFICATIONS. STRUCTURAL DRAWINGS MUST BE WORKED WITH ARCHITECTURAL DRAWINGS. 	RU Nrale
For Fabrica	ATION AND	8. USE OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED. THE CONTRACTOR SHALL	ע ∑ ≥
OF EACH IS NOT I	DRAWING.	REVIEW AND STAMP DRAWINGS ACCORDINGLY PRIOR TO SUBMITTING TO THE ENGINEER. THE OMISSION OF ITEMS FROM SHOP DRAWINGS SHALL NOT RELIEVE CONTRACTOR OF RESPONSIBILITY	<i>ო</i>
THE EOR.		OF FURNISHING AND INSTALLING ITEMS REGARDLESS OF WHETHER SHOP DRAWINGS HAVE BEEN REVIEWED AND APPROVED.	
		WOOD FRAMING (NOT INCLUDING PRE-FABRICATED TRUSSES):	ect 284
ICATED IN TH		1. ALL WOOD CONSTRUCTION SHALL CONFORM TO THE SOUTH CAROLINA BUILDING CODE AND TO	c
ICATED IN T	HE DESIGN	THE NDS.	
E TRUSS SUPP EATURES OF		2. ALL NAILING (UNLESS NOTED OTHERWISE) SHALL CONFORM TO THE SOUTH CAROLINA BUILDING CODE.	on, sig
South Carc		3. ALL STUDS, TOP PLATES AND SILL PLATES IN BEARING WALLS AND SHEARWALLS SHALL BE SPF NO. 2 OR BETTER.	A De
PPROVAL.		4. ALL STUDS, TOP PLATES AND SILL PLATES IN NON-BEARING WALLS SHALL BE SPF NO. 3 OR BETTER.	nin ,
NCE WITH A	NPPLICABLE	5. ALL 2x NOMINAL HEADERS SHALL BE SPF NO. 2 OR BETTER OR SYP NO. 2 OR BETTER.	/ilr
		6. ALL EXPOSED LUMBER SHALL BE PRESERVATIVE TREATED.	
		7. FINGER JOINTED STUDS MAY BE USED IN INTERIOR APPLICATIONS PROVIDED THE STRUCTURAL PROPERTIES EQUAL OR EXCEED THAT OF THE SOLID SAWN LUMBER. FINGER JOINTED LUMBER SHALL NOT BE USED IN EXPOSED CONDITIONS.	2-L, Pla
		 ALL CONNECTIONS IN EXPOSED LUMBER SHALL BE HOT DIPPED GALVANIZED OR STAINLESS STEEL. ALL LUMBER IN CONTACT WITH CONCRETE SHALL BE PRESERVATIVE TREATED. 	ite O
WALL WIN	ID LOADS	10. ALL MANUFACTURED LAMINATED VENEER LUMBER (LVL) SHALL HAVE A MODULUS OF ELASTICITY OF 2E6 psi AND A MINIMUM BENDING STRENGTH OF 2800 psi.	Su Bott
WALL ARE	EA	11. UNDER NO CIRCUMSTANCE SHALL LAMINATED VENEER LUMBER BE USED IN AN EXPOSED CONDITION. WHERE MANUFACTURER LUMBER IS REQUIRED IN AN EXPOSED CONDITION THE CONTRACTOR MUST USED PRESERVATIVE TREATED GLU-LAMINATED LUMBER (GLB).	d R. Po k Ave. Suite Architecture
4	5	12. ALL GLU-LAMINATED LUMBER SHALL BE GRADED ACCORDING TO THE PLANS. IF NO GRADE IS SPECIFIED A MINIMUM GADE OF 4VF2400 SHALL BE USED.	avid Park
		FOUNDATION NOTES:	
+53.5	+53.5	1. FOUNDATION DESIGN IS BASED UPON ASSUMED SOIL VALUES. CONTRACTOR/OWNER SHALL VERIFY PRIOR TO CONSTRUCTION. FOOTINGS ARE DESIGNED TO BEAR ON UNIFORM SUITABLE SOIL CAPABLE OF SUPPORTING 2000 PSF.	3806 3806
-58.0	-70.5	*IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO FOLLOW RECOMMENDATIONS BY A LICENSED GEOTECHNICAL ENGINEER TO ACHIEVE 2000 PSF AND LESS THAN 1" ANTICIPATED SETTLEMENT.	TH CARO
F. C&C R WIND. EGATIVE		2. THE SOIL BEARING CAPACITY AND CONSISTENCY SHALL BE VERIFIED FOR THE BUILDING LIMITS BY A REGISTERED GEO-TECHNICAL ENGINEER WHEN FOUNDATION EXCAVATIONS HAVE BEEN CARRIED DOWN TO THE PROPOSED ELEVATIONS. THE BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A 2'-0" MINIMUM BELOW ADJACENT GRADE. (U.N.O.)	MICON SIDES
		4. WHERE FOOTING EXCAVATIONS ARE TO REMAIN OPEN AND MAY BE EXPOSED TO RAINFALL, THE EXCAVATIONS SHALL BE UNDERCUT AND A 3" THICK MUD MAT OF 2000 PSI CONCRETE SHALL BE PLACED OR CLEAN GRAVEL SHALL BE PLACED IN THE BOTTOM TO PROTECT THE BEARING SOILS.	GABRIEL MUL
		5. WHERE FOOTING STEPS ARE NECESSARY, THEY SHALL BE NO STEEPER THAN 1 VERTICAL TO 2 HORIZONTAL, UNLESS SHOWN OTHERWISE ON PLANS.	M. Gabriel Hauser, PE 919-817-7676 gabriel@hauser-creech.com
		6. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY FOR PREPARING THE BUILDING PAD PER THE GEOTECHNICAL ENGINEER OF RECORD'S RECOMMENDATIONS.	ED ITYG
		7. SITE IS ASSUMED TO BE MASS GRADED. GRADING PLANS WERE NOT PROVIDED AT TIME OF DESIGN AND MUST BE PROVIDED FOR COORDINATION PRIOR TO CONSTRUCTION.	00 BI URSI ACIL
		CONCRETE MASONRY:	⊤ Z Ľ
		 CONCRETE MASONRY SHALL CONFORM TO THE NATIONAL CONCRETE MASONRY ASSOCIATION SPECIFICATIONS, AND HAVE A DENSITY OF125 P.C.F. AND SHALL HAVE A MINIMUM PRISM STRENGTH (F'm) OF 1500 P.S.I. 	ISSUE DATE: 02.18.2025
		2. GROUT FOR FILLING CONCRETE MASONRY CELLS SHALL CONFORM TO STANDARD SPECIFICATIONS FOR "GROUT FOR MASONARY", ASTM C-476-02, AND SHALL HAVE A COMPRESSIVE PRISM STRENGTH (F'm) OF 3000 P.S.I. AT 28 DAYS. THE SLUMP SHALL BE BETWEEN 9" AND 11". WHERE THE MINIMUM DIMENSION OF ANY CONTINUOUS VERTICAL CELL IS 3" OR LESS, USE FINE GROUT, OTHERWISE USE	
		COARSE (PEA GRAVEL) GROUT. 3. MORTAR FOR CONCRETE MASONRY SHALL BE TYPE "S" AND SHALL CONFORM TO ASTM C-270-04. 4. GROUT PROCEDURES AND REBAR INSTALLATION SHALL PER ASTM ACI 530 1-99. PROVIDE 36" LAP	S
		SPLICES IN REBAR IN 12" CMU FIRE WALL.	

HAUSER-CREECH, INC.