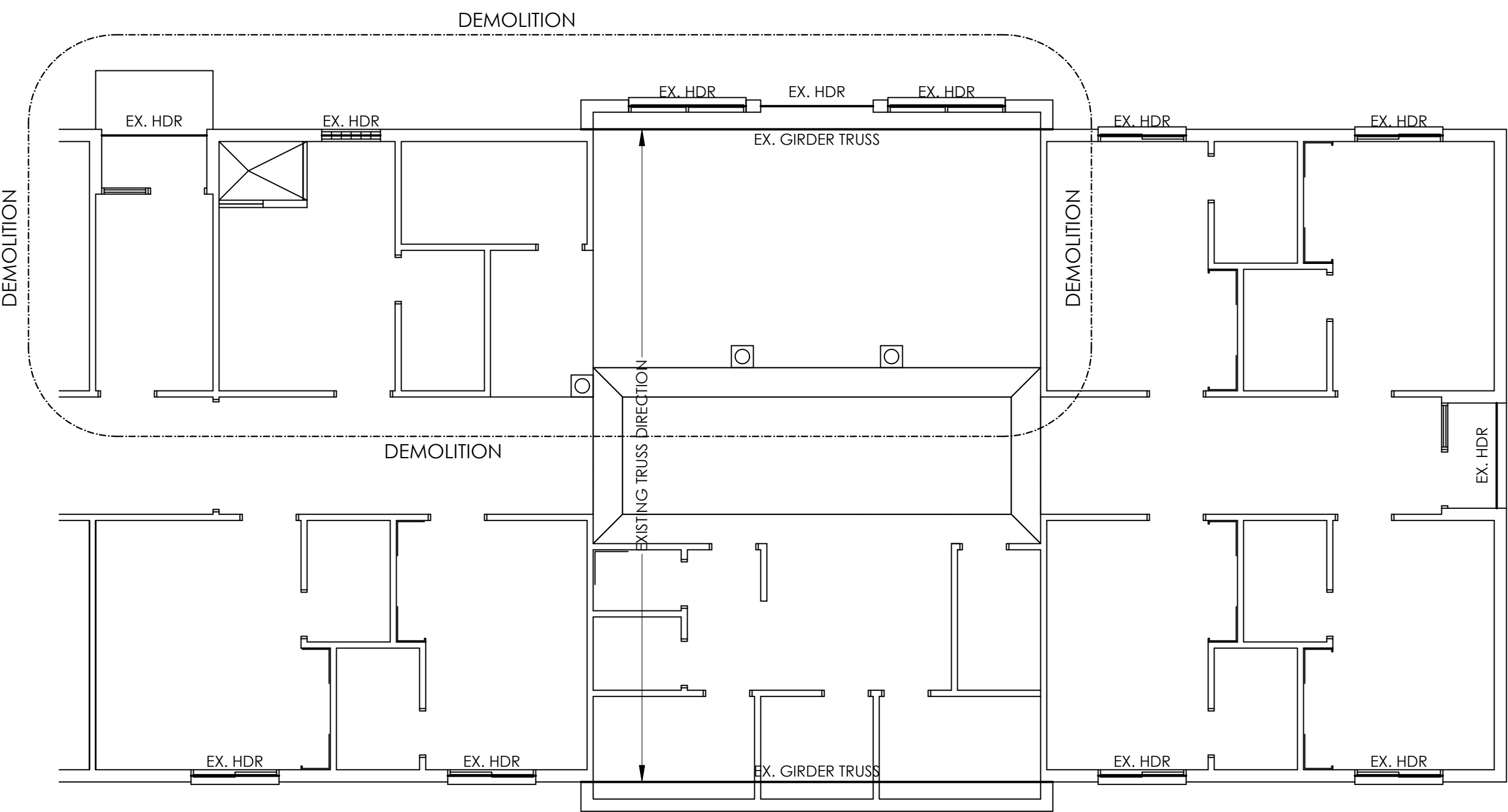


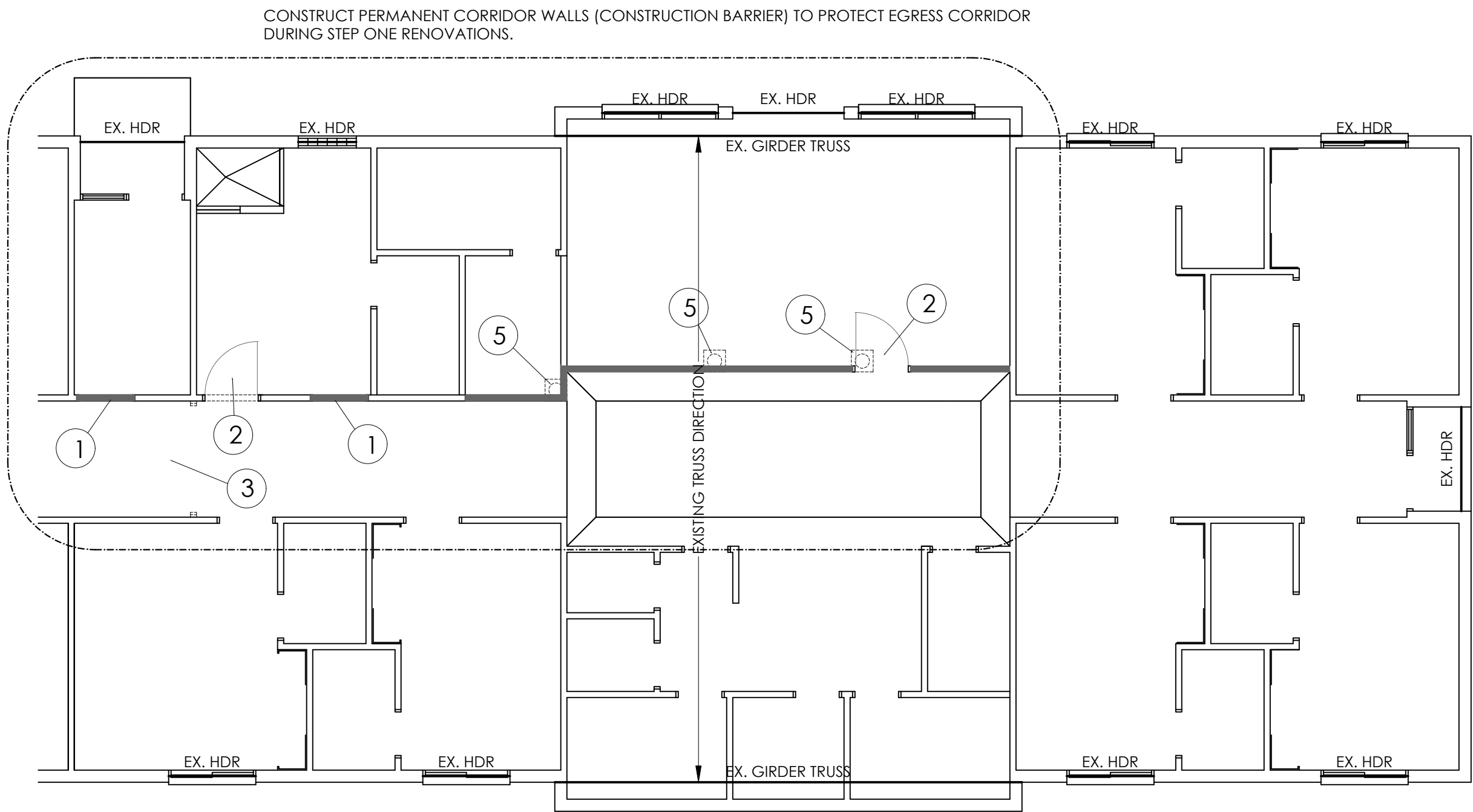
WING "200" RENOVATION / ADDITION - STEP ONE - DEVELOP TWO NEW SEMI-PRIVATE BEDROOMS FOR RELOCATION OF FOUR RESIDENTS.



EXISTING CONDITIONS NOTES:

1. ALL EXTERIOR WALLS ARE ASSUMED TO BE BEARING AND INTERIOR WALLS ARE ASSUMED TO BE NON-BEARING. INTERIOR WALLS INCLUDED IN THE DEMOLITION SCOPE SHALL BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION. NOTIFY EOR OF ANY ASSUMPTION DISCREPANCIES.

CONTRACTOR IS RESPONSIBLE FOR VERIFYING ASSUMED EXISTING CONDITIONS AND REVIEWING ALL PLANS AND DETAILS AGAINST EXISTING CONDITIONS PRIOR TO DEMOLITION. NOTIFY EOR OF ANY DISCREPANCIES. TEMPORARY SHORING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

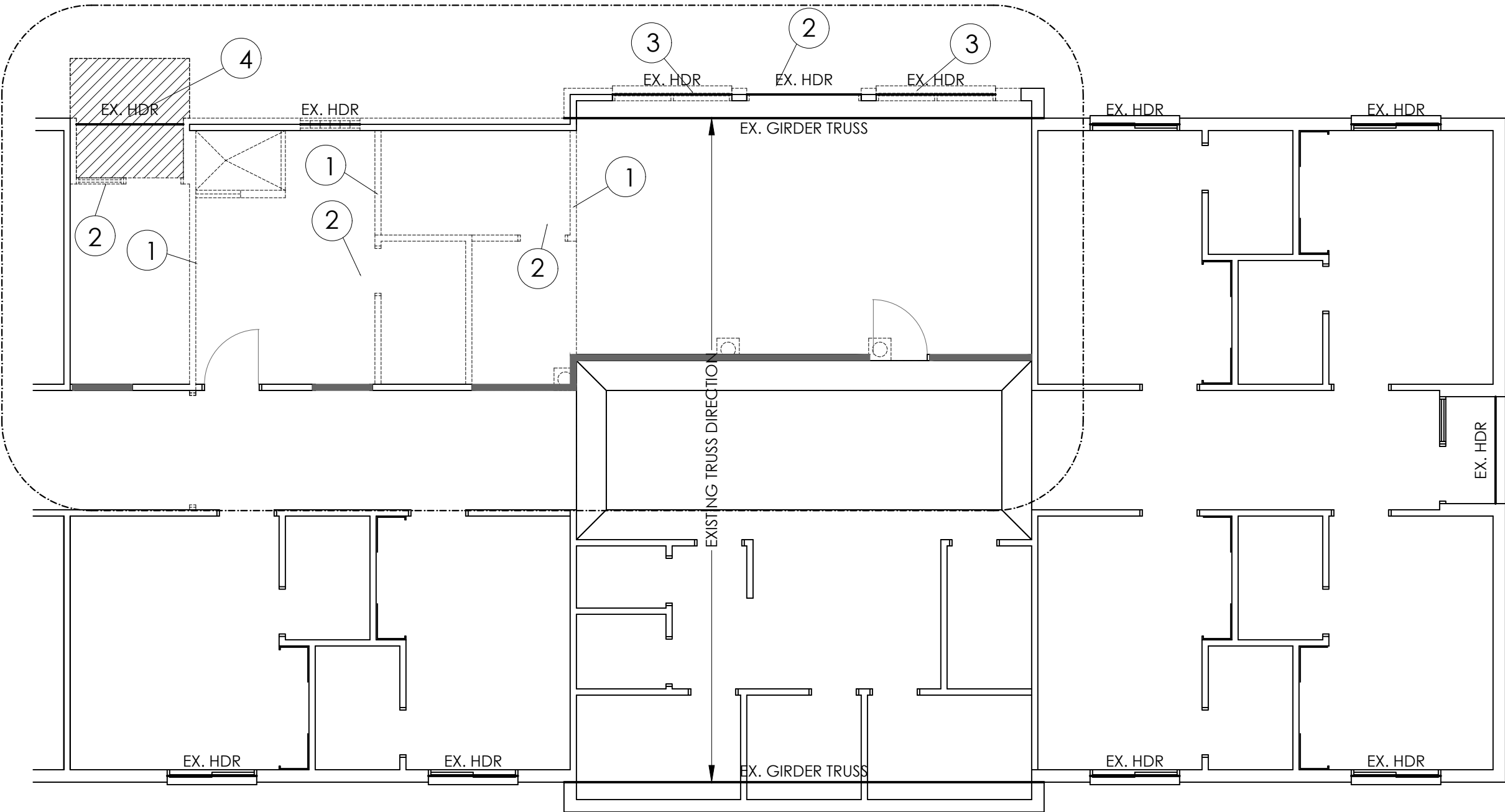


- 1 REMOVE EXISTING DOOR / FRAME, INFILL OPENING WITH 2x4 WOOD STUDS AT 16" O.C. FASTEN TO EXISTING SLAB W/ 0.145" DIA. PAF'S @ 16" O.C. STAGGERED
- 2 ROUGH OPENING AND DOOR PER ARCH.
- 3 REMOVE EXISTING DOORS AND FRAME.
- 4 CONSTRUCT NEW SMOKE RESISTANT PARTITION PER ARCH. - 2x4 WOOD STUDS AT 16" O.C. FASTEN TO EXISTING SLAB W/ 0.145" DIA. PAF'S @ 16" O.C. STAGGERED
- 5 REMOVE EXISTING DECORATIVE COLUMNS AND BASES.

1 FLOOR PLAN STEP ONE PARTIAL PLAN - EXISTING
SCALE: 1/8" = 1'-0"

2 FLOOR PLAN STEP ONE DEMOLITION - CONSTRUCTION BARRIER
SCALE: 1/8" = 1'-0"

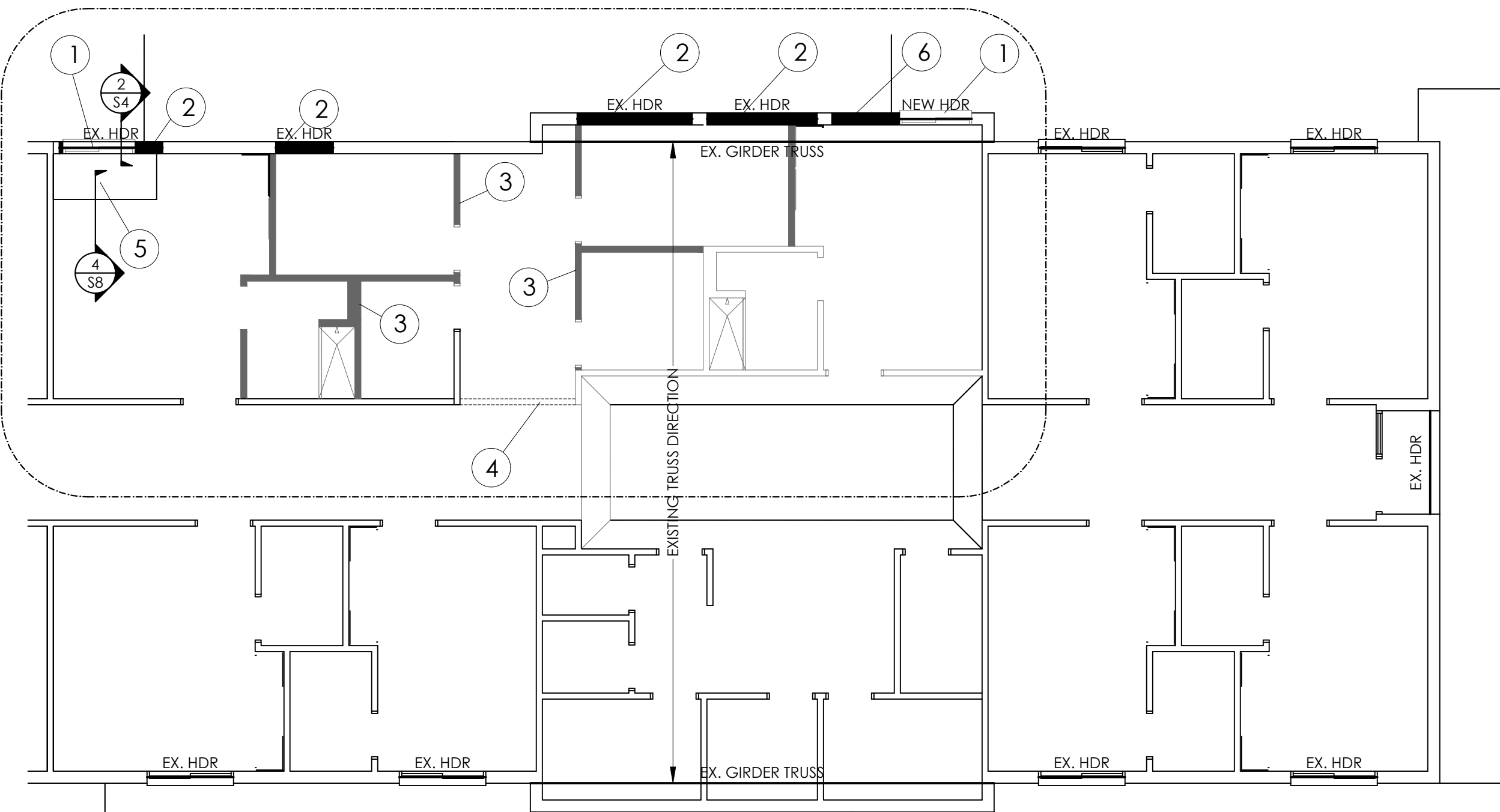
NO DEMOLITION WORK ASSOCIATED WITH STEP TWO SHALL COMMENCE UNTIL ALL OF STEP ONE WORK IS COMPLETED AND THE EGRESS CORRIDOR IS PROTECTED FROM ALL DEMOLITION WORK.



- 1 REMOVE EXISTING PARTITIONS PER ARCH.
- 2 REMOVE EXISTING DOOR AND FRAME.
- 3 REMOVE EXISTING EXTERIOR WINDOWS.
- 4 REMOVE EXTERIOR CONCRETE PAD

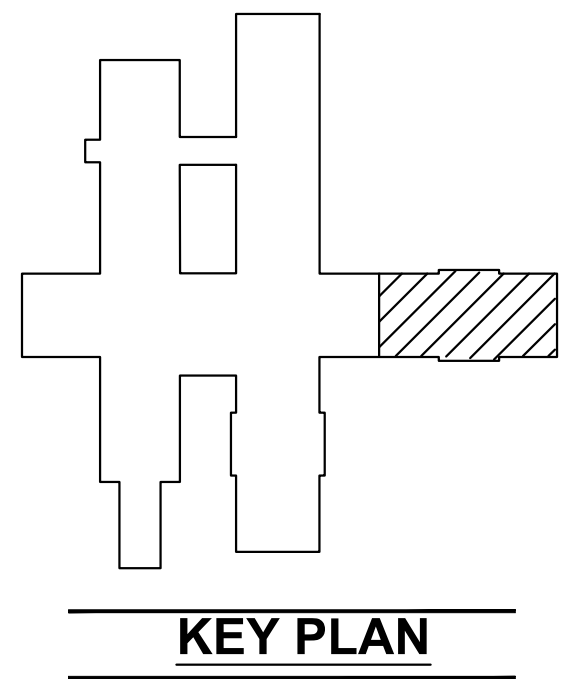
SEE PLUMBING, MECHANICAL, AND ELECTRICAL SHEETS FOR P.M.E. DEMOLITION REQUIREMENTS.

3 FLOOR PLAN STEP ONE PARTIAL PLAN - DEMOLITION
SCALE: 1/8" = 1'-0"



- 1 INSTALL NEW WINDOW HEADER PER PLANS
- 2 EXISTING HEADERS TO REMAIN. INFILL OPENING WITH 2x WOOD STUDS AT 16" O.C. FASTEN TO EXISTING SLAB W/ 0.145" DIA. PAF'S @ 16" O.C. STAGGERED
- 3 TYPICAL NEW NON-BEARING WALL PER ARCH.
- 4 DEMO SECTION OF PARTITION PER ARCH.
- 5 REPLACE SLAB AND EXTERIOR FOOTING
- 6 REMOVE HEADER AND INFILL TO FORM NEW ROUGH OPENING WITH 2x WOOD STUDS AT 16" O.C. FASTEN TO EXISTING SLAB W/ 0.145" DIA. PAF'S @ 16" O.C. STAGGERED

4 FLOOR PLAN STEP ONE PARTIAL PLAN - RENOVATED AREA
SCALE: 1/8" = 1'-0"



HAUSER-CREECH, INC.
PROJECT #: 25-001-001

NORTH CAROLINA
PROFESSIONAL
THEODORE A. DETERS
048492
06/18/25
ENGINEER
THEODORE A. DETERS
NORTH CAROLINA PE NO. 048492

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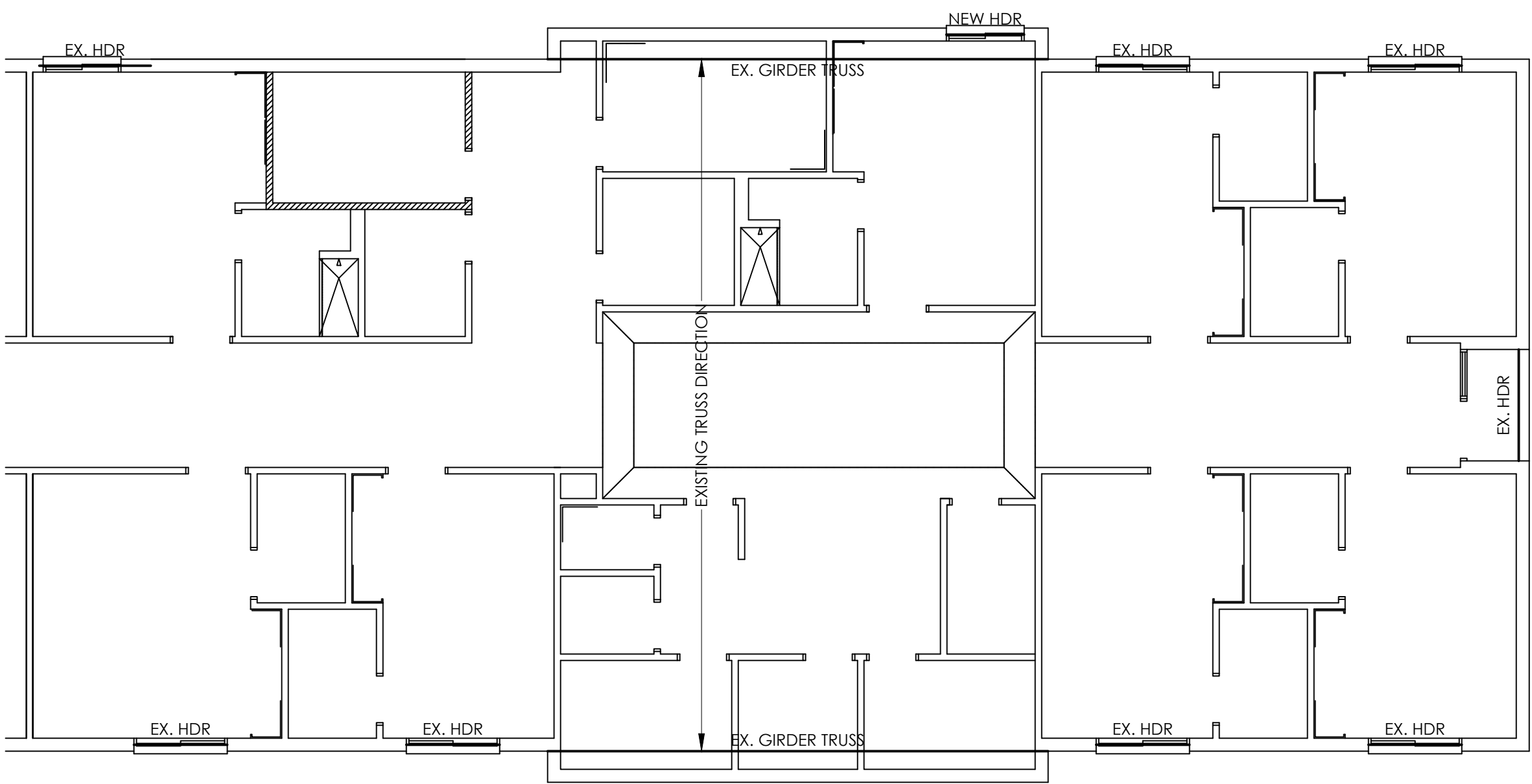
59 BED
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RENOVATION

ISSUE DATE: 06.18.2025

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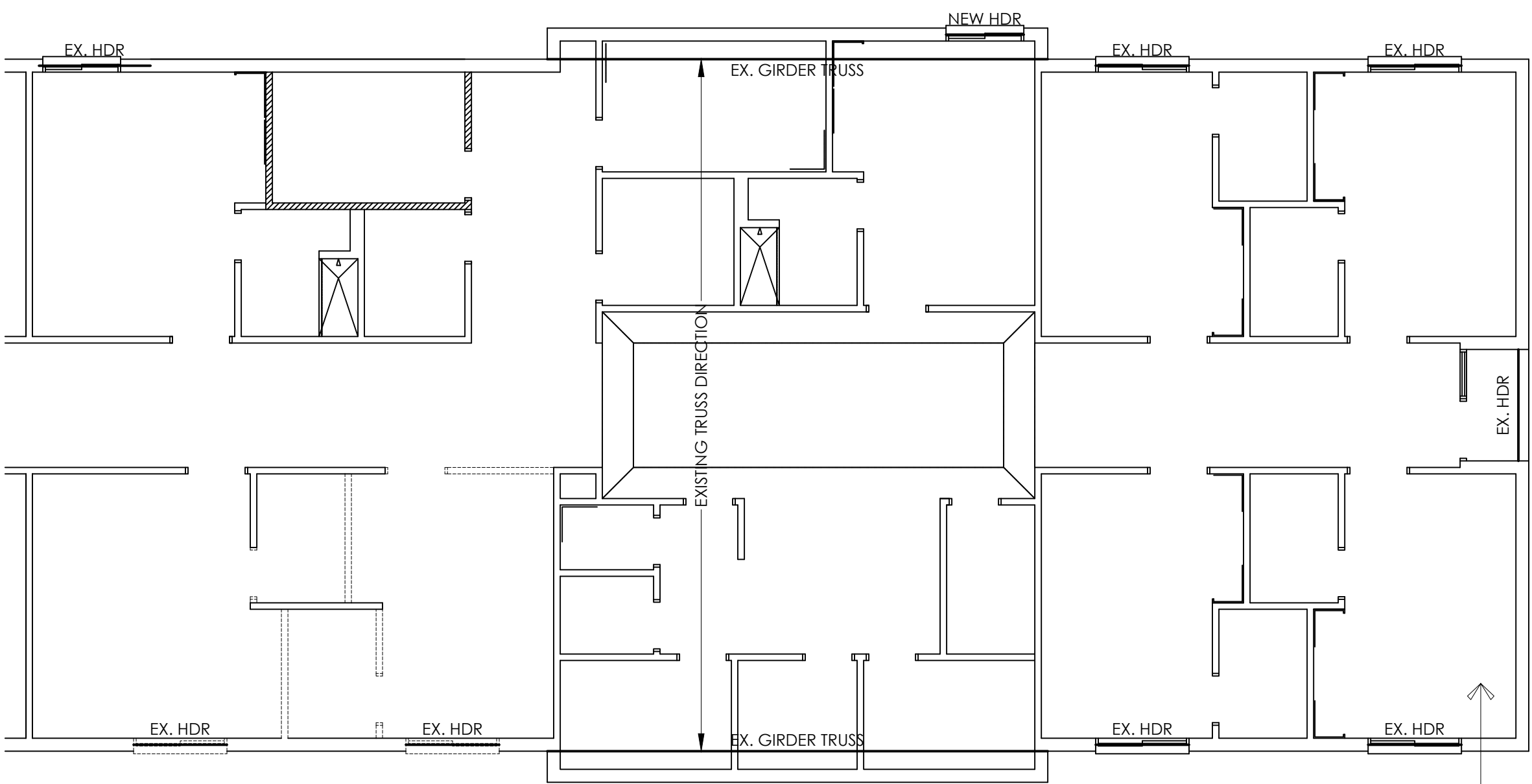
S1.1

WING 200 RENOVATION / ADDITION - STEP TWO - WING 200 TEMPORARY EXIT / RENOVATION OF EXISTING BEDROOMS # 206 AND # 208



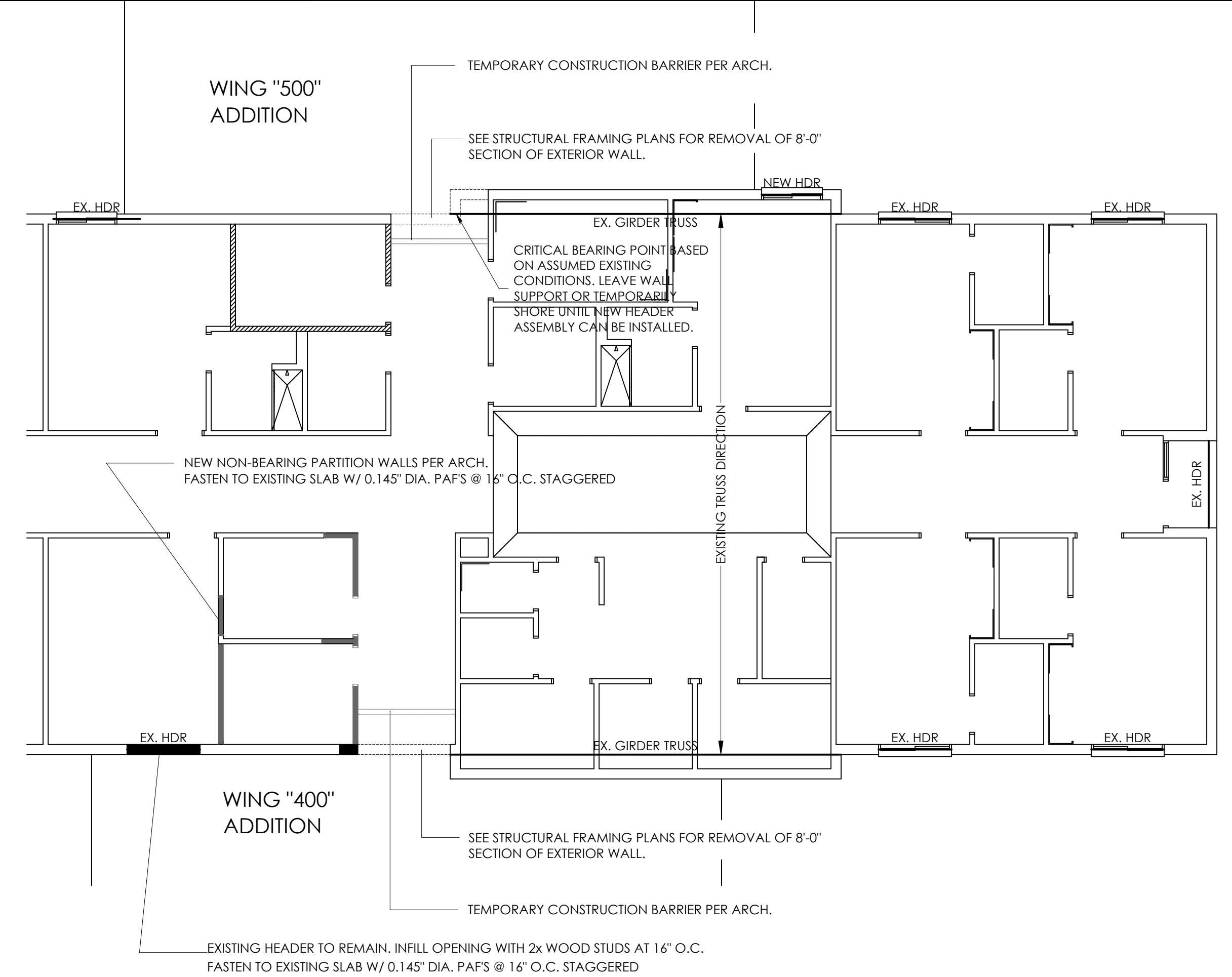
EXISTING CONDITIONS NOTES:

1. ALL EXTERIOR WALLS ARE ASSUMED TO BE BEARING AND INTERIOR WALLS ARE ASSUMED TO BE NON-BEARING. INTERIOR WALLS INCLUDED IN THE DEMOLITION SCOPE SHALL BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION. NOTIFY EOR OF ANY ASSUMPTION DISCREPANCIES.



SEE DETAIL #4, THIS SHEET FOR TEMPORARY EXIT CONSTRUCTION NOTES AND REQUIREMENTS.

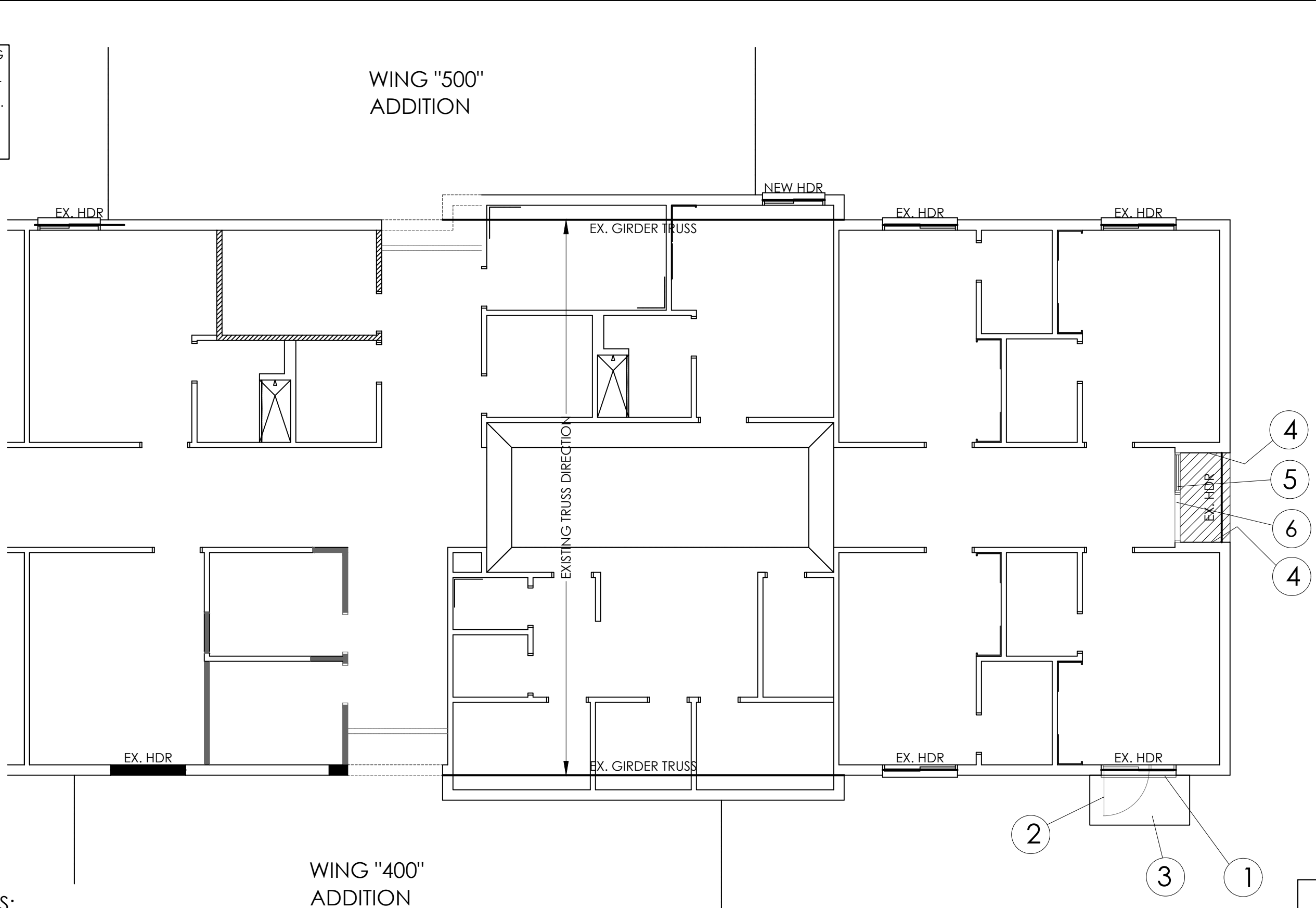
1 FLOOR PLAN
SCALE: 1/8" = 1'-0"
STEP TWO - EXISTING
(AFTER COMPLETION OF STEP ONE)



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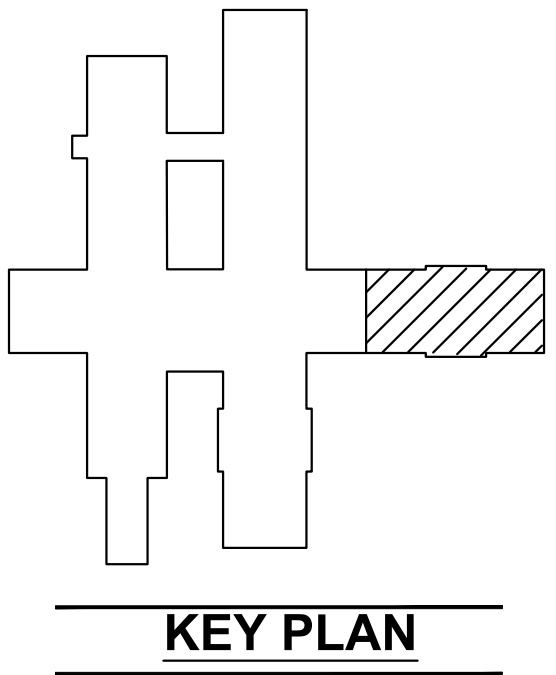
3 FLOOR PLAN
SCALE: 1/8" = 1'-0"
STEP TWO - RENOVATED

2 FLOOR PLAN
SCALE: 1/8" = 1'-0"
STEP TWO - DEMOLITION



- TEMPORARY EXIT NOTES:**
- 1) REMOVE EXTERIOR WINDOW. EXISTING HEADER TO REMAIN
 - 2) INFILL WITH 2x STUDS @ 16" O.C. AND INSTALL TEMPORARY DOOR PER ARCH.
 - 3) PROVIDE CONCRETE PAD PER STRUCTURAL FOUNDATION PLANS

- (AFTER TEMPORARY EXIT IS INSTALLED)
- 4) REMOVE BRICK VENEER AT EACH DOOR ALCOVE WALL.
 - 5) REMOVE EXISTING DOOR AND STOREFRONT.
 - 6) INSTALL TEMPORARY CONSTRUCTION BARRIER PER ARCH.



4 FLOOR PLAN
SCALE: 1/8" = 1'-0"
STEP THREE - TEMPORARY EXIT

HAUSER-CREECH, INC.
PROJECT # 25-001-001

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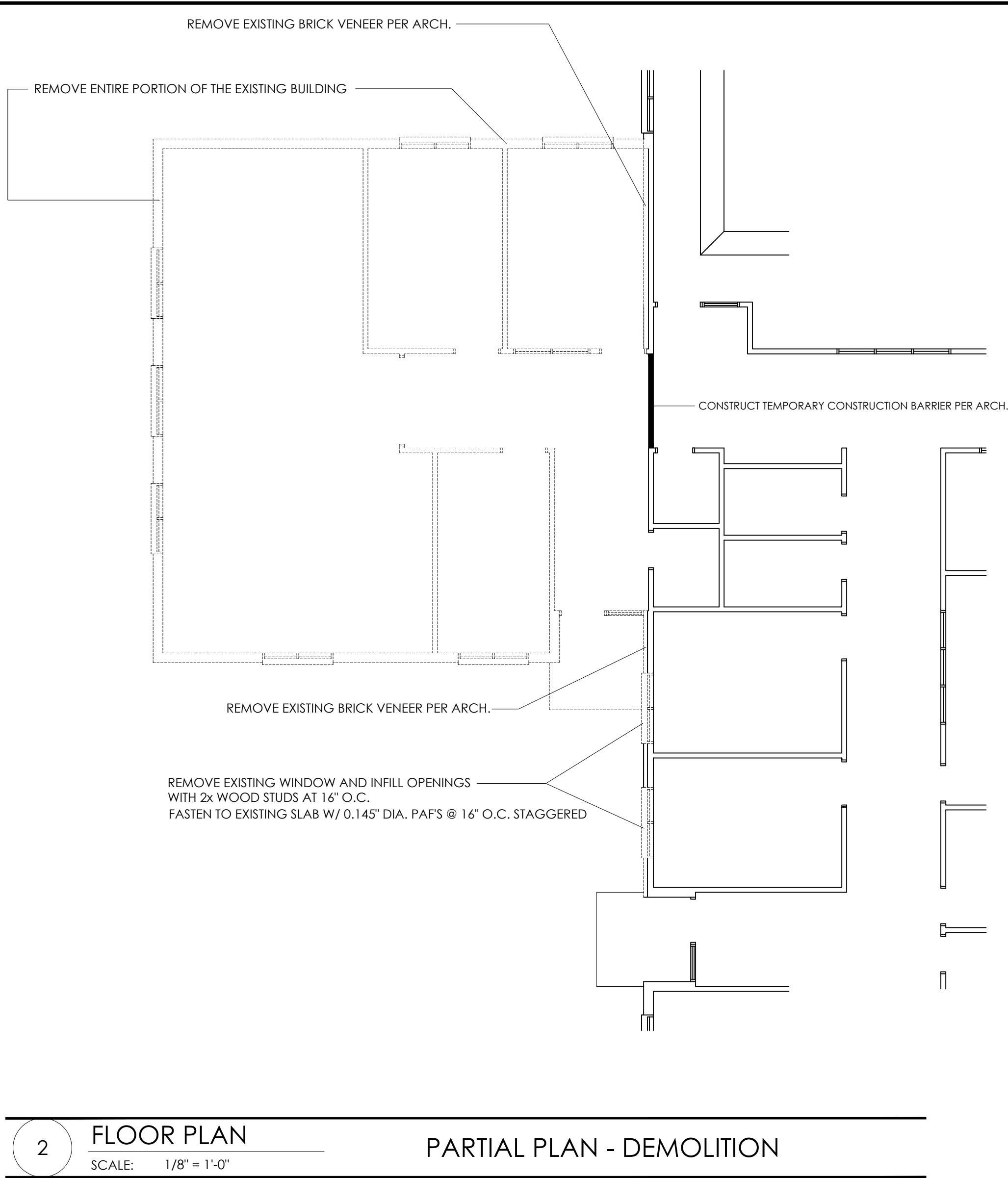
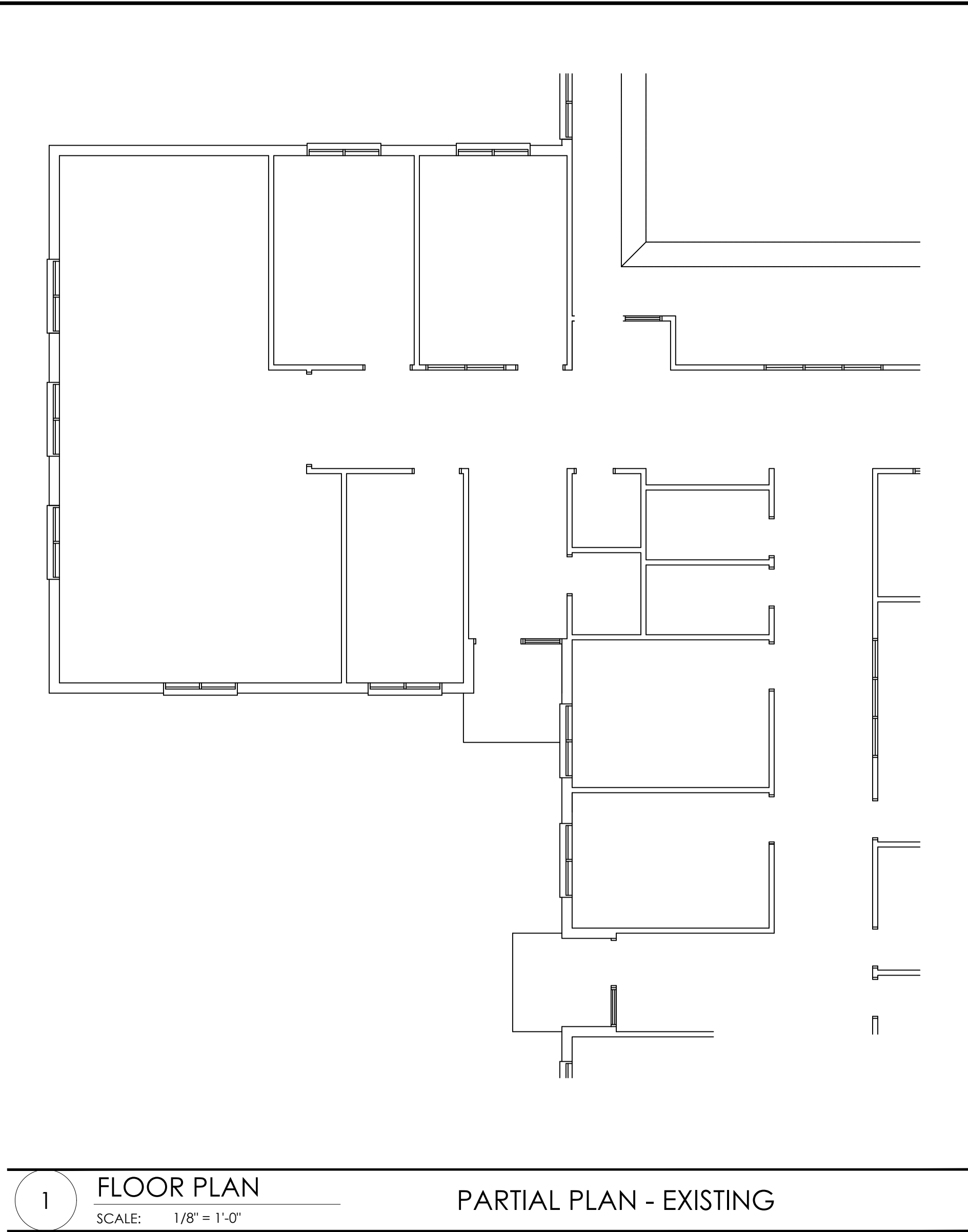
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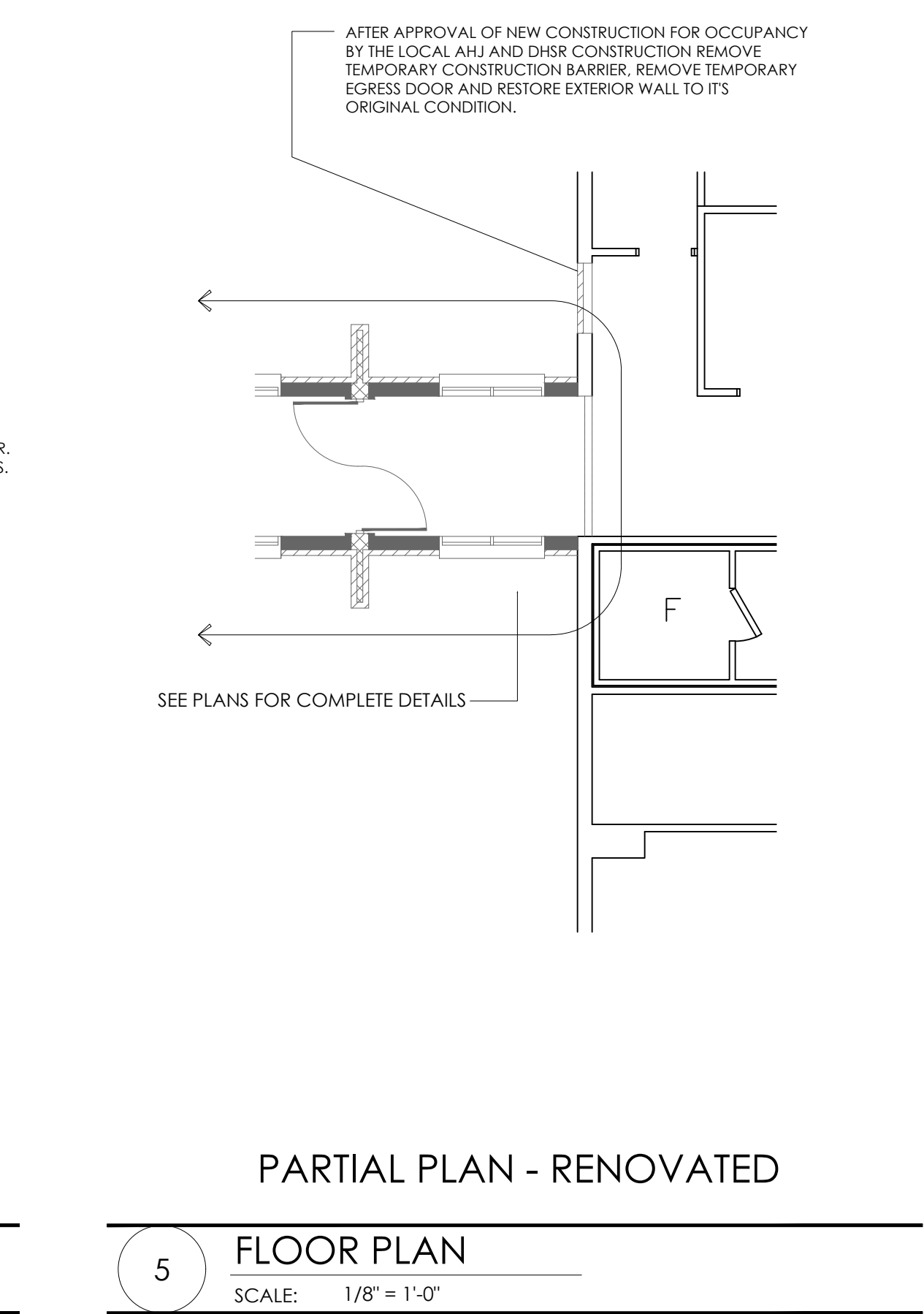
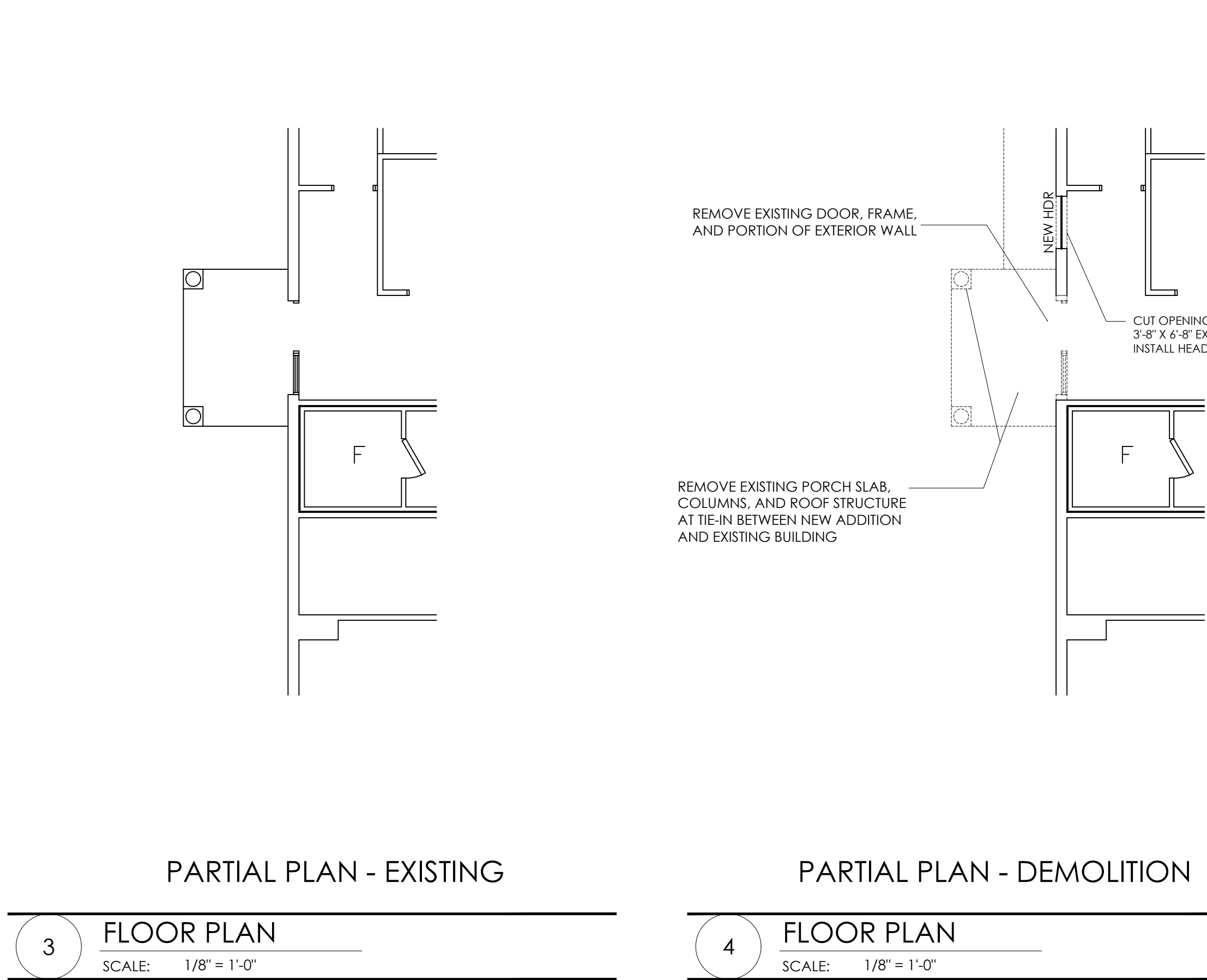
59 BED
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ISSUE DATE: 06.18.2025	
REV	DATE

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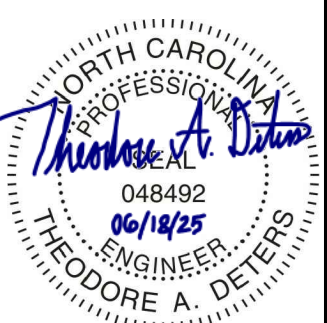
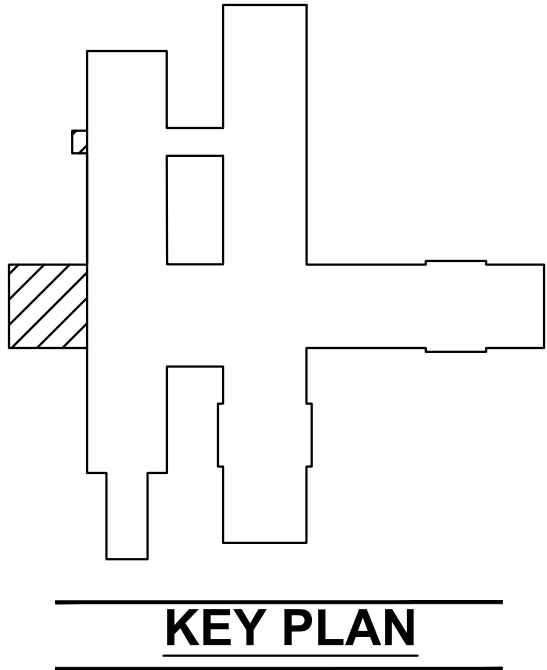


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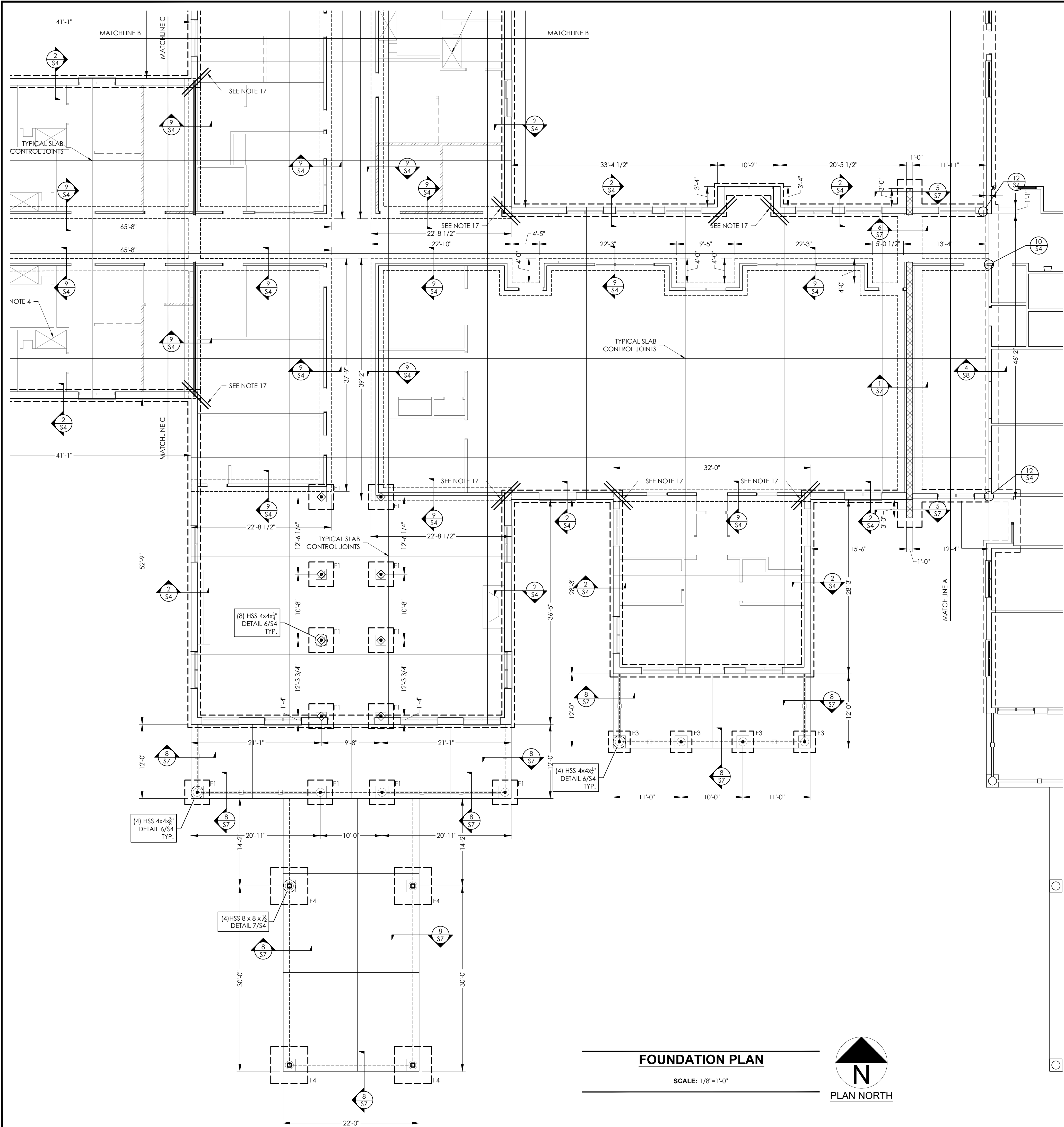


EXISTING CONDITIONS NOTES:

1. ALL EXTERIOR WALLS ARE ASSUMED TO BE BEARING AND INTERIOR WALLS ARE ASSUMED TO BE NON-BEARING. INTERIOR WALLS INCLUDED IN THE DEMOLITION SCOPE SHALL BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION. NOTIFY EOR OF ANY ASSUMPTION DISCREPANCIES.



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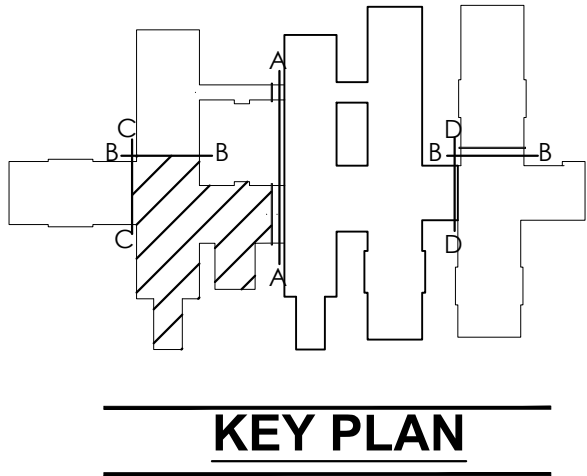


- NOTES:**
1. PROVIDE 4" CONCRETE SLAB ON GRADE REINFORCED W/ WWF 6x6-W1 4xW1 4 OVER 10 MIL POLY VAPOR BARRIER (LAP EDGES 6" MIN.) OVER 4" POROUS BASE ALL DIMENSIONS REFERENCED TO CENTERLINE OF COLUMNS. FACE OF EXTERIOR VENEER, AND CENTERLINE OF INTERIOR BEARING WALLS. SEE ARCHITECTURAL AND STRUCTURAL SECTIONS TO DETERMINE EDGE OF SLAB. VERIFY DIMENSIONS PRIOR TO CONSTRUCTION.
 2. TOP OF EXTERIOR FTG. = F.F.E. -1'-4" AND FIN. GRADE -1'-0" (MIN.)
 3. SEE ARCH. DWGS. FOR DIMENSIONS NOT SHOWN.
 4. SEE DETAIL 5/S-4 FOR RECESSED SLAB DETAILS.
 5. SEE DETAIL 1/S-4 FOR SLAB CONTROL JOINTS (CJ), ALTERNATE LAYOUT PLANS MAY BE SUBMITTED FOR APPROVAL.
 6. SEE ARCHITECTURAL DRAWINGS. FOR LOCATIONS OF RECESSED AND/OR SLOPED SLAB AREAS. PROVIDE POSITIVE DRAINAGE FROM ALL PERIMETER WALLS TO FLOOR DRAIN. COORDINATE W/ PLUMBING DWGS. SEE DETAIL 5/S-4.
 7. LOCATE CONTROL JOINTS UNDERNEATH NON-BEARING WALLS WHERE POSSIBLE.
 8. PROVIDE (4) 2x6 @ EXT. WALLS, (5) 2x4 @ INT. WALLS BEARING (MIN.) AT ALL GIRDER TRUSSES BEARING POINTS AND SHEARWALL END POSTS W/ SIMPSON HTT4 AT STUD BASE.
 9. REFER TO ARCHITECTURAL DRAWINGS FOR RATED WALL LOCATIONS.
 10. SEE FOOTING SCHEDULE FOR SIZES AND REINFORCING.
 11. PROVIDE THICKENED SLAB AS REQUIRED BY WASHER MANUFACTURER. CONTRACTOR TO PROVIDE AND INSTALL REBAR FRAME. SEE 5/S-6.
 12. ALL EXTERIOR STUDS SHALL BE 2x6 SPF NO. 2 STUDS AT 16" O.C. ALL INTERIOR STUDS AT BEARING WALLS AND SHEAR WALLS SHALL BE 2x4 SPF NO. 2 STUDS AT 16" O.C.
 13. OMITTED
 14. INTERIOR FOOTING DIMENSIONS SHOULD NOT BE USED TO LOCATE INTERIOR WALLS. REFER TO ARCHITECTURAL DRAWINGS FOR ALL INTERIOR WALL DIMENSIONS.
 15. OMITTED
 16. OMITTED
 17. PROVIDE (2) 6'-0" LONG #5 BARS AT RE-ENTRANT CORNERS, PLACE AT MID-DEPTH OF SLAB.

FOOTING SCHEDULE		
TYPE	SIZE	REBAR
F1	4'-0" X 4'-0" X 1'-0"	(4) #5s (3'-6" LONG) E.W.
F2	3'-0" X 3'-0" X 1'-0"	(3) #5s (2'-6" LONG) E.W. TOP OF FOOTING = -0'-8" F.F.E.
F3	3'-6" X 3'-6" X 1'-0"	(4) #5s (3'-0" LONG) E.W.
F4	6'-0" X 6'-0" X 1'-6"	(6) #6s (5'-6" LONG) E.W. T + B
F5	4'-0" X 4'-0" X 1'-0" THICKENED SLAB	(4) #5s (3'-6" LONG) E.W.

WRAP ALL EXTERIOR WALLS WITH MINIMUM 7/16" OSB. PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED JOINTS. EDGE NAIL WITH 8d COMMONS AT 4" O.C. AND FIELD NAIL WITH 8d COMMONS AT 12" O.C.

"SHEARWALL" DESIGNATES INTERIOR 2X4 STUDS SHEATHED W/ MINIMUM 7/16" OSB. PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED JOINTS. EDGE NAIL WITH 8d COMMONS AT 4" O.C. AND FIELD NAIL WITH 8d COMMONS AT 12" O.C.



HAUSER-CREECH, INC.
PROJECT #: 25-001-001



THEODORE A. DETERS
NORTH CAROLINA PE NO. 048492



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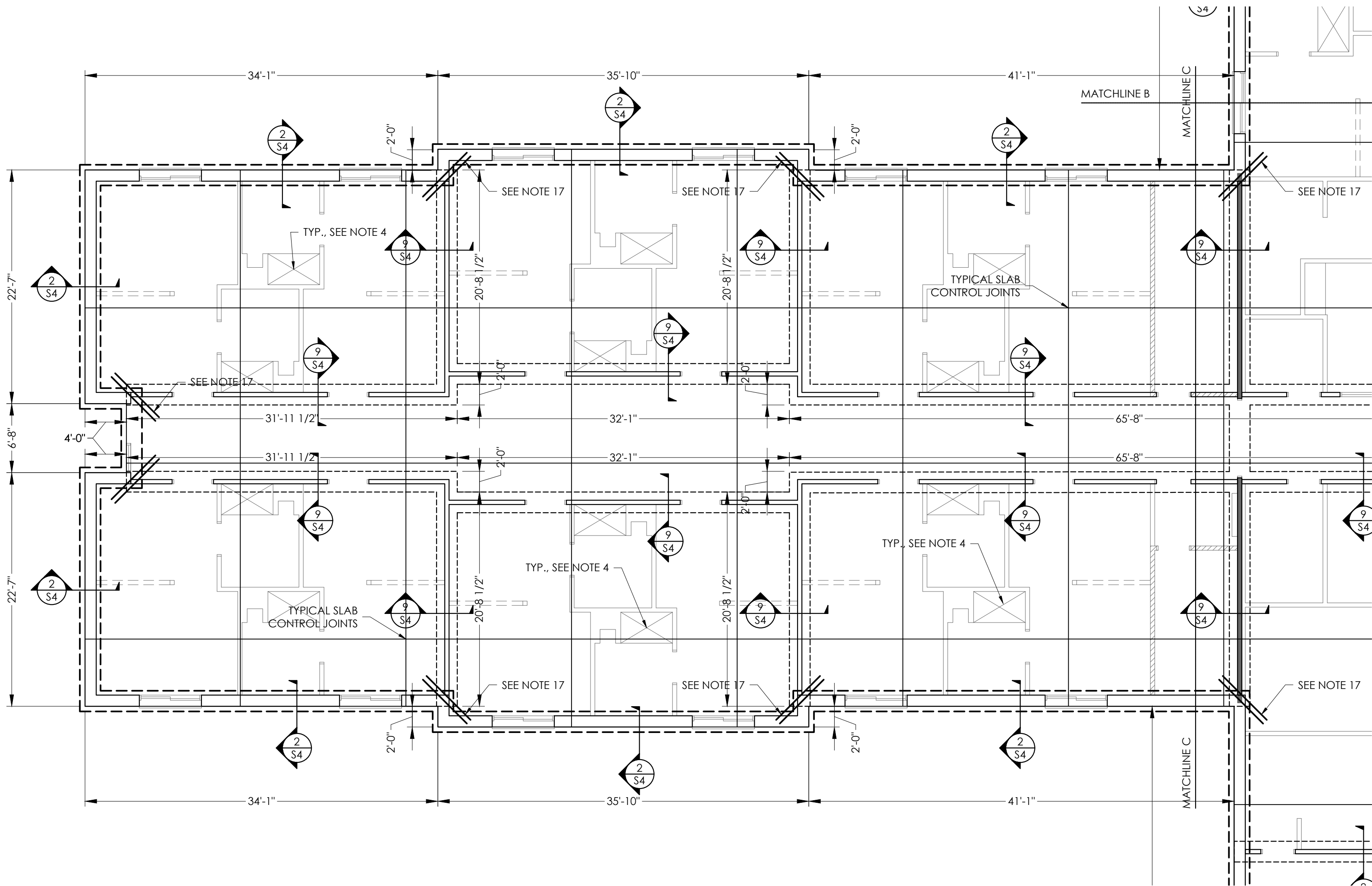
David R. Polston - Architect
3806 Park Ave. Suite 2-L, Wilmington, NC 28403
Architecture Planning Design

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ISSUE DATE: 06.18.2025

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S2.1



FOUNDATION PLAN

SCALE: 1/8"=1'-0"



NOTES:

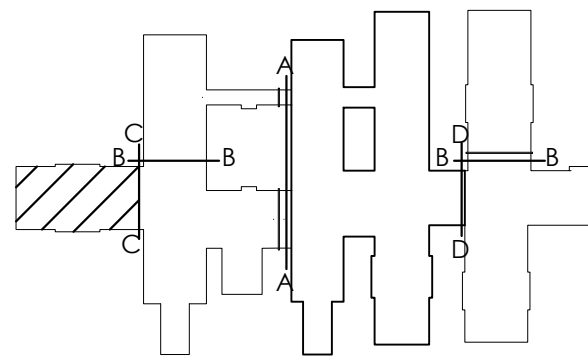
1. PROVIDE 4" CONCRETE SLAB ON GRADE REINFORCED W/ WWF 6x6-W1 4xW1 4 OVER 10 MIL POLY VAPOR BARRIER (LAP EDGES 6" MIN.) OVER 4" POROUS BASE ALL DIMENSIONS REFERENCED TO CENTERLINE OF COLUMNS, FACE OF EXTERIOR VENEER, AND CENTERLINE OF INTERIOR BEARING WALLS. SEE ARCHITECTURAL AND STRUCTURAL SECTIONS TO DETERMINE EDGE OF SLAB. VERIFY DIMENSIONS PRIOR TO CONSTRUCTION.
2. TOP OF EXTERIOR FTG. = F.F.E. -1'-4" AND FIN. GRADE -1'-0" (MIN.)
3. SEE ARCH. DWGS. FOR DIMENSIONS NOT SHOWN.
4. SEE DETAIL 5/S-4 FOR RECESSED SLAB DETAILS.
5. SEE DETAIL 1/S-4 FOR SLAB CONTROL JOINTS (CJ), ALTERNATE LAYOUT PLANS MAY BE SUBMITTED FOR APPROVAL.
6. SEE ARCHITECTURAL DRAWINGS. FOR LOCATIONS OF RECESSED AND/OR SLOPED SLAB AREAS. PROVIDE POSITIVE DRAINAGE FROM ALL PERIMETER WALLS TO FLOOR DRAIN. COORDINATE W/ PLUMBING DWGS. SEE DETAIL 5/S-4.
7. LOCATE CONTROL JOINTS UNDERNEATH NON-BEARING WALLS WHERE POSSIBLE.
8. PROVIDE (4) 2x6 @ EXT. WALLS, (5) 2x4 @ INT. WALLS BEARING (MIN.) AT ALL GIRDER TRUSSES BEARING POINTS AND SHEARWALL END POSTS W/ SIMPSON HTT4 AT STUD BASE.
9. REFER TO ARCHITECTURAL DRAWINGS FOR RATED WALL LOCATIONS.
10. SEE FOOTING SCHEDULE FOR SIZES AND REINFORCING.
11. PROVIDE THICKENED SLAB AS REQUIRED BY WASHER MANUFACTURER. CONTRACTOR TO PROVIDE AND INSTALL REBAR FRAME. SEE 5/S-6.
12. ALL EXTERIOR STUDS SHALL BE 2x6 SPF NO. 2 STUDS AT 16" O.C. ALL INTERIOR STUDS AT BEARING WALLS AND SHEAR WALLS SHALL BE 2x4 SPF NO. 2 STUDS AT 16" O.C.
13. OMITTED
14. INTERIOR FOOTING DIMENSIONS SHOULD NOT BE USED TO LOCATE INTERIOR WALLS. REFER TO ARCHITECTURAL DRAWINGS FOR ALL INTERIOR WALL DIMENSIONS.
15. OMITTED
16. OMITTED
17. PROVIDE (2) 6'-0" LONG #5 BARS AT RE-ENTRANT CORNERS, PLACE AT MID-DEPTH OF SLAB.

FOOTING SCHEDULE

TYPE	SIZE	REBAR
F1	4'-0" X 4'-0" X 1'-0"	(4) #5s (3'-6" LONG) E.W.
F2	3'-0" X 3'-0" X 1'-0"	(3) #5s (2'-6" LONG) E.W. TOP OF FOOTING = -0'-8" F.F.E.
F3	3'-6" X 3'-6" X 1'-0"	(4) #5s (3'-0" LONG) E.W.
F4	6'-0" X 6'-0" X 1'-6"	(6) #6s (5'-6" LONG) E.W. T + B
F5	4'-0" X 4'-0" X 1'-0" THICKENED SLAB	(4) #5s (3'-6" LONG) E.W.

WRAP ALL EXTERIOR WALLS WITH MINIMUM $\frac{7}{16}$ " OSB. PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED JOINTS. EDGE NAIL WITH 8d COMMONS AT 4" O.C. AND FIELD NAIL WITH 8d COMMONS AT 12" O.C.

"SHEARWALL" DESIGNATES INTERIOR 2X4 STUDS SHEATHED W/ MINIMUM $\frac{7}{16}$ " OSB. PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED JOINTS. EDGE NAIL WITH 8d COMMONS AT 4" O.C. AND FIELD NAIL WITH 8d COMMONS AT 12" O.C.



KEY PLAN

HAUSER-CREECH, INC.
PROJECT # 25-001-001



THEODORE A. DETERS
NORTH CAROLINA PE NO. 048492



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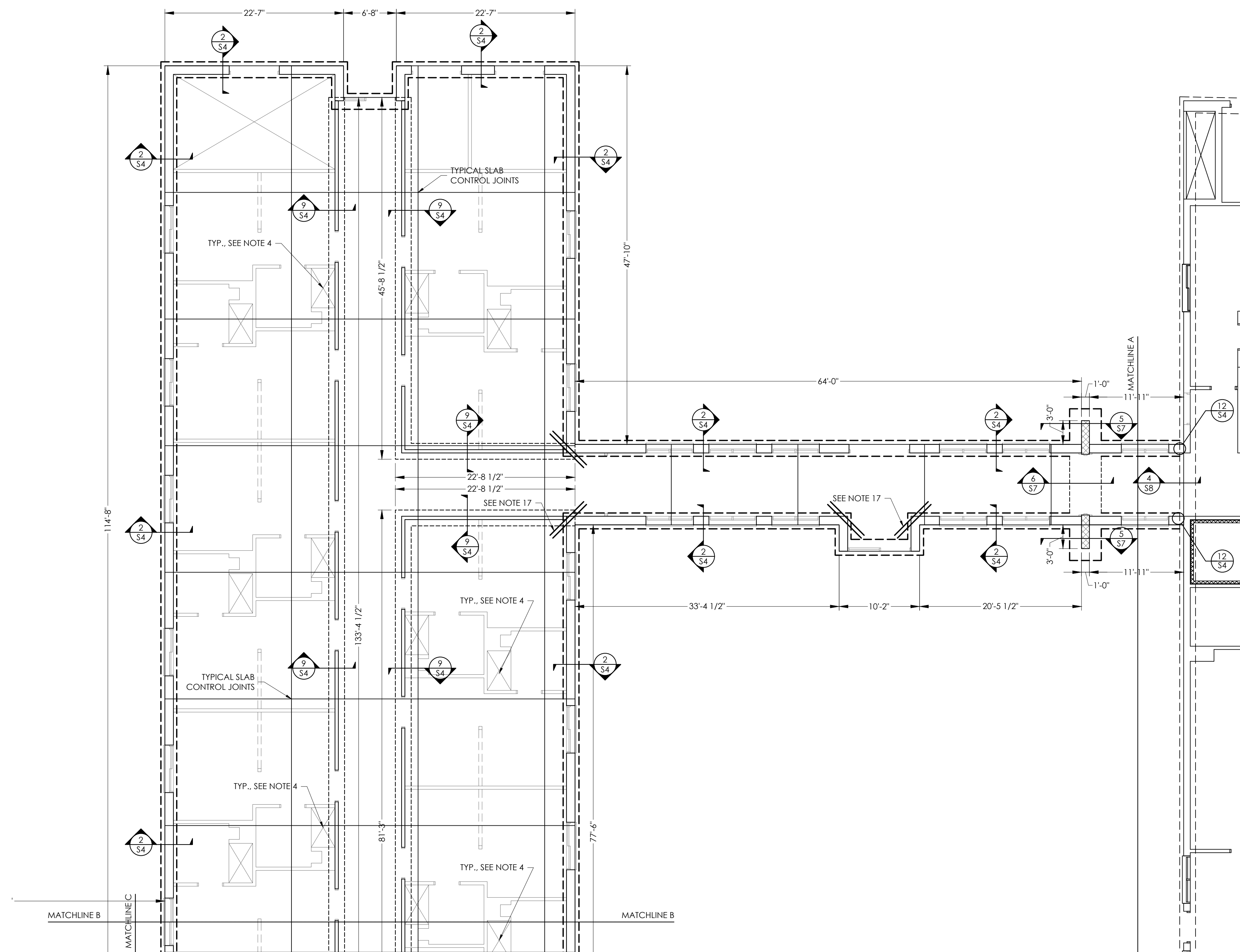
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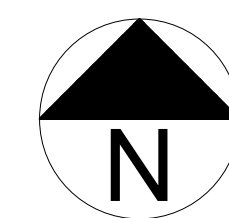
REV	DATE

S2.2



FOUNDATION PLAN

SCALE: 1/8"=1'-0"



PLAN NORTH

NOTES:

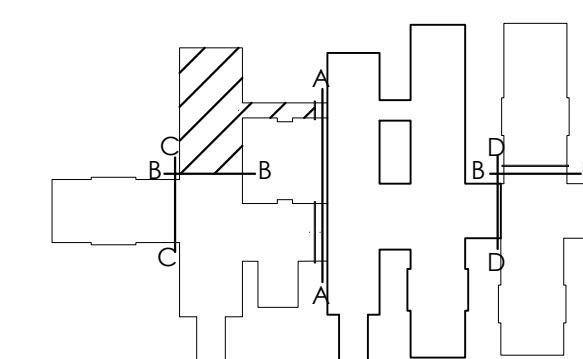
1. PROVIDE 4" CONCRETE SLAB ON GRADE REINFORCED W/ WWF 6x6-W1.4W1.4 REFER TO 10 MIL POLY VAPOR BARRIER (LAP EDGES 6" MIN.) OVER 4" POROUS BASE. ALL DIMENSIONS REFERENCED TO CENTERLINE OF COLUMNS, FACE OF EXTERIOR VENEER, AND CENTERLINE OF INTERIOR BEARING WALLS. SEE ARCHITECTURAL AND STRUCTURAL SECTIONS TO DETERMINE EDGE OF SLAB. VERIFY DIMENSIONS PRIOR TO CONSTRUCTION.
2. TOP OF EXTERIOR FTG. = F.F.E. -1'-4" AND FIN. GRADE -1'-0" (MIN.).
3. SEE ARCH. DWGS. FOR DIMENSIONS NOT SHOWN.
4. SEE DETAIL 5/S-4 FOR RECESSED SLAB DETAILS.
5. SEE DETAIL 1/S-4 FOR SLAB CONTROL JOINTS (CJ), ALTERNATE LAYOUT PLANS MAY BE SUBMITTED FOR APPROVAL.
6. SEE ARCHITECTURAL DRAWINGS, FOR LOCATIONS OF RECESSED AND/OR SLOPED SLAB AREAS. PROVIDE POSITIVE DRAINAGE FROM ALL PERIMETER WALLS TO FLOOR DRAIN. COORDINATE W/ PLUMBING DWGS. SEE DETAIL 5/S-4.
7. LOCATE CONTROL JOINTS UNDERNEATH NON-BEARING WALLS WHERE POSSIBLE.
8. PROVIDE (4) 2X6 @ EXT. WALLS, (5) 2X4 @ INT. HTLS BEARING (MIN.) AT ALL GIRDER TRUSSES BEARING POINTS AND SHEARWALL UNDER POSTS W/ SIMPSON HTTS AT STUD BASE.
9. REFER TO ARCHITECTURAL DRAWINGS FOR RATED WALL LOCATIONS.
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11. PROVIDE THICKENED SLAB AS REQUIRED BY WASHER MANUFACTURER. CONTRACTOR TO PROVIDE AND INSTALL REBAR FRAME. SEE 5/S-6.
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13. OMITTED
14. INTERIOR FOOTING DIMENSIONS SHOULD NOT BE USED TO LOCATE INTERIOR WALLS. REFER TO ARCHITECTURAL DRAWINGS FOR ALL INTERIOR WALL DIMENSIONS.
15. OMITTED
16. OMITTED
17. PROVIDE (2) 6'-0" LONG 5# BARS AT RE-ENTRANT CORNERS, PLACE AT MID-DEPTH OF SLAB.

FOOTING SCHEDULE

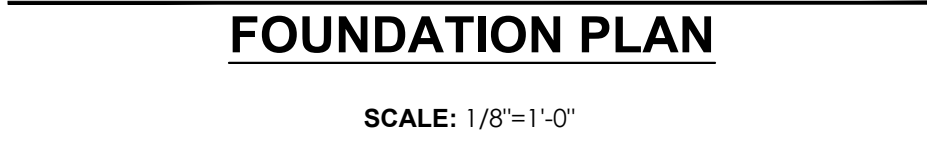
	TYPE	SIZE	REBAR
	F1	4'-0" X 4'-0" X 1'-0"	(4) #5s (3'-6" LONG) E.W.
	F2	3'-0" X 3'-0" X 1'-0"	(3) #5s (2'-6" LONG) E.W. TOP OF FOOTING = 0'-8" F.F.E.
	F3	3'-6" X 3'-6" X 1'-0"	(4) #5s (3'-0" LONG) E.W.
	F4	6'-0" X 6'-0" X 1'-6"	(6) #6s (5'-6" LONG) E.W. T + B
	F5	4'-0" X 4'-0" X 1'-0" THICKENED SLAB	(4) #5s (3'-6" LONG) E.W.

WRAP ALL EXTERIOR WALLS WITH MINIMUM $\frac{7}{16}$ " OSB.
PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED
JOINTS. EDGE NAIL WITH 8d COMMONS AT 4" O.C. AND
FIELD NAIL WITH 8d COMMONS AT 12" O.C.

"SHEARWALL" DESIGNATES INTERIOR 2X4 STUDS SHEATHED W/ MINIMUM $\frac{7}{16}$ " OSB. PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED JOINTS. EDGE NAIL WITH 8d COMMONS AT 4" O.C. AND FIELD NAIL WITH 8d COMMONS AT 12" O.C.



KEY PLAN

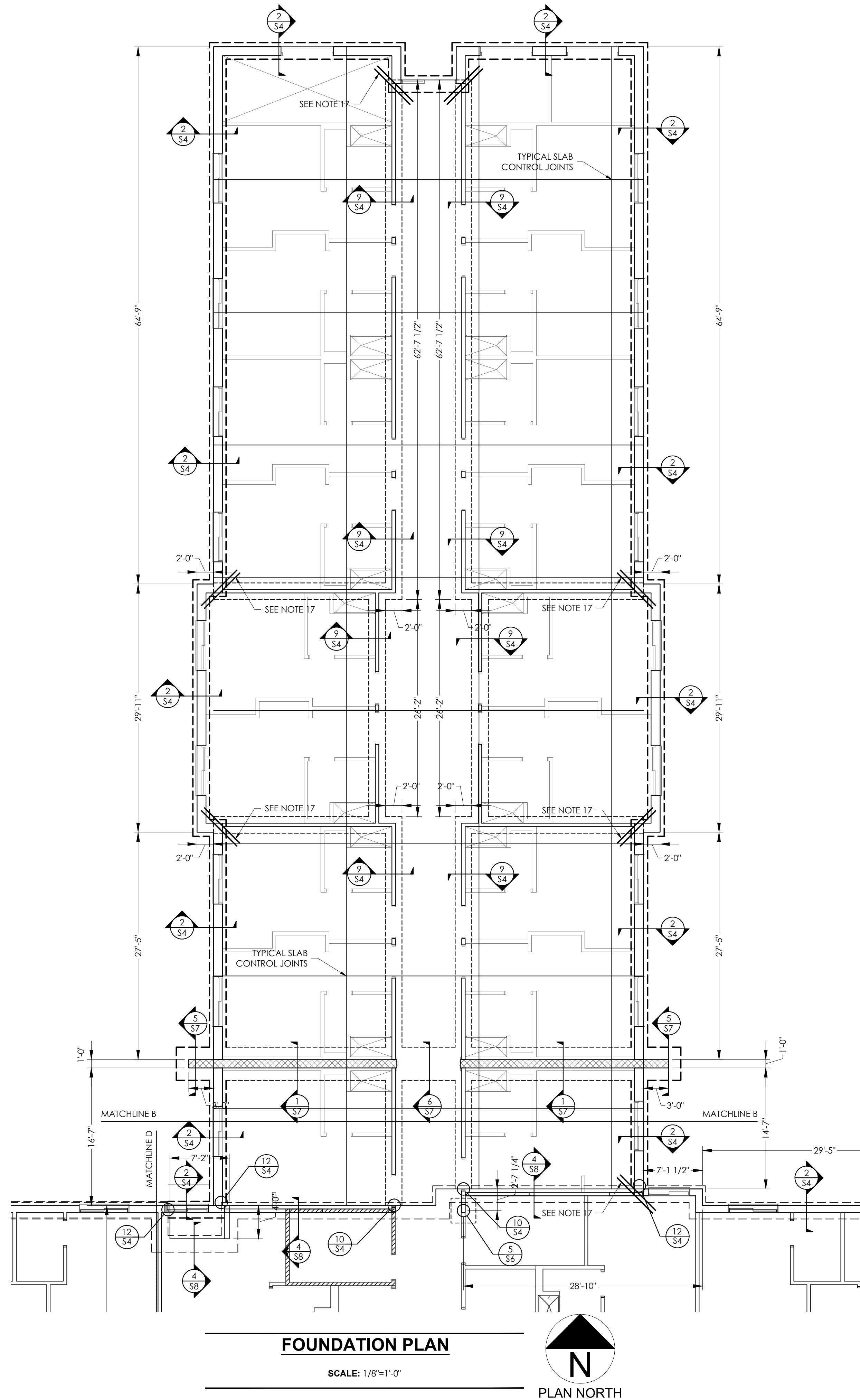


1. PROVIDE 4" CONCRETE SLAB ON GRADE REINFORCED W/ WWF 6x6-1/4, 4x1/4 OVER 10 MIL POLY VAPOR BARRIER (LAP EDGES 6" MIN.) OVER 4" POROUS BASE ALL DIMENSIONS REFERENCED TO CENTERLINE OF COLUMNS, FACE OF EXTERIOR VENEER, AND CENTERLINE OF INTERIOR BEARING WALLS. SEE ARCHITECTURAL AND STRUCTURAL SECTIONS TO DETERMINE EDGE OF SLAB. VERIFY DIMENSIONS PRIOR TO CONSTRUCTION.
2. TOP OF EXTERIOR FTG. = F.F.E. -1'-4" AND FIN. GRADE -1'-0" (MIN.)
3. SEE ARCH. DWGS. FOR DIMENSIONS NOT SHOWN.
4. SEE DETAIL 5/5-4 FOR RECESSED SLAB DETAILS.
5. SEE DETAIL 1/5-4 FOR SLAB CONTROL JOINTS (CJ), ALTERNATE LAYOUT PLANS MAY BE SUBMITTED FOR APPROVAL.
6. PROVIDE STRUCTURAL DRAWINGS, FOR LOCATIONS OF RECESSED AND/OR SLOPED SLAB AREAS.
7. PROVIDE POSITIVE DRAINAGE FROM ALL PERIMETER WALLS TO FLOOR DRAIN. COORDINATE W/ PLUMBING DWGS. SEE DETAIL 5/5-4.
7. LOCATE CONTROL JOINTS UNDERNEATH NON-BEARING WALLS WHERE POSSIBLE.
8. PROVIDE (4) 2x6 @ EXT. WALLS, (5) 2x4 @ INT. WALLS BEARING (MIN.) AT ALL GIRDER TRUSSES BEARING POINTS AND SHEARWALL END POSTS W/ SIMPSON HT74 AT STUD BASE.
9. REFER TO ARCHITECTURAL DRAWINGS FOR RATED WALL LOCATIONS.
10. SEE FOOTING SCHEDULE FOR SIZES AND REINFORCING.
11. PROVIDE THICKENED SLAB AS REQUIRED BY WASHER MANUFACTURER. CONTRACTOR TO PROVIDE AND INSTALL REBAR FRAME. SEE 5/5-6.
12. ALL EXTERIOR STUDS SHALL BE 2x6 SPF NO. 2 STUDS AT 16" O.C. ALL INTERIOR STUDS AT BEARING WALLS AND SHEAR WALLS SHALL BE 2x4 SPF NO. 2 STUDS AT 16" O.C.
13. OMITTED
14. INTERIOR FLOOR DIMENSIONS SHOULD NOT BE USED TO LOCATE INTERIOR WALLS. REFER TO ARCHITECTURAL DRAWINGS FOR ALL INTERIOR WALL DIMENSIONS.
15. OMITTED
16. OMITTED
17. PROVIDE (2) 6'-0" LONG #5 BARS AT RE-ENTRANT CORNERS. PLACE AT MID-DEPTH OF SLAB.

WRAP ALL EXTERIOR WALLS WITH MINIMUM $\frac{7}{16}$ " OSB.
PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED
JOINTS. EDGE NAIL WITH 8d COMMONS AT 4' O.C. AND
FIELD NAIL WITH 8d COMMONS AT 12' O.C.

"SHEARWALL" DESIGNATES INTERIOR 2X4 STUDS SHEATHED W/ MINIMUM $\frac{7}{16}$ " OSB. PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED JOINTS. EDGE NAIL WITH 8d COMMONS AT 4" O.C. AND FIELD NAIL WITH 8d COMMONS AT 12" O.C.



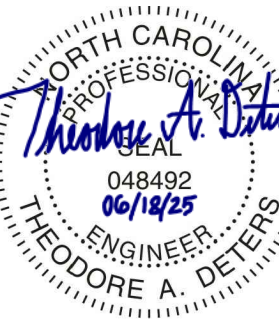
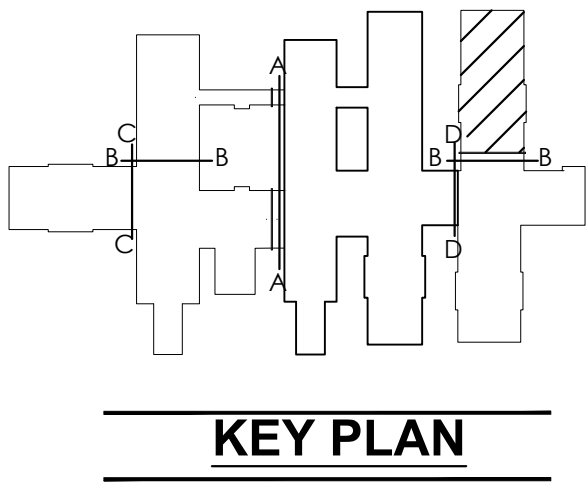


- NOTES:**
1. PROVIDE 4" CONCRETE SLAB ON GRADE REINFORCED W/ WWF 6x6-W1 4xW1 4 OVER 10 MIL POLY VAPOR BARRIER (LAP EDGES 6" MIN.) OVER 4" POROUS BASE ALL DIMENSIONS REFERENCED TO CENTERLINE OF COLUMNS, FACE OF EXTERIOR VENEER, AND CENTERLINE OF INTERIOR BEARING WALLS. SEE ARCHITECTURAL AND STRUCTURAL SECTIONS TO DETERMINE EDGE OF SLAB. VERIFY DIMENSIONS PRIOR TO CONSTRUCTION.
 2. TOP OF EXTERIOR FTG. = F.F.E. -1'-4" AND FIN. GRADE -1'-0" (MIN.)
 3. SEE ARCH. DWGS. FOR DIMENSIONS NOT SHOWN.
 4. SEE DETAIL 5/S-4 FOR RECESSED SLAB DETAILS.
 5. SEE DETAIL 1/S-4 FOR SLAB CONTROL JOINTS (CJ), ALTERNATE LAYOUT PLANS MAY BE SUBMITTED FOR APPROVAL.
 6. SEE ARCHITECTURAL DRAWINGS. FOR LOCATIONS OF RECESSED AND/OR SLOPED SLAB AREAS. PROVIDE POSITIVE DRAINAGE FROM ALL PERIMETER WALLS TO FLOOR DRAIN. COORDINATE W/ PLUMBING DWGS. SEE DETAIL 5/S-4.
 7. LOCATE CONTROL JOINTS UNDERNEATH NON-BEARING WALLS WHERE POSSIBLE.
 8. PROVIDE (4) 2x6 @ EXT. WALLS, (5) 2x4 @ INT. WALLS BEARING (MIN.) AT ALL GIRDER TRUSSES BEARING POINTS AND SHEARWALL END POSTS W/ SIMPSON HTT4 AT STUD BASE.
 9. REFER TO ARCHITECTURAL DRAWINGS FOR RATED WALL LOCATIONS.
 10. SEE FOOTING SCHEDULE FOR SIZES AND REINFORCING.
 11. PROVIDE THICKENED SLAB AS REQUIRED BY WASHER MANUFACTURER. CONTRACTOR TO PROVIDE AND INSTALL REBAR FRAME. SEE 5/S-6.
 12. ALL EXTERIOR STUDS SHALL BE 2x6 SPF NO. 2 STUDS AT 16" O.C. ALL INTERIOR STUDS AT BEARING WALLS AND SHEAR WALLS SHALL BE 2x4 SPF NO. 2 STUDS AT 16" O.C.
 13. OMITTED
 14. INTERIOR FOOTING DIMENSIONS SHOULD NOT BE USED TO LOCATE INTERIOR WALLS. REFER TO ARCHITECTURAL DRAWINGS FOR ALL INTERIOR WALL DIMENSIONS.
 15. OMITTED
 16. OMITTED
 17. PROVIDE (2) 6'-0" LONG #5 BARS AT RE-ENTRANT CORNERS, PLACE AT MID-DEPTH OF SLAB.

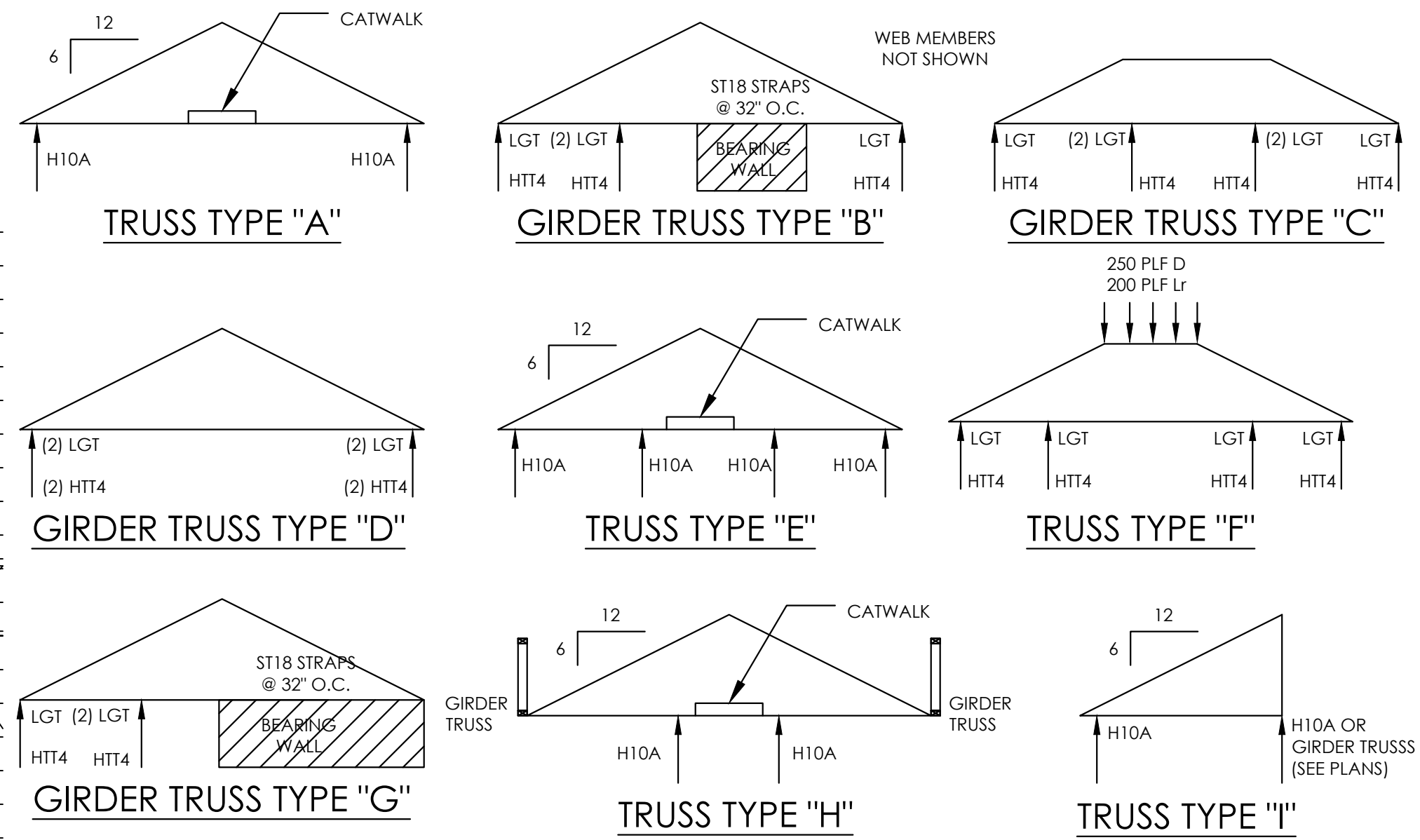
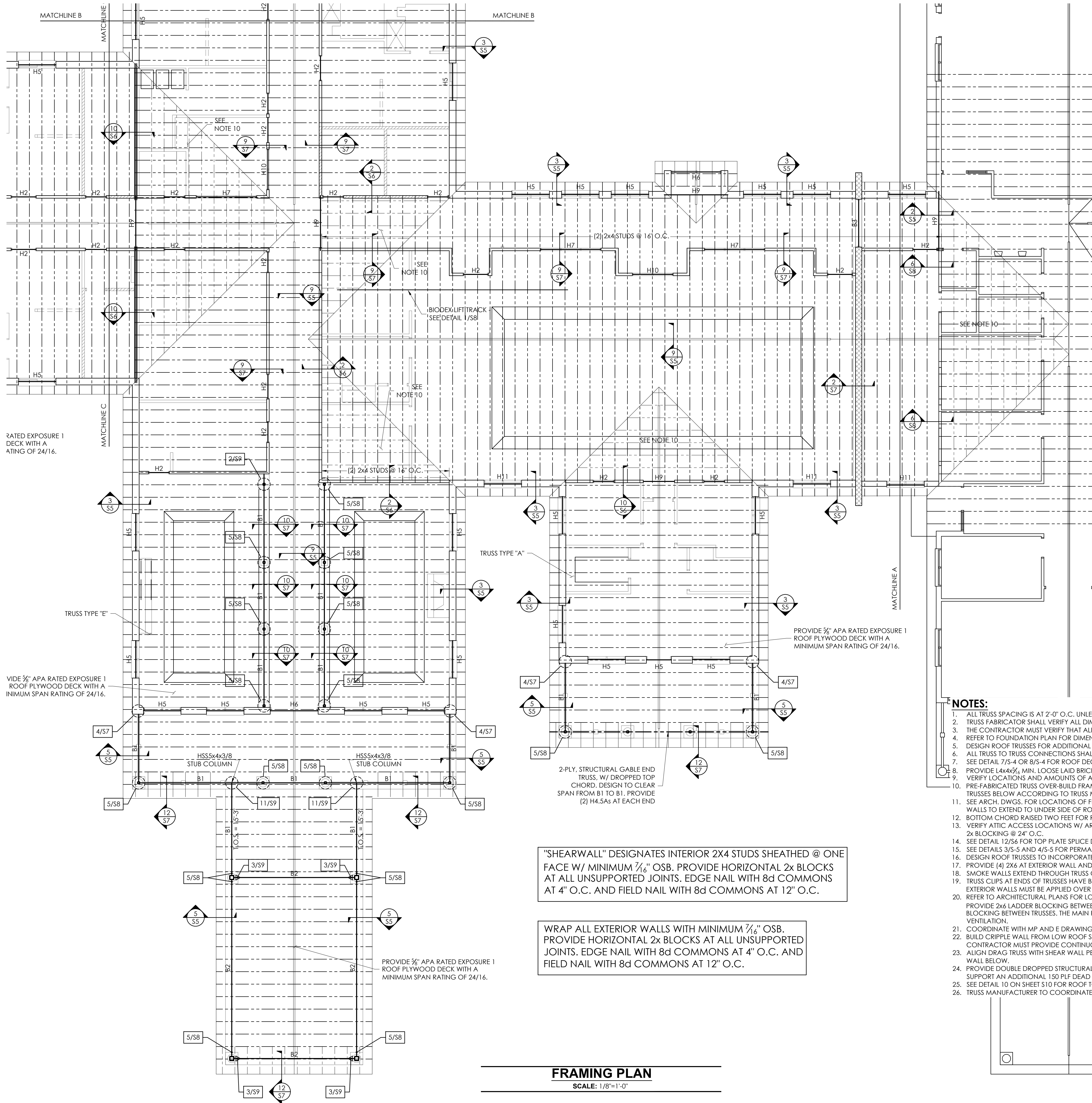
FOOTING SCHEDULE		
TYPE	SIZE	REBAR
F1	4'-0" X 4'-0" X 1'-0"	(4) #5s (3'-6" LONG) E.W.
F2	3'-0" X 3'-0" X 1'-0"	(3) #5s (2'-6" LONG) E.W. TOP OF FOOTING = -0'-8" F.F.E.
F3	3'-6" X 3'-6" X 1'-0"	(4) #5s (3'-0" LONG) E.W.
F4	6'-0" X 6'-0" X 1'-6"	(6) #6s (5'-6" LONG) E.W. T + B
F5	4'-0" X 4'-0" X 1'-0" THICKENED SLAB	(4) #5s (3'-6" LONG) E.W.

WRAP ALL EXTERIOR WALLS WITH MINIMUM 7/16" OSB. PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED JOINTS. EDGE NAIL WITH 8d COMMONS AT 4" O.C. AND FIELD NAIL WITH 8d COMMONS AT 12" O.C.

"SHEARWALL" DESIGNATES INTERIOR 2X4 STUDS SHEATHED W/ MINIMUM 7/16" OSB. PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED JOINTS. EDGE NAIL WITH 8d COMMONS AT 4" O.C. AND FIELD NAIL WITH 8d COMMONS AT 12" O.C.

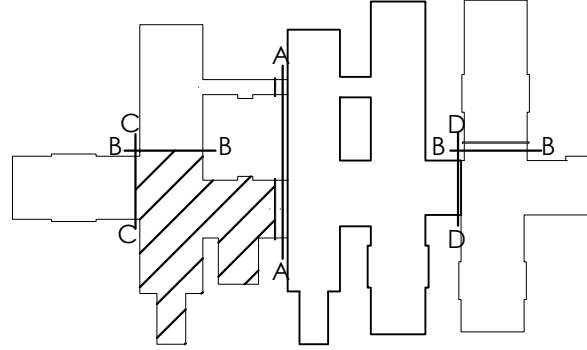


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HEADER AND BEAM SCHEDULE		
TYPE	SIZE	NOTES
H1	(2) 2x8	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S6
H2	(2) 2x10	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S6
H3	(2) 2x12	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S6
H4	(3) 2x8	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S6
H5	(3) 2x10	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S6
H6	(3) 2x12	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S6
H7	(2) 1 3/4" x 11 7/8" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0
H8	(3) 1 3/4" x 11 7/8" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0
H9	(2) 1 3/4" x 14" LVL FLUSH BEAM. BOTTOM OF BEAM FLUSH WITH BOTTOM OF ROOF TRUSSES	Fb=2850 PSI, E=2.0. STRAP ENDS OF BEAM TO STUD GROUP BELOW W/ (2) H6 TIES. PROVIDE HTT4 HOLDDOWN AT STUD BASE.
H10	(2) 1 3/4" x 9 1/4" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0
H11	(3) 1 3/4" x 9 1/4" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0
B1	W8x28 STEEL BEAM. T.O.S = 8'-10 1/2" AFF	
B2	W16x57 STEEL BEAM. T.O.S = 15'-3" AFF	
B3	(2) 8" DEEP BOND BEAMS	PROVIDE (2) #5 CONTINUOUS BARS IN EACH BOND BEAM
B4	C12 X 20.7	
B5	W8x40 STEEL BEAM. T.O.S = 8'-10 1/2" AFF	

- NOTES:**
- ALL TRUSS SPACING IS AT 2'-0" O.C. UNLESS NOTED OTHERWISE. SPACE TRUSSES AT ATTIC ACCESS DOORS TO ALLOW FOR PROPER INSTALLATION.
 - TRUSS FABRICATOR SHALL VERIFY ALL DIMENSIONS, LAYOUTS AND COORDINATE WITH BEARING WALL AND BEAM LOCATIONS. ALTERNATE LAYOUT PLANS MAY BE SUBMITTED FOR APPROVAL.
 - THE CONTRACTOR MUST VERIFY THAT ALL LATERAL BRACING REQUIRED FOR TRUSS WEBS IS INSTALLED PER THE TRUSS SHOP DRAWINGS AND DETAIL 4/S-5.
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 - SEE DETAIL 7/S-4 OR 8/S-4 FOR ROOF DECK NAILING PATTERN.
 - PROVIDE 14x4x1/4" MIN. LOOSE LAID BRICK LINTEL ABOVE ALL OPENINGS UP TO 8'-0" WHERE OPENINGS EXCEED 8'-0" PROVIDE 1/2" Ø THRU BOLTS TO HEADER FOR SUPPORT OF BRICK LINTEL.
 - VERIFY LOCATIONS AND AMOUNTS OF ALL HEADERS.
 - PRE-FABRICATED TRUSS OVER-BUILD FRAMING. ROOF SHEATHING SHALL BE CONTINUOUS BENEATH TRUSS OVERBUILD. PROVIDE ATTACHMENT OF OVERBUILD FRAMING TO ROOF SHEATHING AND TRUSSES BELOW ACCORDING TO TRUSS MANUFACTURER.
 - SEE ARCH. DWGS. FOR LOCATIONS OF FIRE/SMOKE WALLS AND DRAFT PARTITIONS. TRUSSES MUST BE COORDINATED WITH FIRE/SMOKE WALLS. WHERE ARCHITECTURAL PLANS REQUIRE SMOKE/FIRE WALLS TO EXTEND TO UNDER SIDE OF ROOF SHEATHING, THE TRUSSES MUST BE STOP AT THE FACE OF THE WALL.
 - BOTTOM CHORD RAISED TWO FEET FOR RECESSED CEILING - DASHED LINE SHOWS APPROXIMATE LOCATION. VERIFY ALL LOCATIONS WITH ARCH DWGS.
 - VERIFY ATTIC ACCESS LOCATIONS W/ ARCH. DWGS. SPACE TRUSSES AS REQUIRED FOR PROPER INSTALLATION. WHERE TRUSS SPACING EXCEEDS 24" O.C., LADDER BLOCK BETWEEN CHORDS WITH 2x BLOCKING @ 24" O.C.
 - SEE DETAIL 12/S6 FOR TOP PLATE SPLICE DETAIL.
 - SEE DETAILS 3/S-5 AND 4/S-5 FOR PERMANENT ROOF TRUSS BRACING.
 - DESIGN ROOF TRUSSES TO INCORPORATE FIXED WINDOW INSTALLATION. COORDINATE WITH ARCHITECTURAL DRAWINGS.
 - PROVIDE (4) 2X6 AT EXTERIOR WALL AND (5) 2X4 @ INTERIOR WALL BELOW ALL GIRDER TRUSS BEARING POINTS PROVIDE LGT TIE DOWN WITH HTT4 AT STUD BASE.
 - SMOKE WALLS EXTEND THROUGH TRUSS OVERBUILD TO ROOF SHEATHING. BREAK TRUSS OVERBUILD ON BOTH SIDES OF WALL.
 - TRUSS CLIPS AT ENDS OF TRUSSES HAVE BEEN DESIGNED TO TRANSFER LATERAL SHEAR LOAD AND UPLIFT INTO THE WALLS. ANY SUBSTITUTIONS MUST BE APPROVED BY THE EOR. H10A TIE DOWNS AT EXTERIOR WALLS MUST BE APPLIED OVER THE EXTERIOR WALL OSB SHEATHING.
 - REFER TO ARCHITECTURAL PLANS FOR LOCATION OF DORMERS ON MAIN ROOF. DORMERS SHALL BE FRAMED USING 2x4 STUDS AT 16" O.C. WITH 2X4 RAFTERS AND COLLAR TIES AT 24" O.C. PROVIDE 2x6 LADDER BLOCKING BETWEEN TRUSSES FOR ATTACHMENT OF THE DORMER WALL SILL PLATE. PROVIDE A MINIMUM OF (1) 1/2" X 3" WOOD SCREW AT 24" O.C. FROM DORMER SILL TO BLOCKING BETWEEN TRUSSES. THE MAIN ROOF SHEATHING MUST EXTEND BELOW DORMER. IF REQUIRED CUT A MAXIMUM 20"x36" HOLE IN THE MAIN ROOF SHEATHING BELOW THE DORMER FOR VENTILATION.
 - COORDINATE WITH MP AND E DRAWINGS FOR THE ROOF TOP PLATFORM AND EXTERIOR LADDER LOCATION. SEE SHEET S8 FOR DETAILS.
 - BUILD CRIPPLE WALL FROM LOW ROOF SHEATHING TO BOTTOM OF CANOPY TRUSS. PROVIDE 2X6 LADDER BLOCKING AT 24" O.C. BETWEEN LOW ROOF TRUSSES UNDER CRIPPLE WALL. CONTRACTOR MUST PROVIDE CONTINUOUS UPLIFT CONNECTIONS.
 - ALIGN DRAG TRUSS WITH SHEAR WALL PER DETAIL 4/S5. DESIGN DRAG TRUSS TO TRANSFER 200 PLF LATERAL LOAD FROM TOP CHORD TO BOTTOM CHORD. LATERAL LOAD IS RESISTED BY SHEAR WALL BELOW.
 - PROVIDE DOUBLE DROPPED STRUCTURAL GABLE END TRUSS AT END OF PORCH ROOF. HANG SOFFIT FRAMING FROM BOTTOM CHORD OF TRUSSES. TRUSS DESIGNER TO DESIGN GABLE END TO SUPPORT AN ADDITIONAL 150 PLF DEAD LOAD AT THE BOTTOM CHORD.
 - SEE DETAIL 10 ON SHEET S10 FOR ROOF TOP CURB ATTACHMENTS.
 - TRUSS MANUFACTURER TO COORDINATE FIXED WINDOW OPENINGS IN GABLE END TRUSSES - SEE ARCH ELEVATIONS.



KEY PLAN



PLAN NORTH



PRUITT HEALTH
TOWN CENTER
HARRISBURG, NC

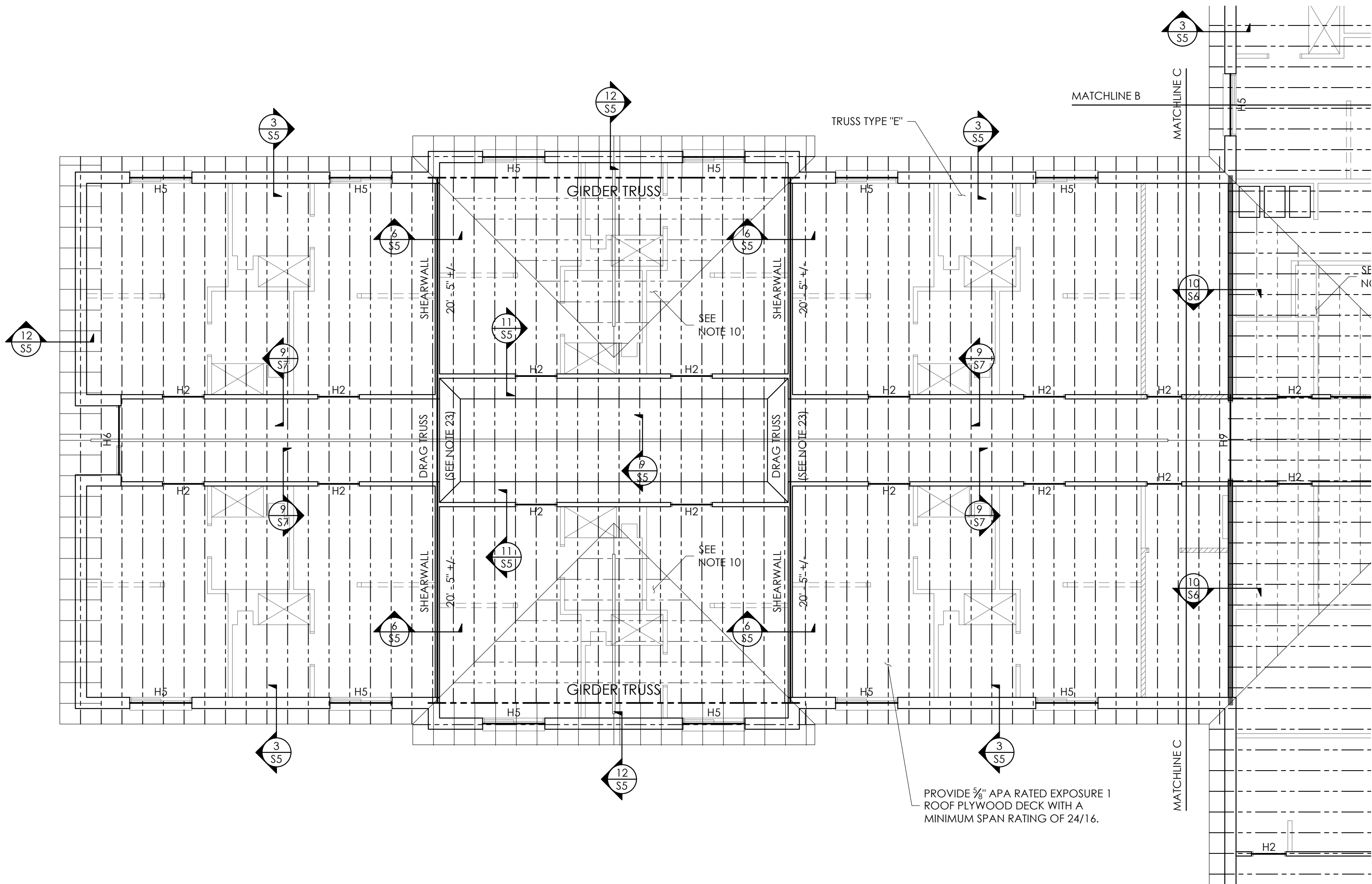
David R. Polston - Architect
3806 Park Ave. Suite 2-L, Wilmington, NC 28403
Architecture Planning Design

59 BED
ADDITION /
RENOVATION

ISSUE DATE: 06.18.2025

REV	DATE

S3.1



FRAMING PLAN
SCALE: 1/8"=1'-0"

NOTES:

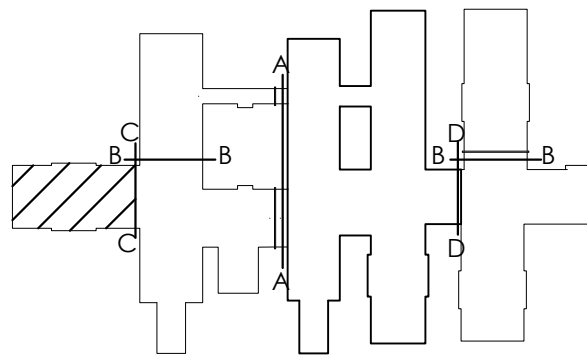
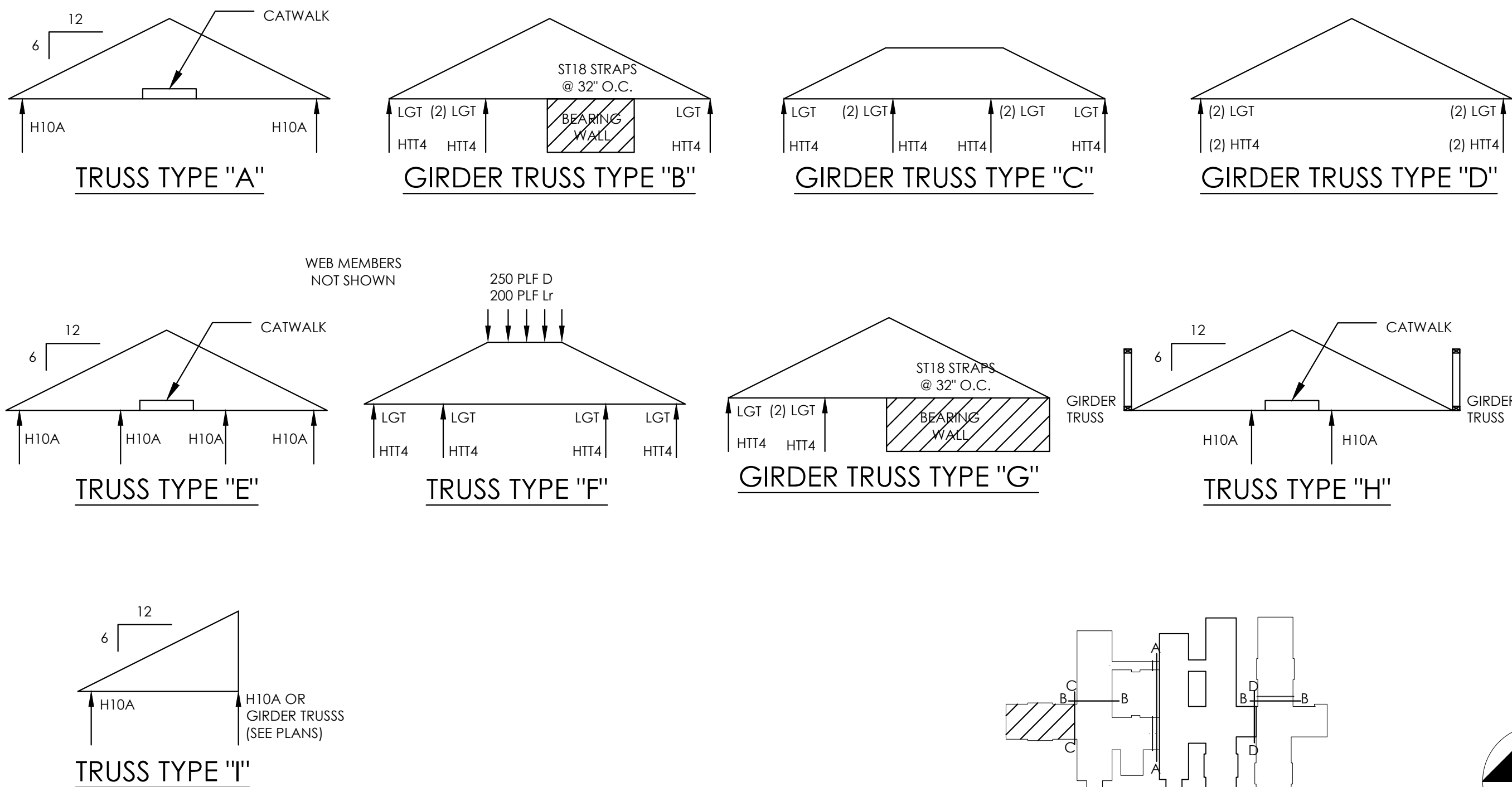
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HEADER AND BEAM SCHEDULE

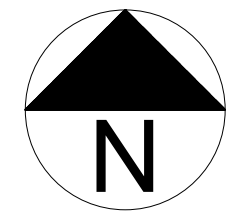
TYPE	SIZE	NOTES
H1	(2) 2x8	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S6
H2	(2) 2x10	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S6
H3	(2) 2x12	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S6
H4	(3) 2x8	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S6
H5	(3) 2x10	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S6
H6	(3) 2x12	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S6
H7	(2) 1 3/4" x 11 7/8" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0
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H9	(2) 1 3/4" x 14" LVL FLUSH BEAM. BOTTOM OF BEAM FLUSH WITH BOTTOM OF ROOF TRUSSES	Fb=2850 PSI, E=2.0. STRAP ENDS OF BEAM TO STUD GROUP BELOW W/ (2) H6 TIES. PROVIDE HTT4 HOLDDOWN AT STUD BASE.
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B1	W8x28 STEEL BEAM. T.O.S = 8'-10 1/2" AFF	
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"SHEARWALL" DESIGNATES INTERIOR 2X4 STUDS SHEATHED @ ONE FACE W/ MINIMUM $\frac{7}{16}$ " OSB. PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED JOINTS. EDGE NAIL WITH 8d COMMONS AT 4" O.C. AND FIELD NAIL WITH 8d COMMONS AT 12" O.C.

WRAP ALL EXTERIOR WALLS WITH MINIMUM $\frac{7}{16}$ " OSB. PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED JOINTS. EDGE NAIL WITH 8d COMMONS AT 4" O.C. AND FIELD NAIL WITH 8d COMMONS AT 12" O.C.



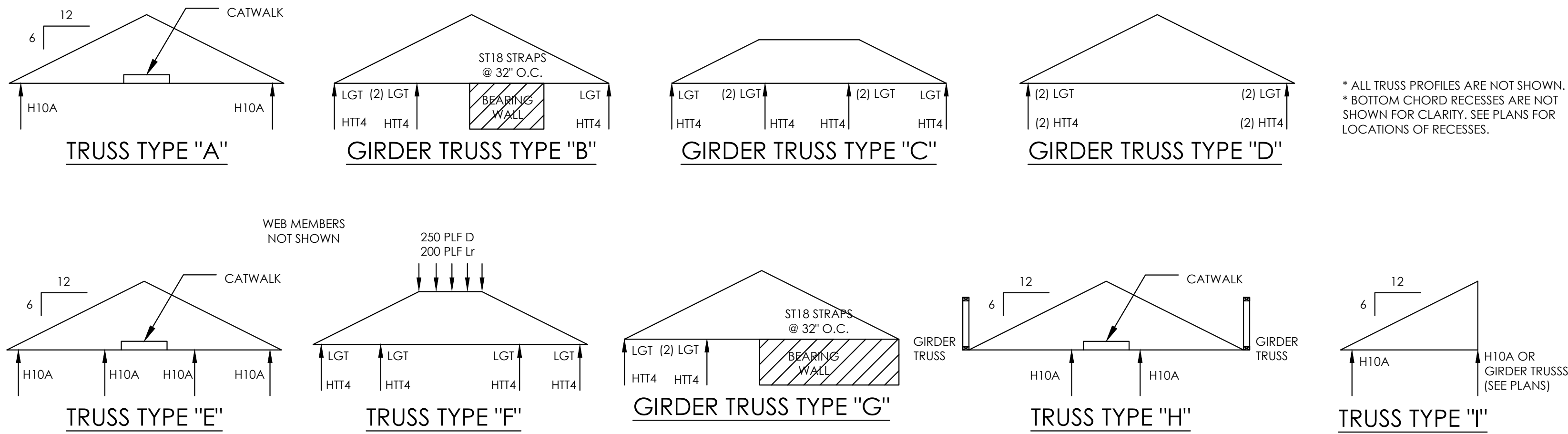
KEY PLAN



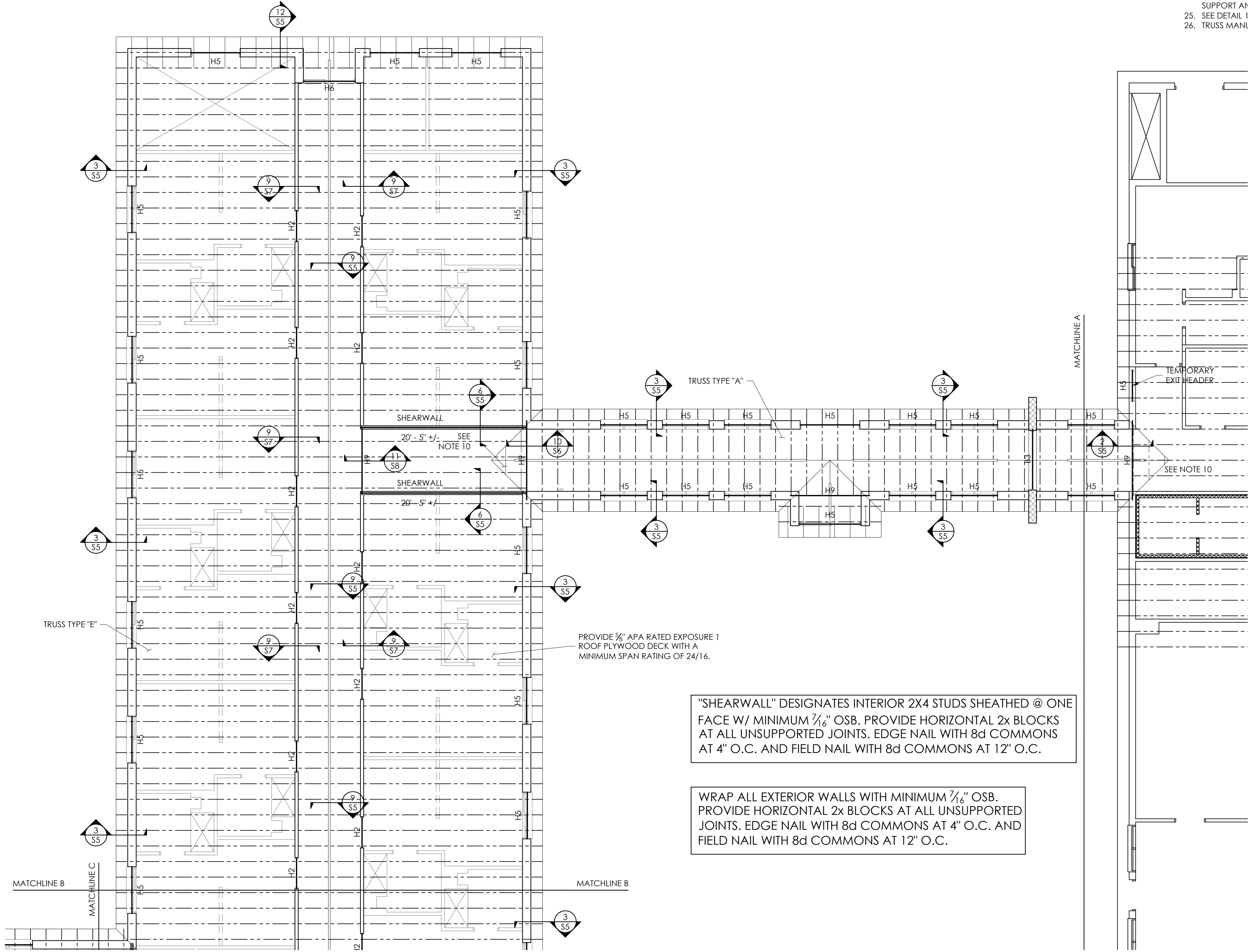
PLAN NORTH



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 7. SEE DETAIL 7/S-4 OR 8/S-4 FOR ROOF DECK NAILING PATTERN.
 8. PROVIDE $4 \times 4 \times \frac{1}{4}$ MIN. LOOSE LAID BRICK LINTEL ABOVE ALL OPENINGS UP TO 8'-0" WHERE OPENINGS EXCEED 8'-0" PROVIDE $\frac{1}{2}$ " Ø THRU BOLTS TO HEADER FOR SUPPORT OF BRICK LINTEL.
 9. VERIFY LOCATIONS AND AMOUNTS OF ALL HEADERS.
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 11. SEE ARCH. DWGS. FOR LOCATIONS OF FIRE/SMOKE WALLS AND DRAFT PARTITIONS. TRUSSES MUST BE COORDINATED WITH FIRE/SMOKE WALLS. WHERE ARCHITECTURAL PLANS REQUIRE SMOKE/FIRE WALLS TO EXTEND TO UNDER SIDE OF ROOF SHEATHING, THE TRUSSES MUST BE STOP AT THE FACE OF THE WALL.
 12. BOTTOM CHORD RAISED TWO FEET FOR RECESSED CEILING - DASHED LINE SHOWS APPROXIMATE LOCATION. VERIFY ALL LOCATIONS WITH ARCH DWGS.
 13. VERIFY ATTIC ACCESS LOCATIONS W/ ARCH. DWGS. SPACE TRUSSES AS REQUIRED FOR PROPER INSTALLATION. WHERE TRUSS SPACING EXCEEDS 24" O.C., LADDER BLOCK BETWEEN CHORDS WITH 2x BLOCKING @ 24" O.C.
 14. SEE DETAIL 12/S6 FOR TOP PLATE SPICE DETAIL.
 15. SEE DETAILS 3/S-5 AND 4/S-5 FOR PERMANENT ROOF TRUSS BRACING.
 16. DESIGN ROOF TRUSSES TO INCORPORATE FIXED WINDOW INSTALLATION. COORDINATE WITH ARCHITECTURAL DRAWINGS.
 17. PROVIDE (4) 2x6 AT EXTERIOR WALL AND (5) 2x4 @ INTERIOR WALL BELOW ALL GIRDER TRUSS BEARING POINTS PROVIDE LGT TIE DOWN WITH HIT4 AT STUD BASE.
 18. SMOKE WALLS EXTEND THROUGH TRUSS OVERBUILD TO ROOF SHEATHING. BREAK TRUSS OVERBUILD ON BOTH SIDES OF WALL.
 19. TRUSS CLIPS AT ENDS OF TRUSSES HAVE BEEN DESIGNED TO TRANSFER LATERAL SHEAR LOAD AND UPLIFT INTO THE WALLS. ANY SUBSTITUTIONS MUST BE APPROVED BY THE EOR. H10A TIE DOWNS AT EXTERIOR WALLS MUST BE APPLIED OVER THE EXTERIOR WALL OSB SHEATHING.
 20. REFER TO ARCHITECTURAL PLANS FOR LOCATION OF DORMERS ON MAIN ROOF. DORMERS SHALL BE FRAMED USING 2x4 STUDS AT 16" O.C. WITH 2x4 RAFTERS AND COLLAR TIES AT 24" O.C. PROVIDE 2x6 LADDER BLOCKING BETWEEN TRUSSES FOR ATTACHEMENT OF THE DORMER WALL SILL PLATE. PROVIDE A MINIMUM OF (1) $\frac{1}{2}$ " X 3" WOOD SCREW AT 24" O.C. FROM DORMER SILL TO BLOCKING BETWEEN TRUSSES. THE MAIN ROOF SHEATHING MUST EXTEND BELOW DORMER. IF REQUIRED CUT A MAXIMUM 20"x36" HOLE IN THE MAIN ROOF SHEATHING BELOW THE DORMER FOR VENTILATION.
 21. COORDINATE WITH MP AND E DRAWINGS FOR THE ROOF TOP PLATFORM AND EXTERIOR LADDER LOCATION. SEE SHEET S8 FOR DETAILS.
 22. BUILD CRIPPLE WALL FROM LOW ROOF SHEATHING TO BOTTOM OF CANOPY TRUSS. PROVIDE 2x6 LADDER BLOCKING AT 24" O.C. BETWEEN LOW ROOF TRUSSES UNDER CRIPPLE WALL. CONTRACTOR MUST PROVIDE CONTINUOUS UPLIFT CONNECTIONS.
 23. ALIGN DRAG TRUSS WITH SHEAR WALL PER DETAIL 6/S5. DESIGN DRAG TRUSS TO TRANSFER 200 PLF LATERAL LOAD FROM TOP CHORD TO BOTTOM CHORD. LATERAL LOAD IS RESISTED BY SHEAR WALL BELOW.
 24. PROVIDE DOUBLE DROPPED STRUCTURAL GABLE END TRUSS AT END OF PORCH ROOF. HANG SOFFIT FRAMING FROM BOTTOM CHORD OF TRUSSES. TRUSS DESIGNER TO DESIGN GABLE END TO SUPPORT AN ADDITIONAL 150 PLF DEAD LOAD AT THE BOTTOM CHORD.
 25. SEE DETAIL 10 ON SHEET S10 FOR ROOF TOP CURB ATTACHMENTS.
 26. TRUSS MANUFACTURER TO COORDINATE FIXED WINDOW OPENINGS IN GABLE END TRUSSES - SEE ARCH ELEVATIONS.

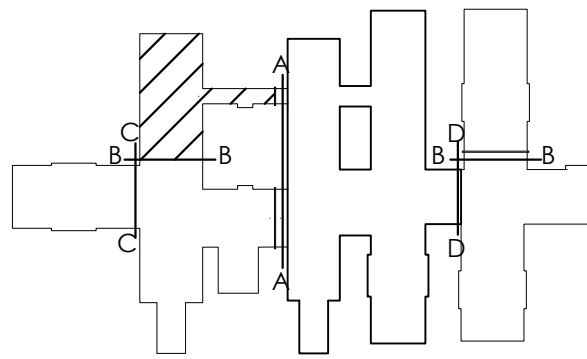


"SHEARWALL" DESIGNATES INTERIOR 2X4 STUDS SHEATHED @ ONE FACE W/ MINIMUM $\frac{1}{16}$ " OSB. PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED JOINTS. EDGE NAIL WITH 8d COMMONS AT 4" O.C. AND FIELD NAIL WITH 8d COMMONS AT 12" O.C.

WRAP ALL EXTERIOR WALLS WITH MINIMUM $\frac{1}{16}$ " OSB. PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED JOINTS. EDGE NAIL WITH 8d COMMONS AT 4" O.C. AND FIELD NAIL WITH 8d COMMONS AT 12" O.C.

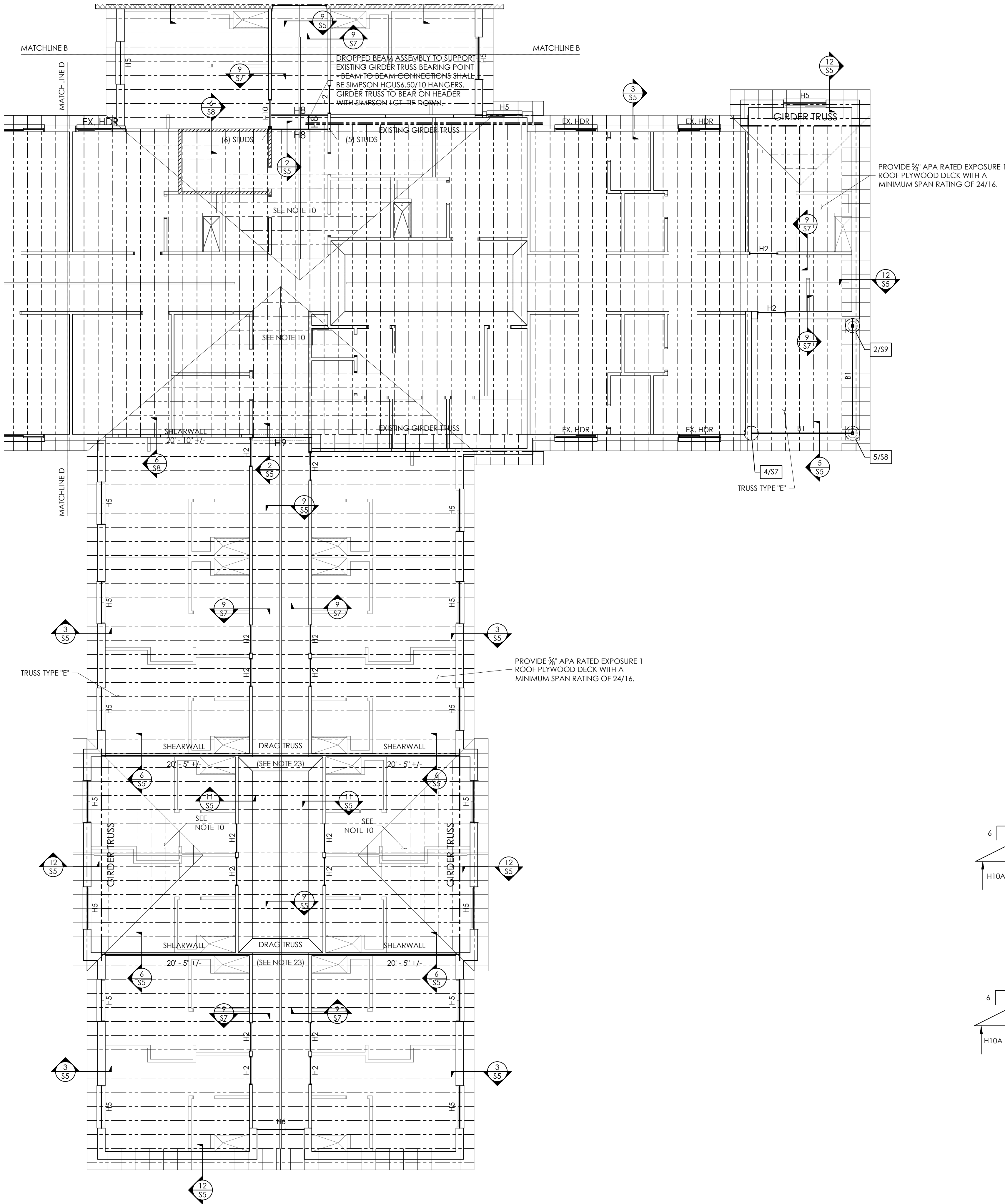
FRAMING PLAN
SCALE: 1/8"=1'-0"

HEADER AND BEAM SCHEDULE		
TYPE	SIZE	NOTES
H1	(2) 2x8	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S6
H2	(2) 2x10	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S6
H3	(2) 2x12	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S6
H4	(3) 2x8	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S6
H5	(3) 2x10	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S6
H6	(3) 2x12	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S6
H7	(2) 1 3/4" x 11 7/8" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0
H8	(3) 1 3/4" x 11 7/8" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0
H9	(2) 1 3/4" x 14" LVL FLUSH BEAM. BOTTOM OF BEAM FLUSH WITH BOTTOM OF ROOF TRUSSES	Fb=2850 PSI, E=2.0. STRAP ENDS OF BEAM TO STUD GROUP BELOW W/ (2) H6 TIES. PROVIDE HIT4 HOLDDOWN AT STUD BASE.
H10	(2) 1 3/4" x 9 1/4" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0
H11	(3) 1 3/4" x 9 1/4" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0
B1	W8x28 STEEL BEAM. T.O.S = 8'-10 1/2" AFF	
B2	W16x57 STEEL BEAM. T.O.S = 15'-3" AFF	
B3	(2) 8" DEEP BOND BEAMS	PROVIDE (2) #5 CONTINUOUS BARS IN EACH BOND BEAM
B4	C12 X 20.7	
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KEY PLAN





FRAMING PLAN
SCALE: 1/8"=1'-0"

NOTES:

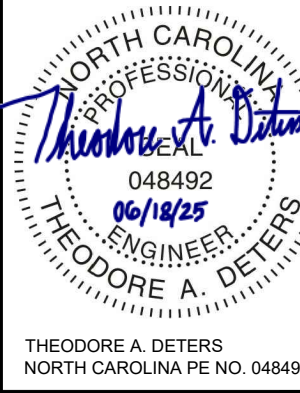
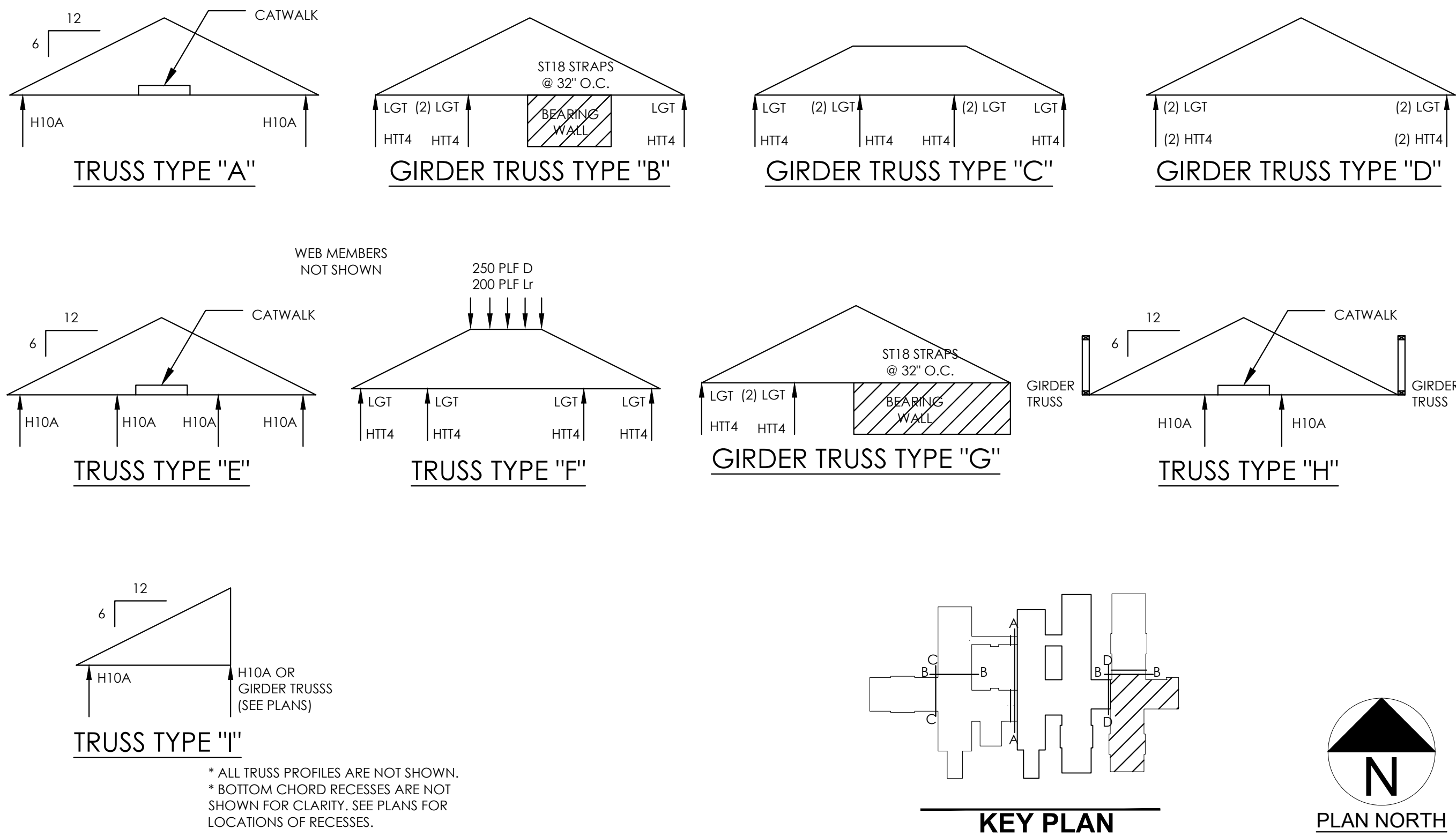
1. ALL TRUSS SPACING IS AT 2'-0" O.C. UNLESS NOTED OTHERWISE. SPACE TRUSSES AT ATTIC ACCESS DOORS TO ALLOW FOR PROPER INSTALLATION.
2. TRUSS FABRICATOR SHALL VERIFY ALL DIMENSIONS, LAYOUTS AND COORDINATE WITH BEARING WALL AND BEAM LOCATIONS. ALTERNATE LAYOUT PLANS MAY BE SUBMITTED FOR APPROVAL.
3. THE CONTRACTOR MUST VERIFY THAT ALL LATERAL BRACING REQUIRED FOR TRUSS WEBS IS INSTALLED PER THE TRUSS SHOP DRAWINGS AND DETAIL 4/S-5.
4. REFER TO FOUNDATION PLAN FOR DIMENSIONS AND TO ARCHITECTURAL PLANS FOR DIMENSIONS NOT SHOWN.
5. DESIGN ROOF TRUSSES FOR ADDITIONAL MECHANICAL, SPRINKLER, AND ARCHITECTURAL LOADS AS REQUIRED.
6. ALL TRUSS TO TRUSS CONNECTIONS SHALL BE SPECIFIED BY THE TRUSS DESIGNER AND SHALL BE CLEARLY INDICATED ON THE TRUSS SHOP DRAWINGS.
7. SEE DETAIL 7/S-4 OR 8/S-4 FOR ROOF DECK NAILING PATTERN.
8. PROVIDE 14x4x $\frac{1}{4}$ " MIN. LOOSE LAID BRICK LINTEL ABOVE ALL OPENINGS UP TO 8'-0" WHERE OPENINGS EXCEED 8'-0" PROVIDE $\frac{1}{2}$ " Ø THRU BOLTS TO HEADER FOR SUPPORT OF BRICK LINTEL.
9. VERIFY LOCATIONS AND AMOUNTS OF ALL HEADERS.
10. PRE-FABRICATED TRUSS OVERBUILD FRAMING. ROOF SHEATHING SHALL BE CONTINUOUS BENEATH TRUSS OVERBUILD. PROVIDE ATTACHMENT OF OVERBUILD FRAMING TO ROOF SHEATHING AND TRUSSES BELOW ACCORDING TO TRUSS MANUFACTURER.
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HEADER AND BEAM SCHEDULE

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H4	(3) 2x8	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S6
H5	(3) 2x10	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S6
H6	(3) 2x12	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S6
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TOWN CENTER
HARRISBURG, NC

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Architecture Planning Design

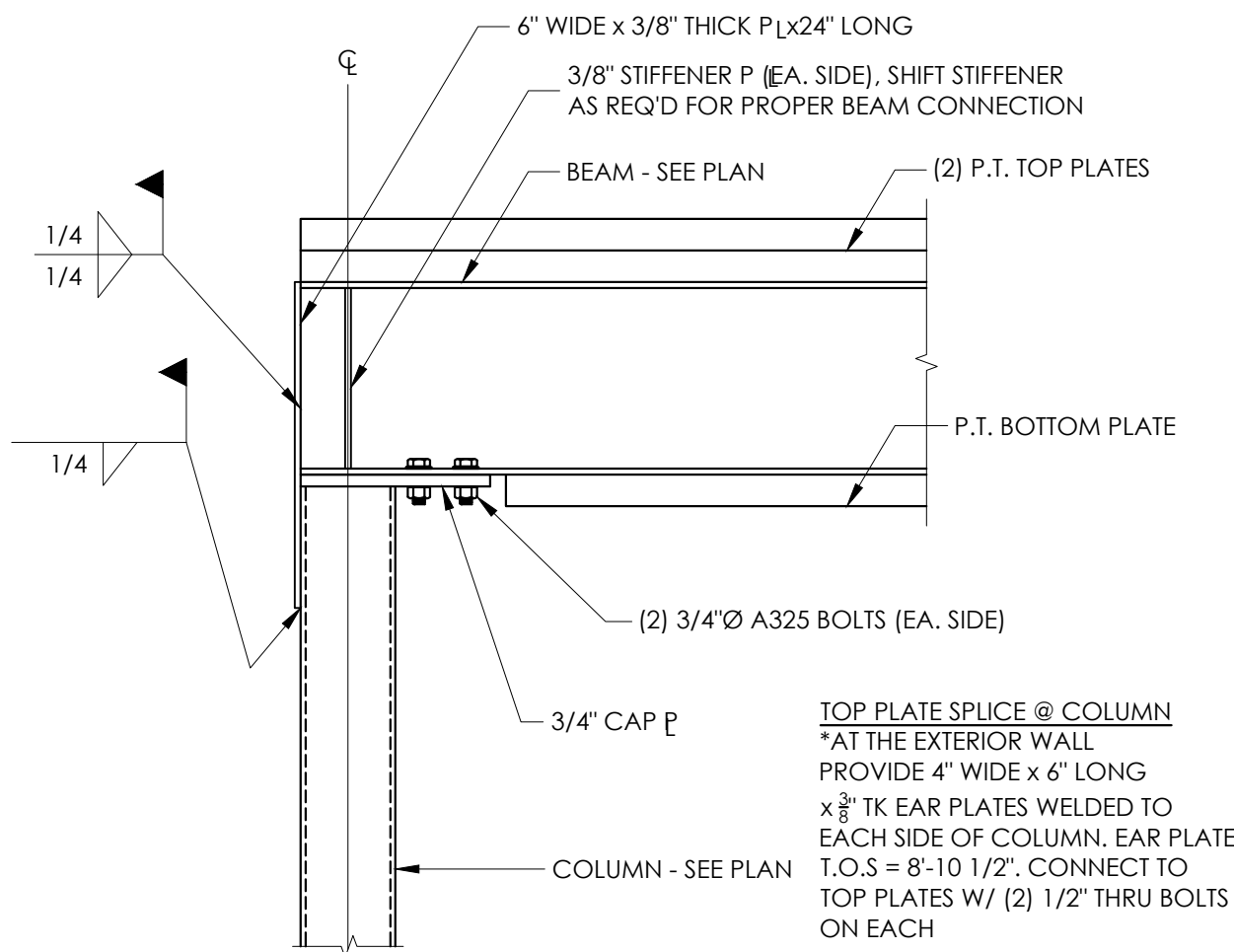
59 BED
ADDITION /
RENOVATION

ISSUE DATE: 06.18.2025

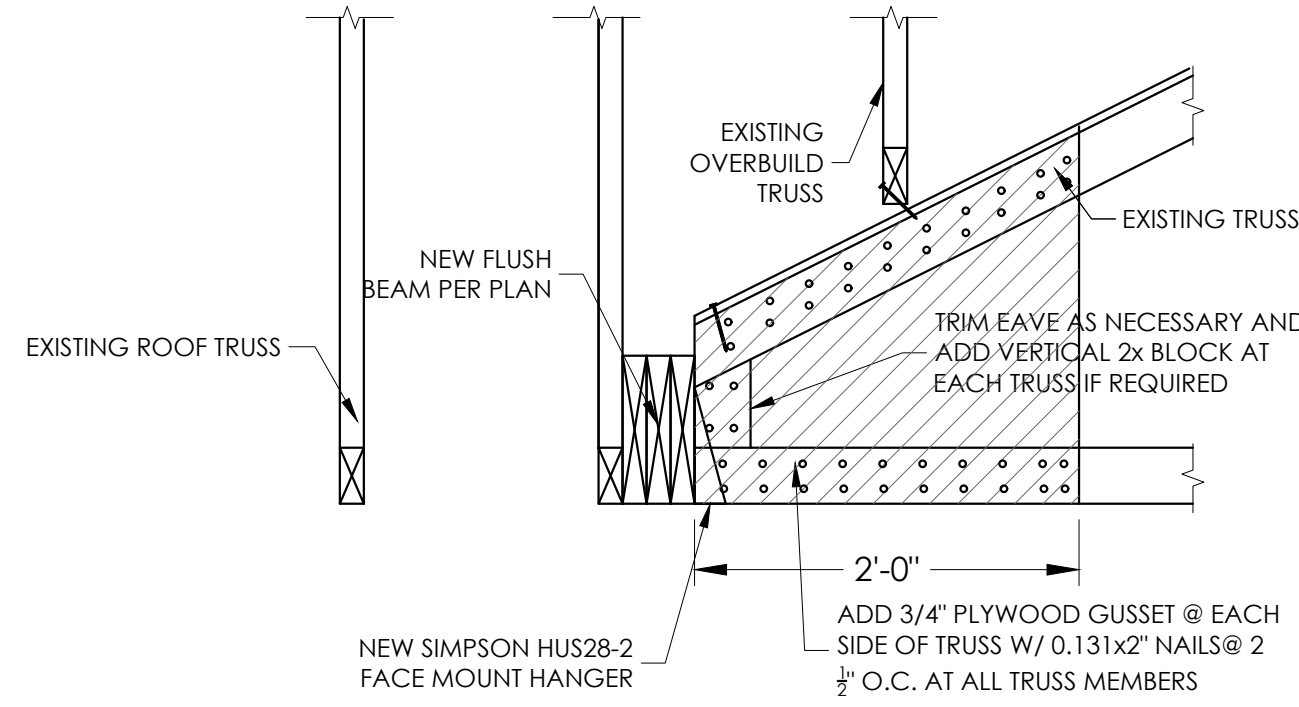
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S3.4

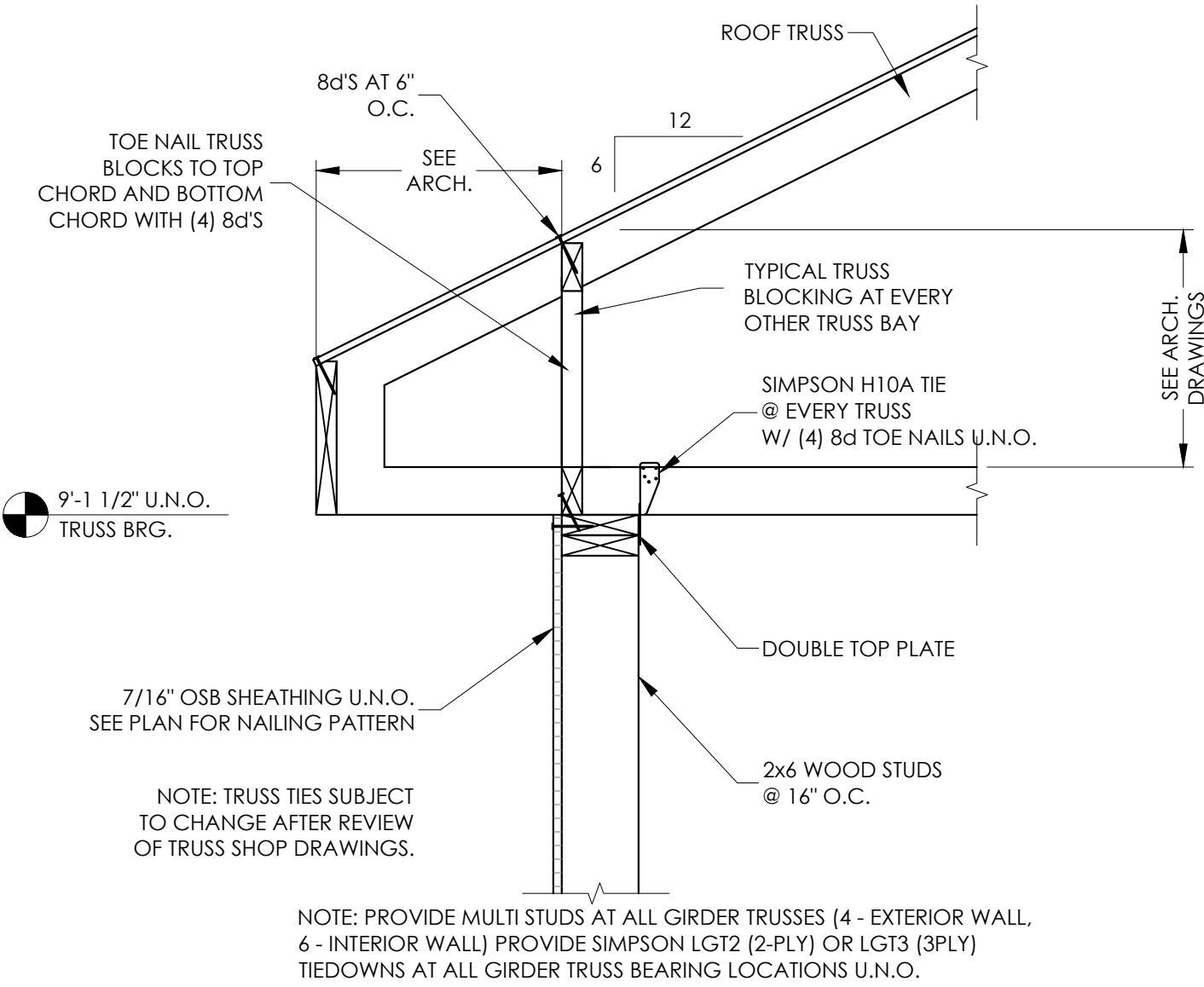




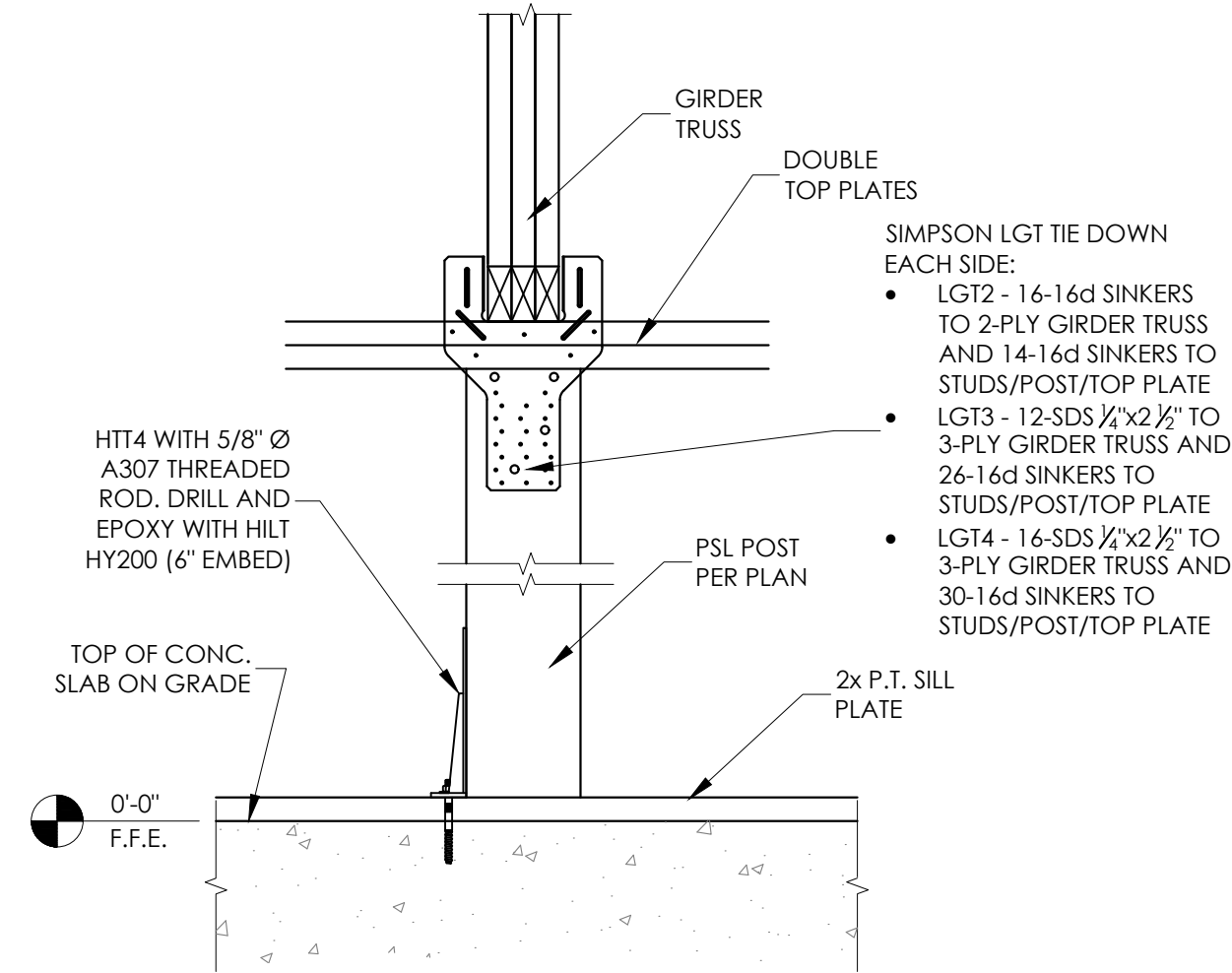
1 BEAM DETAIL @ COLUMN
SCALE: NONE



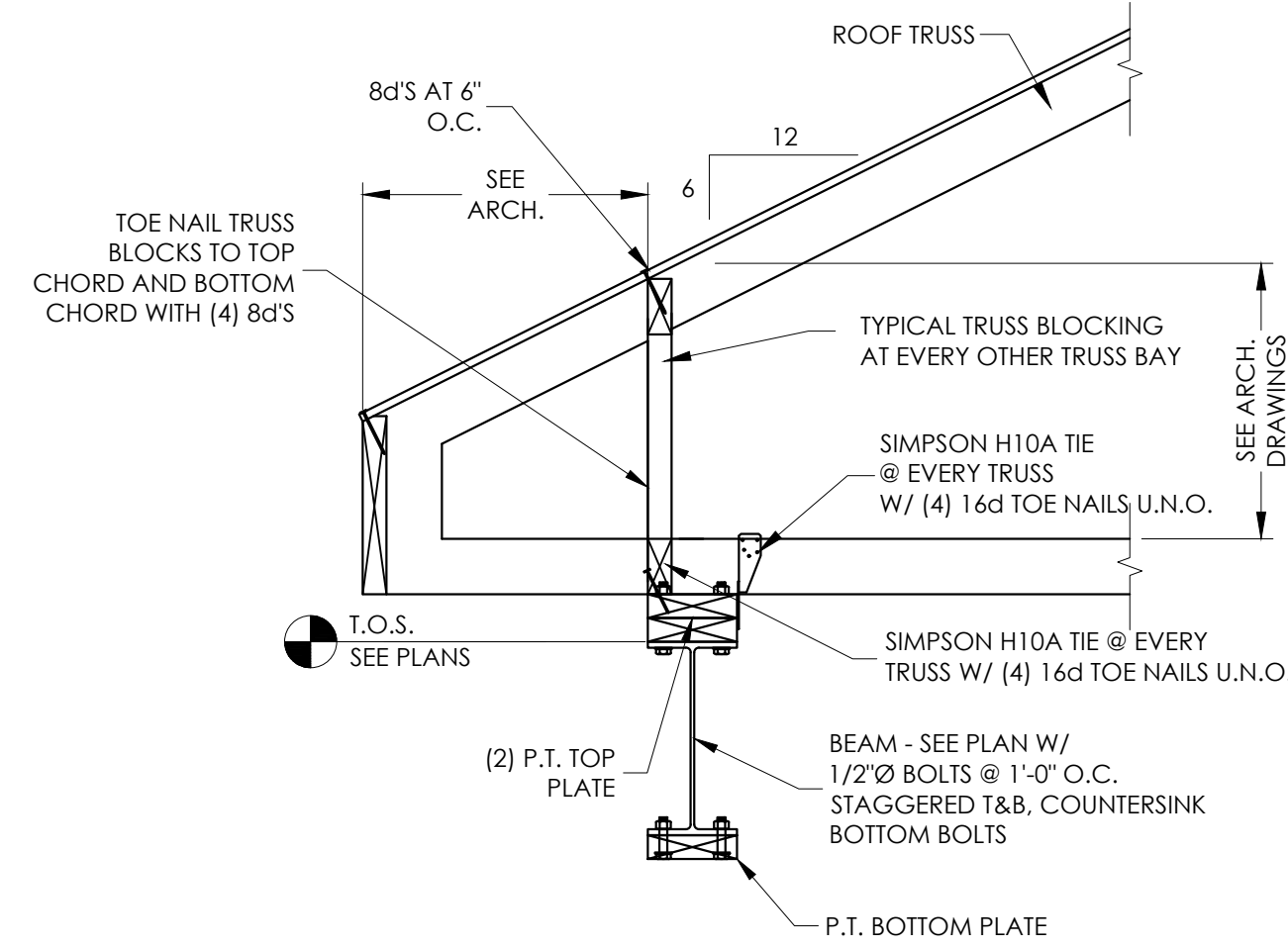
2 EXISTING TRUSS AT NEW FLUSH BEAM
SCALE: NONE



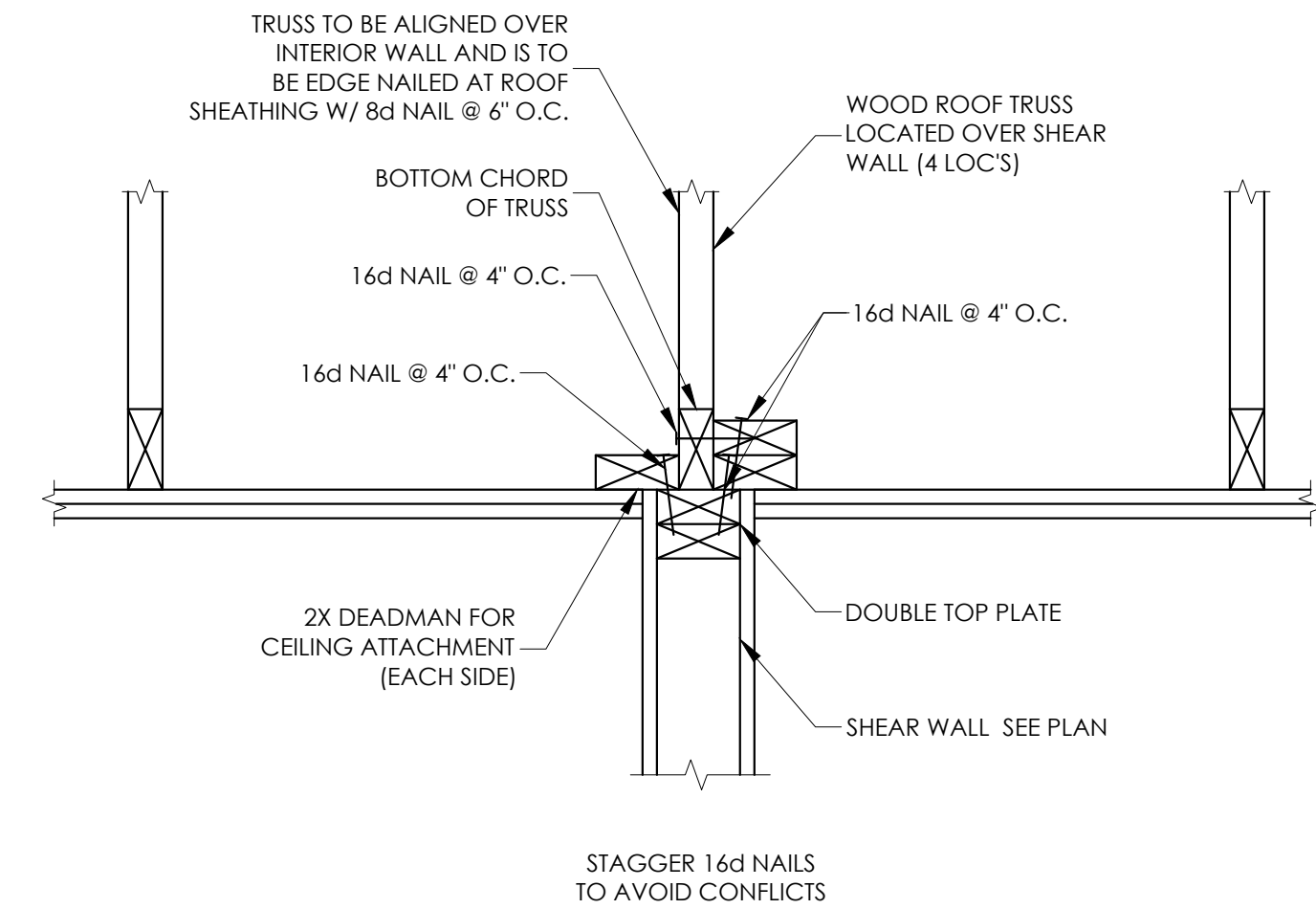
3 PREFAB. TRUSS @ EXT. WALL
SCALE: NONE



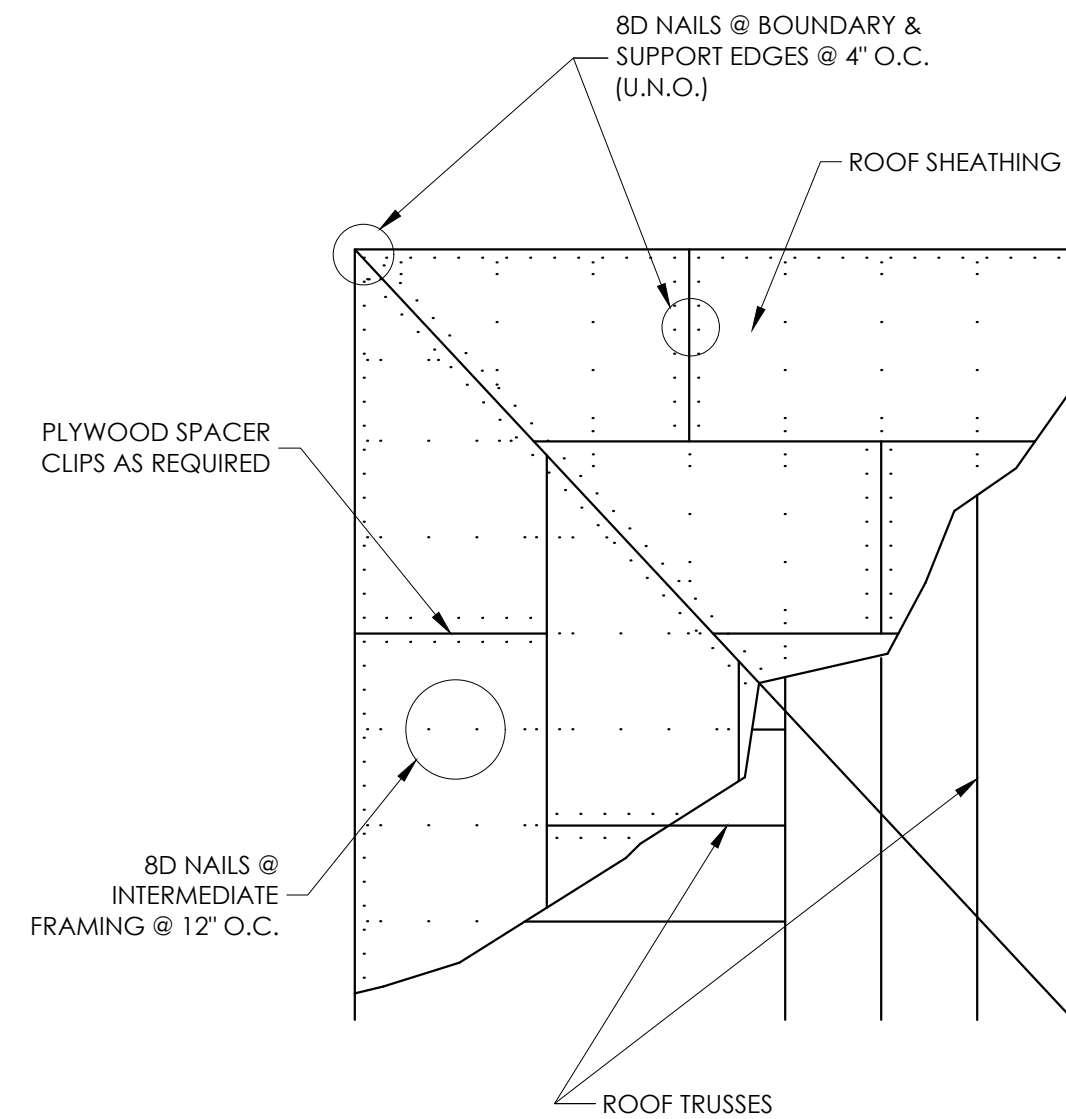
4 GIRDER TIE DOWN
SCALE: NONE



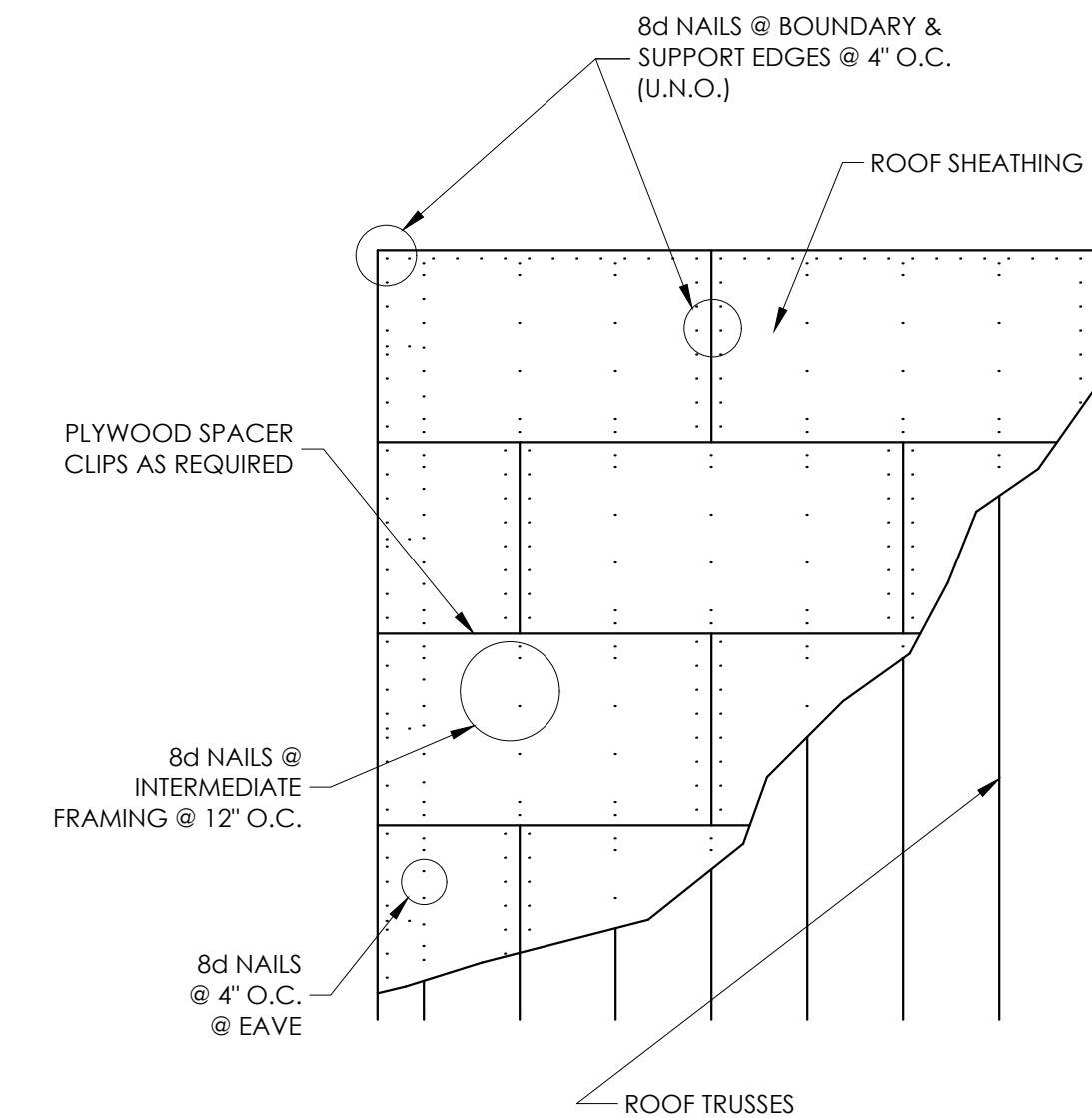
5 TRUSS BEARING @ BEAM
SCALE: NONE



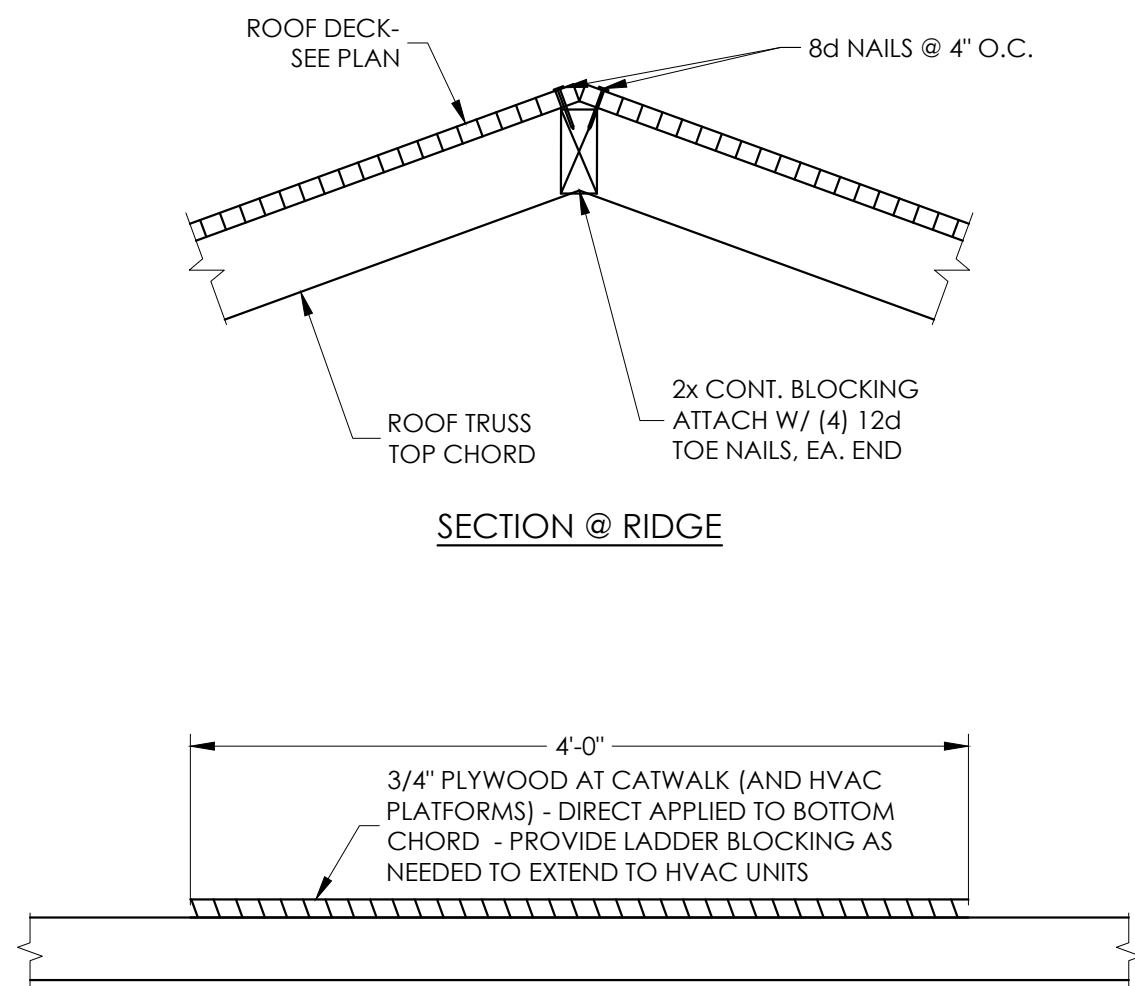
6 SHEARWALL AT DRAG TRUSS
SCALE: NONE



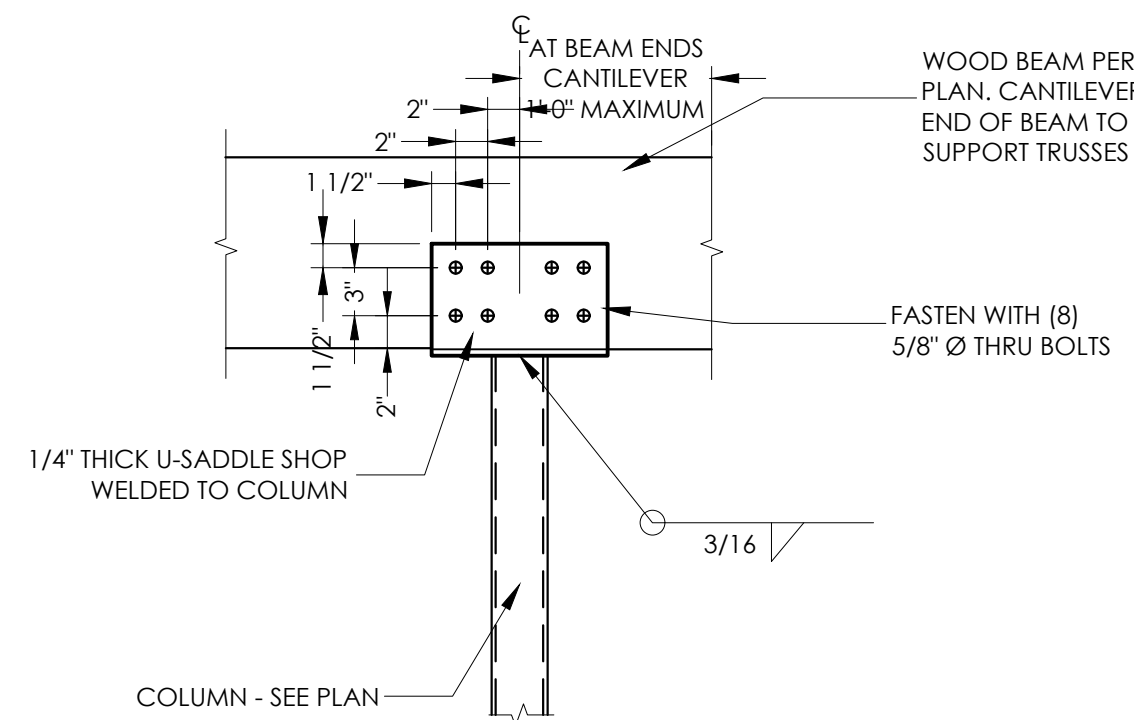
7 ROOF DECK NAILING PATTERN
SCALE: NONE



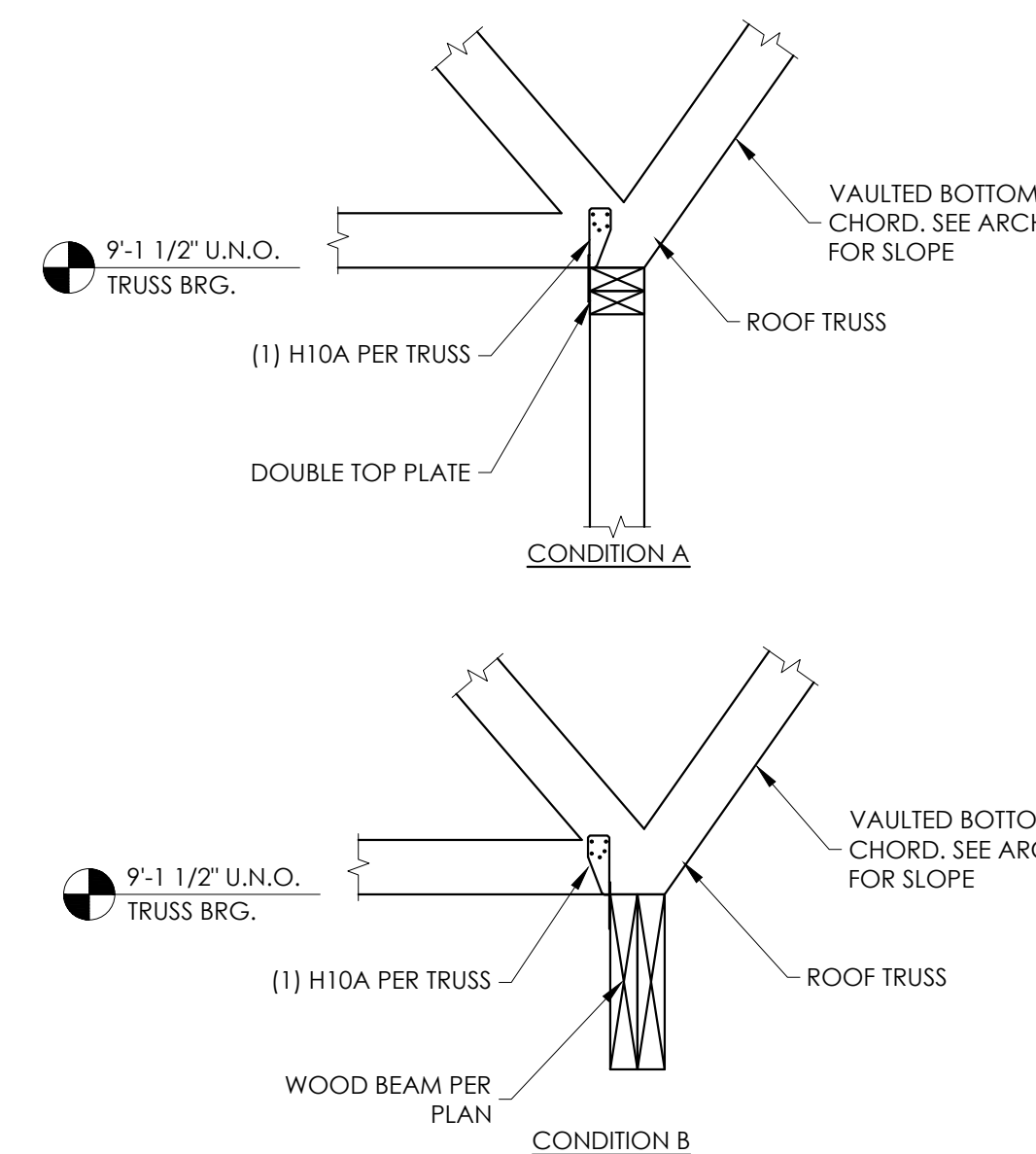
8 ROOF DECK NAILING PATTERN
SCALE: NONE



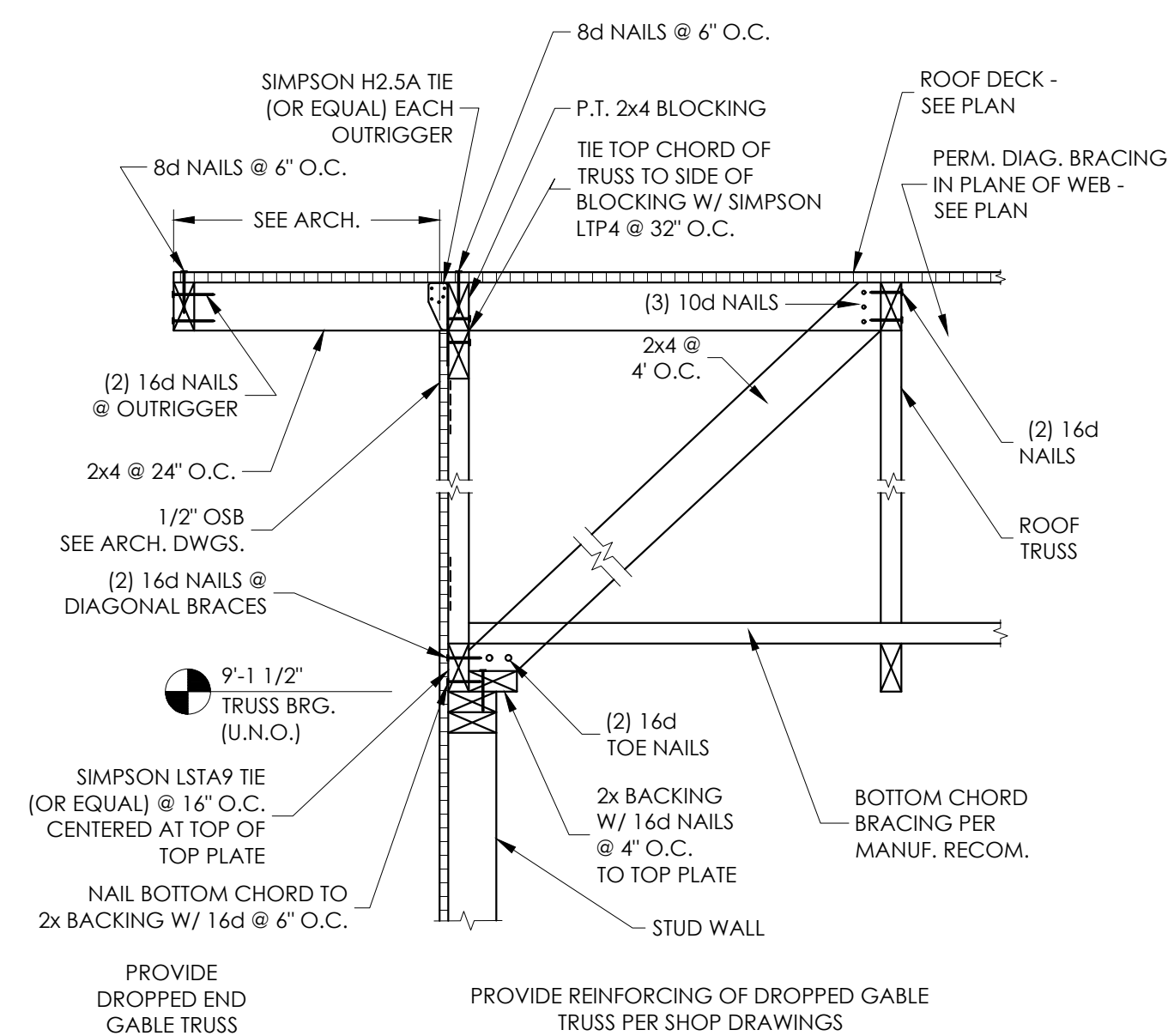
9 SECTION AT RIDGE AND CATWALK
SCALE: NONE



10 WOOD BEAM ON STEEL COLUMN
SCALE: NONE



11 TRUSS @ INTERIOR BEAM BEARING
SCALE: NONE



12 RAKE SECTION @ EXT. WALL
SCALE: NONE

HAUSER-CREECH, INC.
PROJECT #: 25-001-001



THEODORE A. DETERS
NORTH CAROLINA PE NO. 048492



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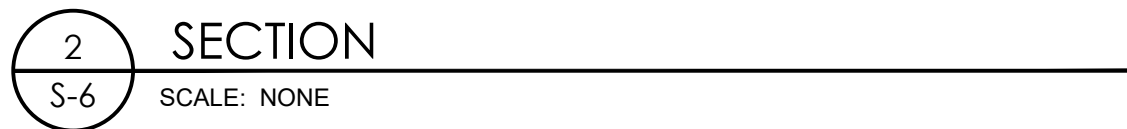
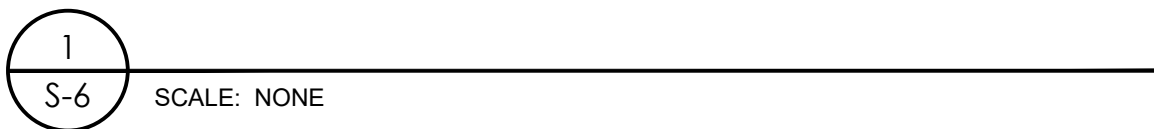
David R. Polston - Architect
3806 Park Ave. Suite 2-L, Wilmington, NC 28403
Architecture Planning Design

59 BED
ADDITION /
RENOVATION

ISSUE DATE: 06.18.2025

REV	DATE

S5



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PROJECT # 25-001-001

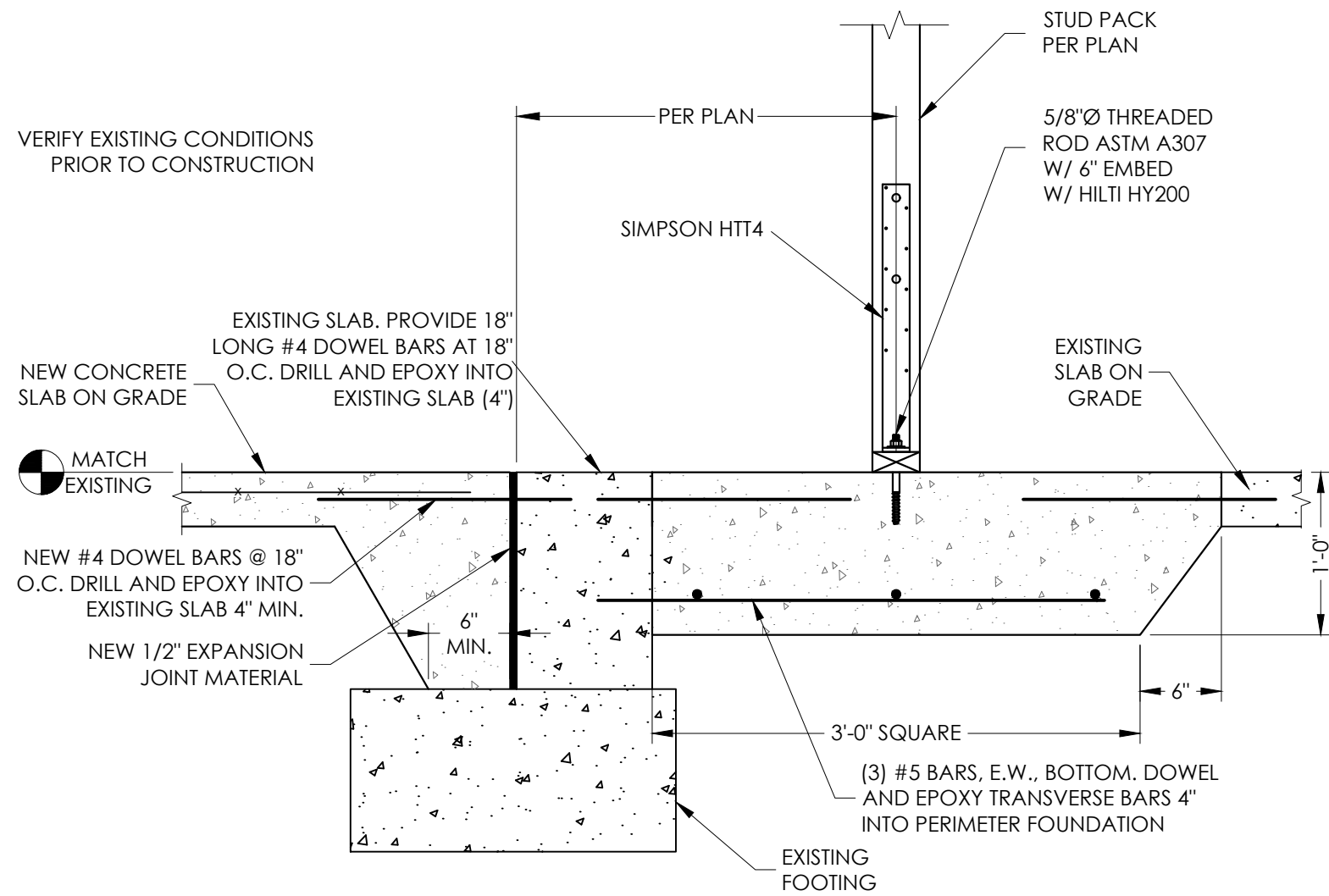
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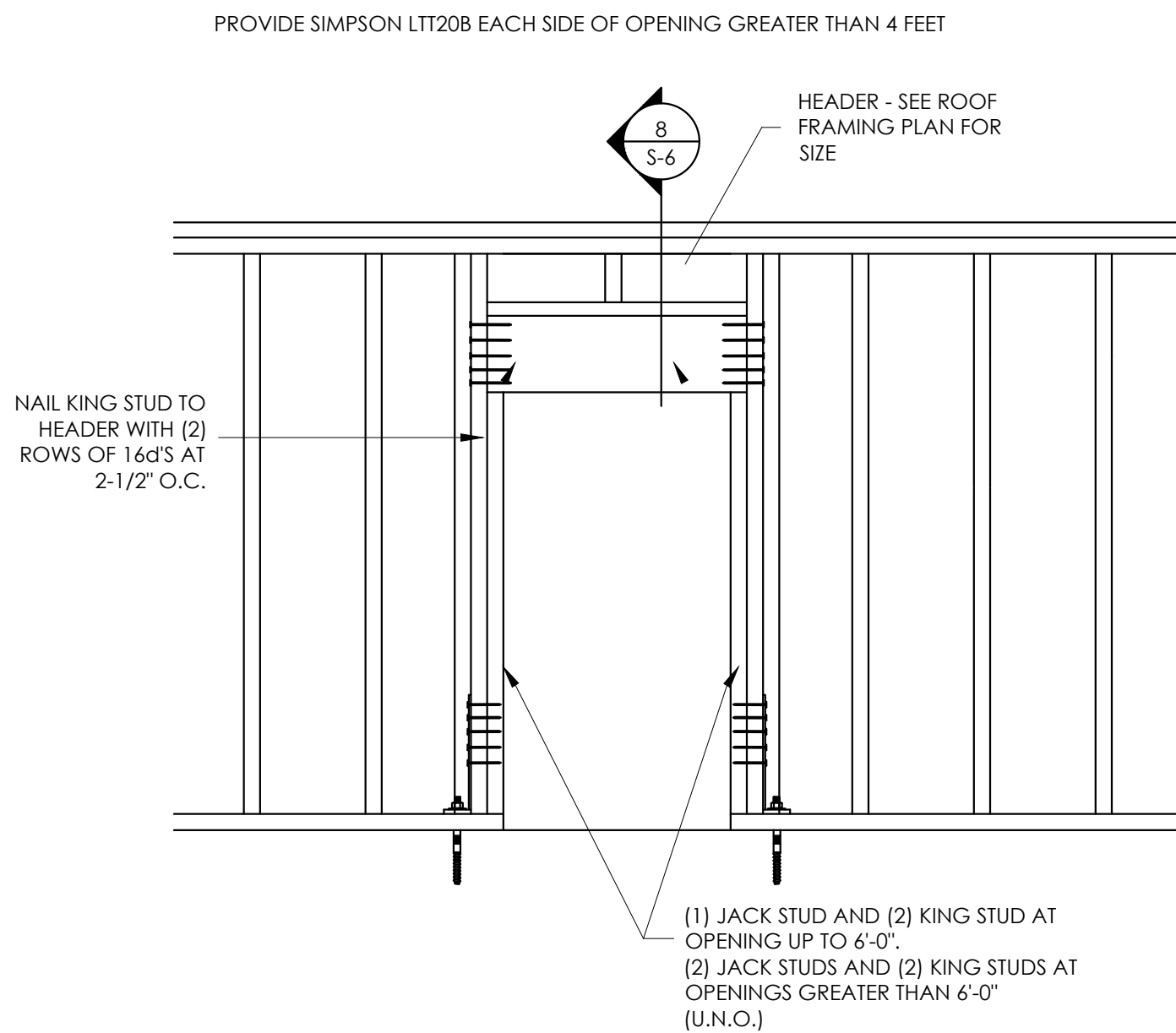
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59 BED ADDITION / RENOVATION	
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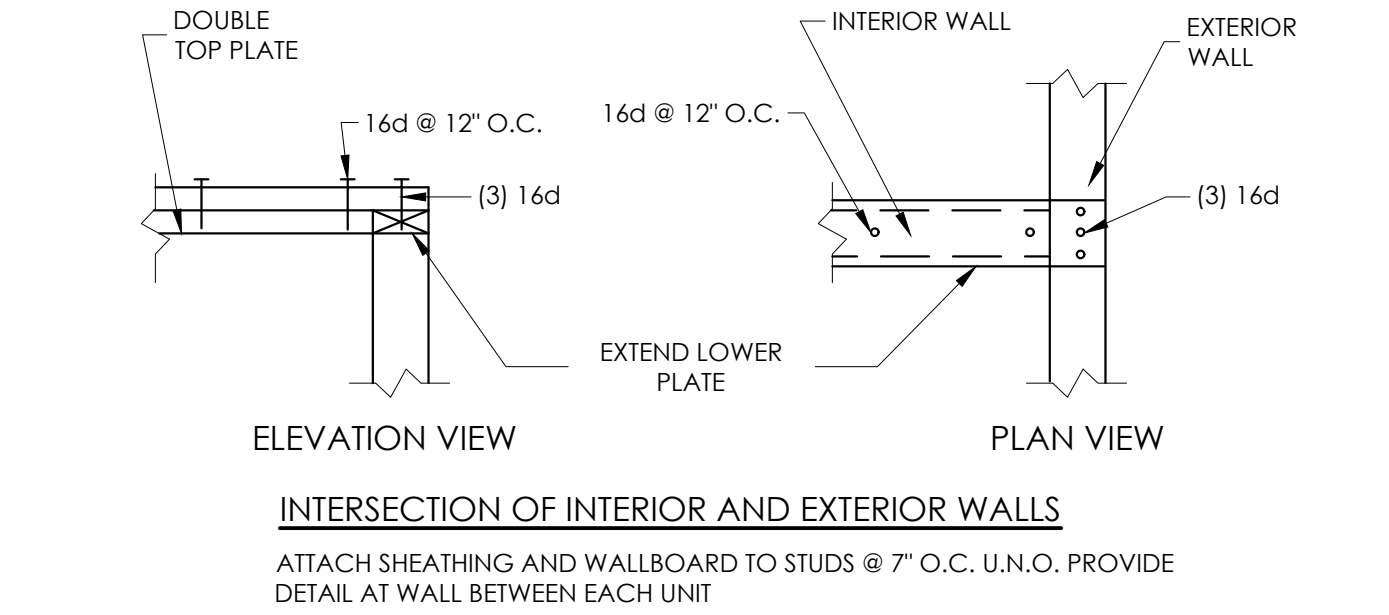
S6



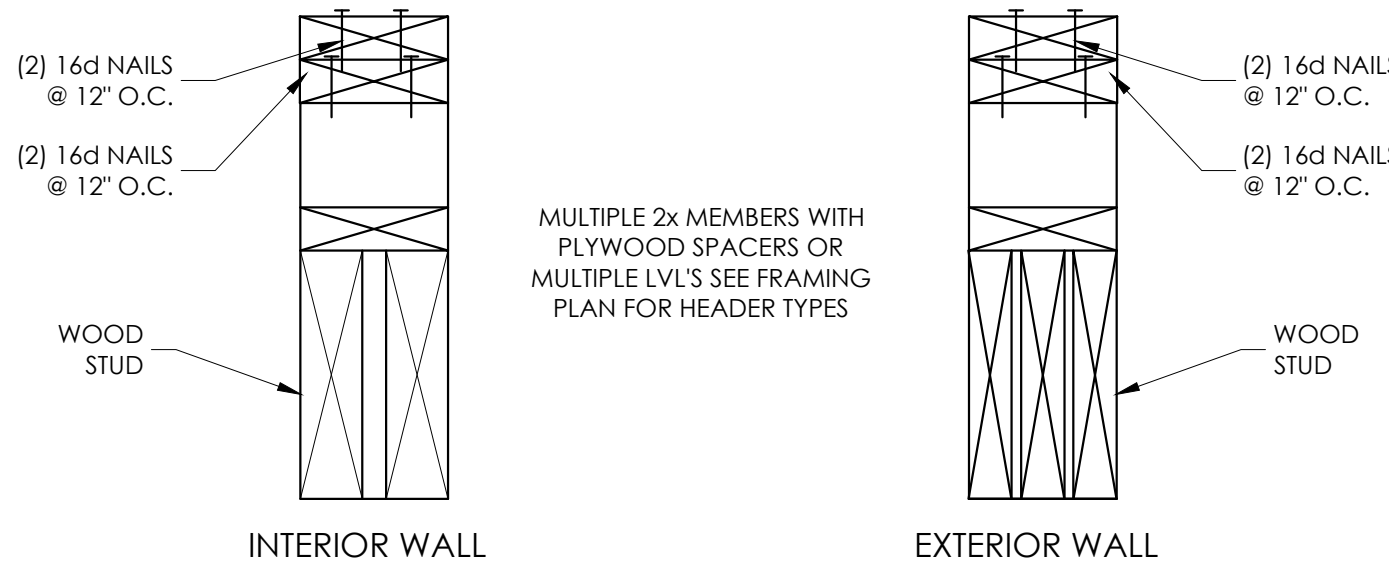
5 SECTION @ CUT-IN FOOTING
SCALE: NONE



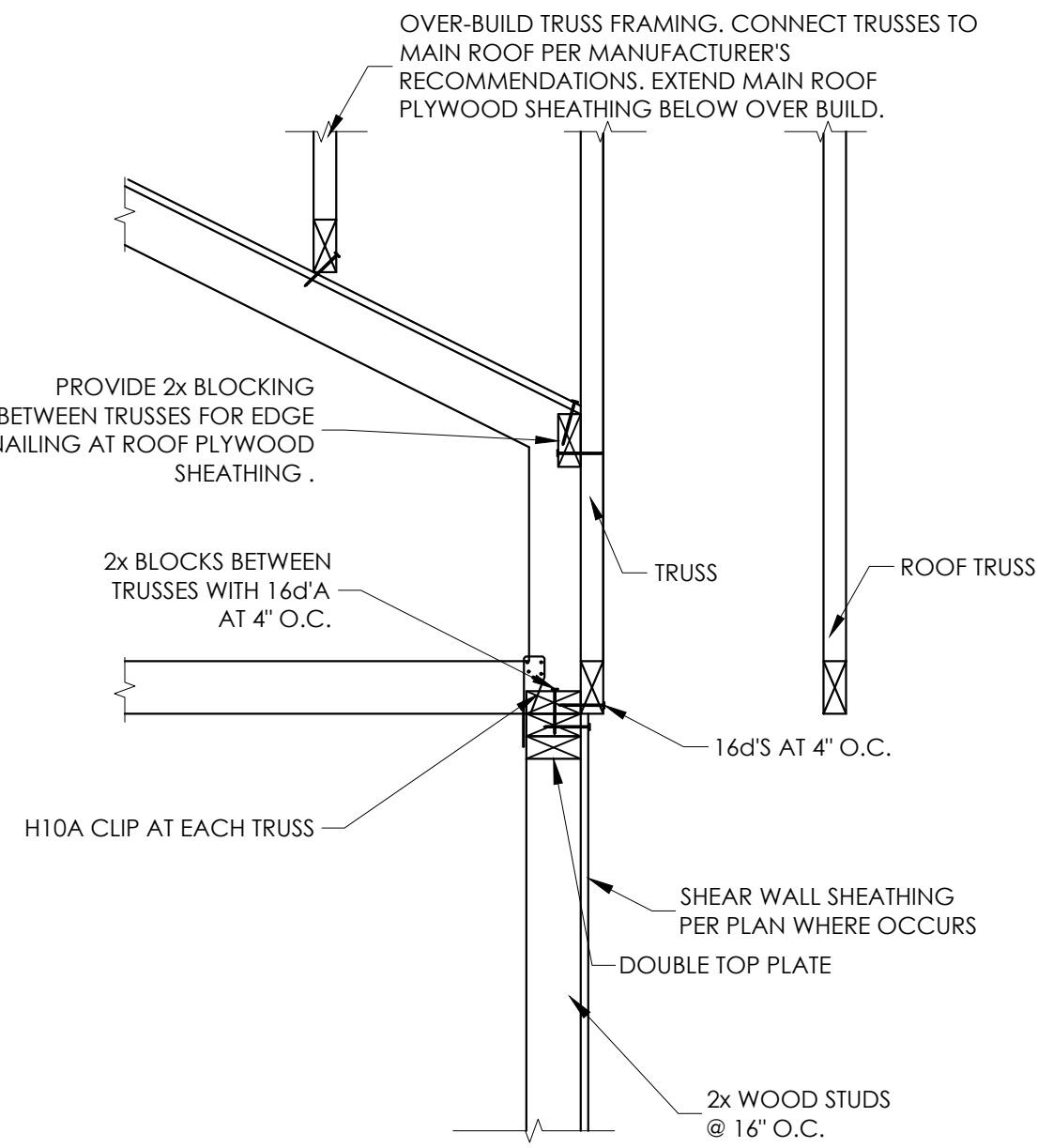
7 HEADER ELEVATION
SCALE: NONE



8 HEADER & TOP PLATE DETAIL
SCALE: NONE

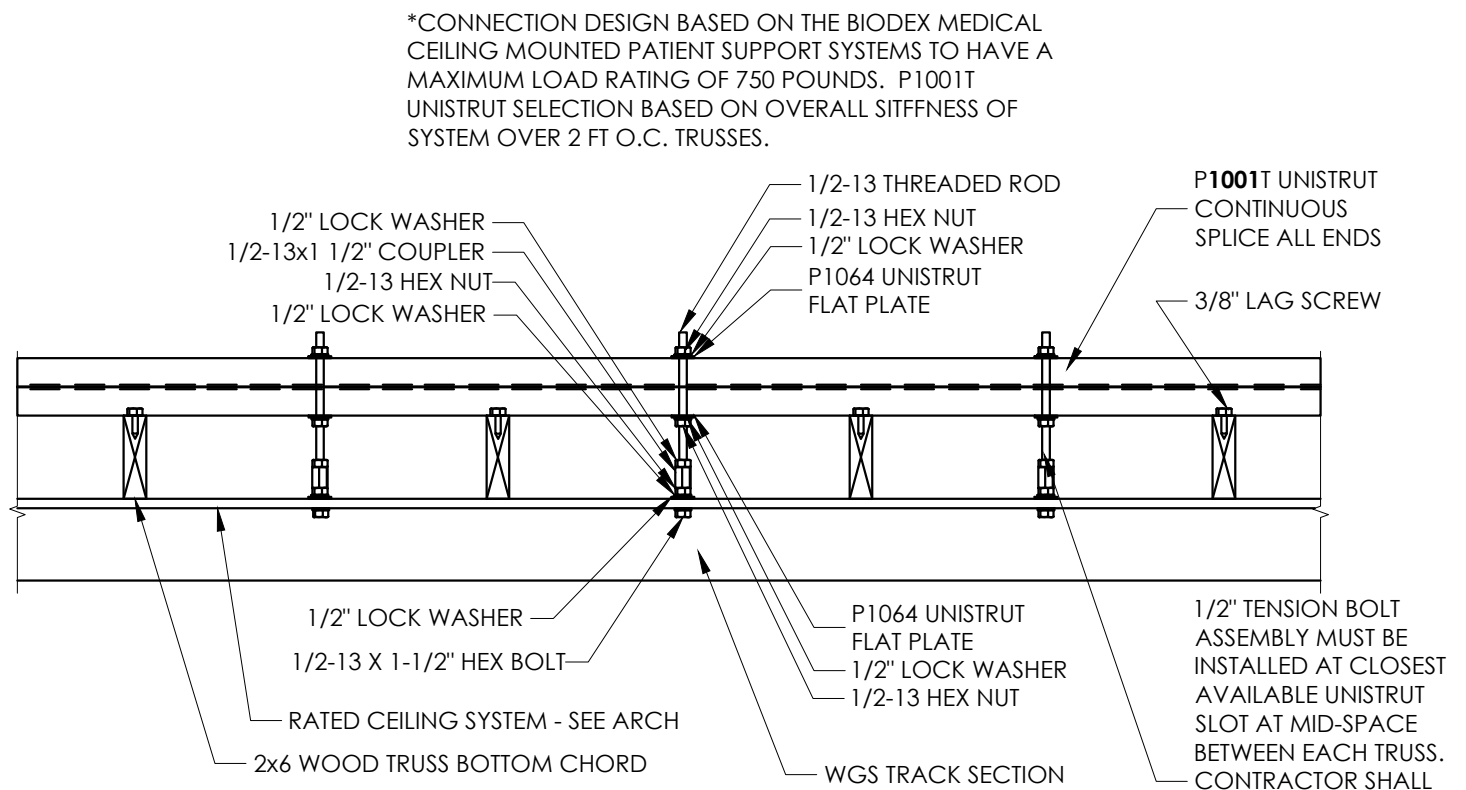


12 TOP PLATE SPLICE DETAIL
SCALE: NONE

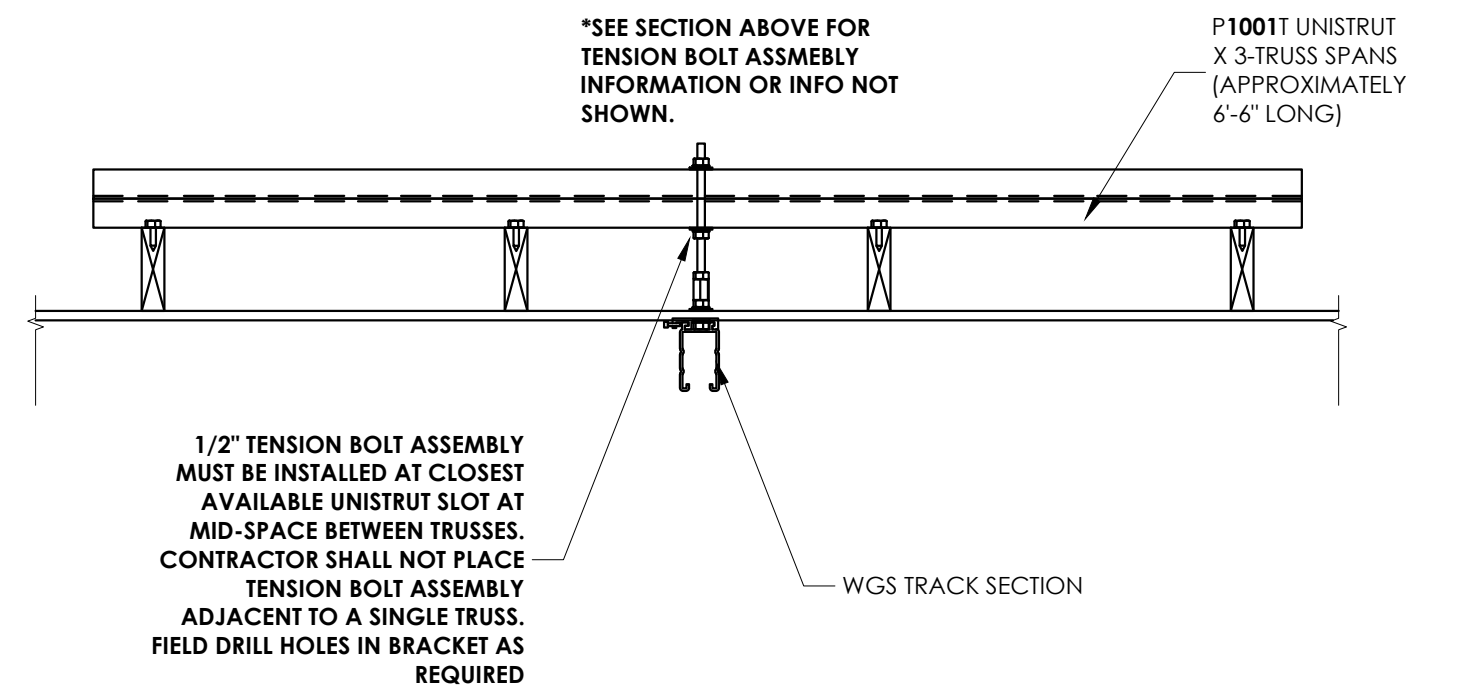


10 INTERIOR TRUSS BEARING
SCALE: NONE



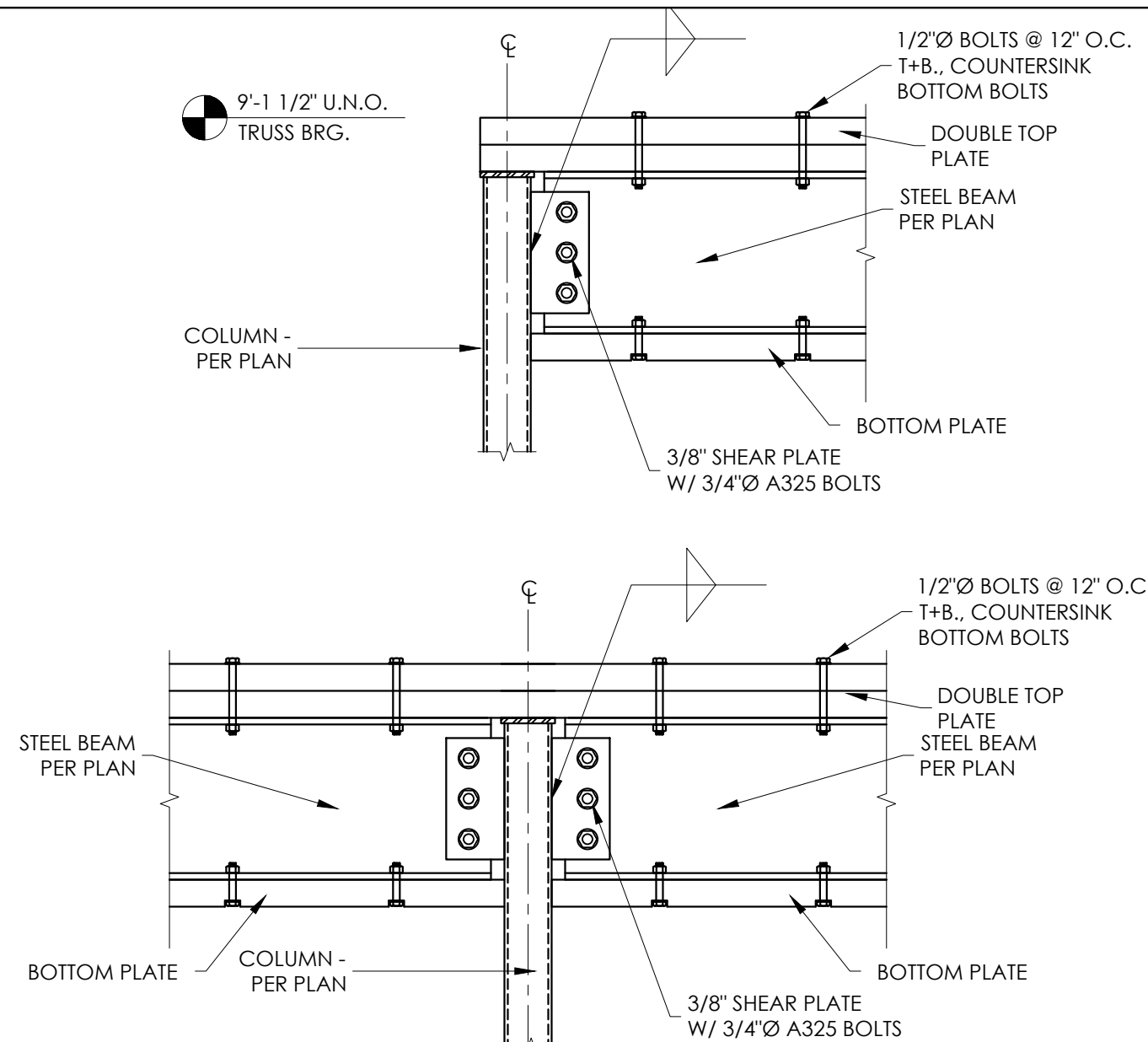


TRACK PERPENDICULAR TO TRUSS SECTION



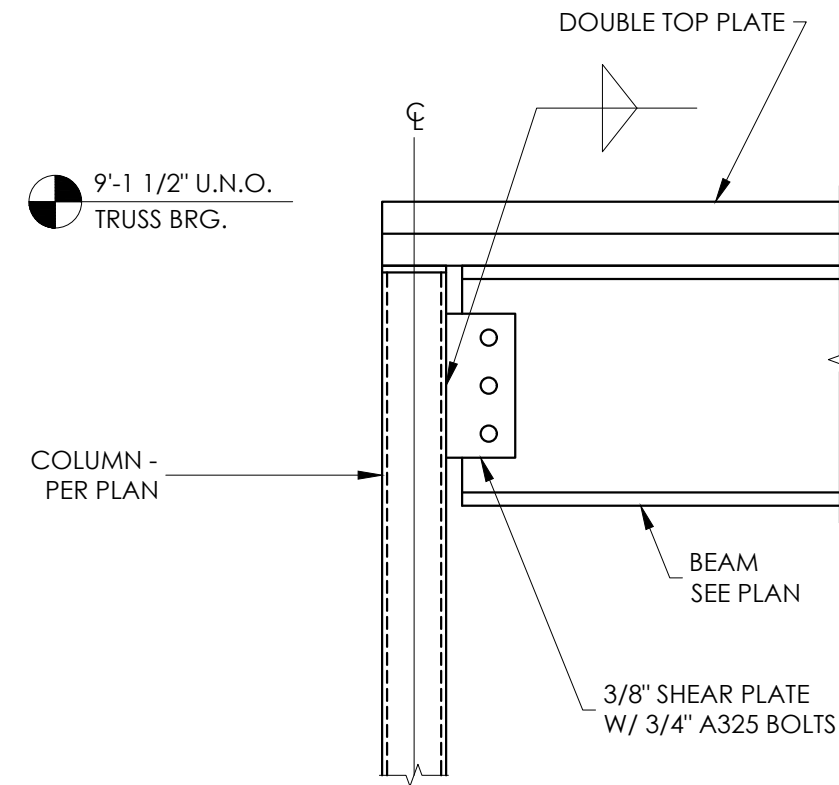
TRACK PARALLEL TO TRUSS SECTION

1 BIODEX TRUSS ATTACHMENT
S-8 SCALE: NONE



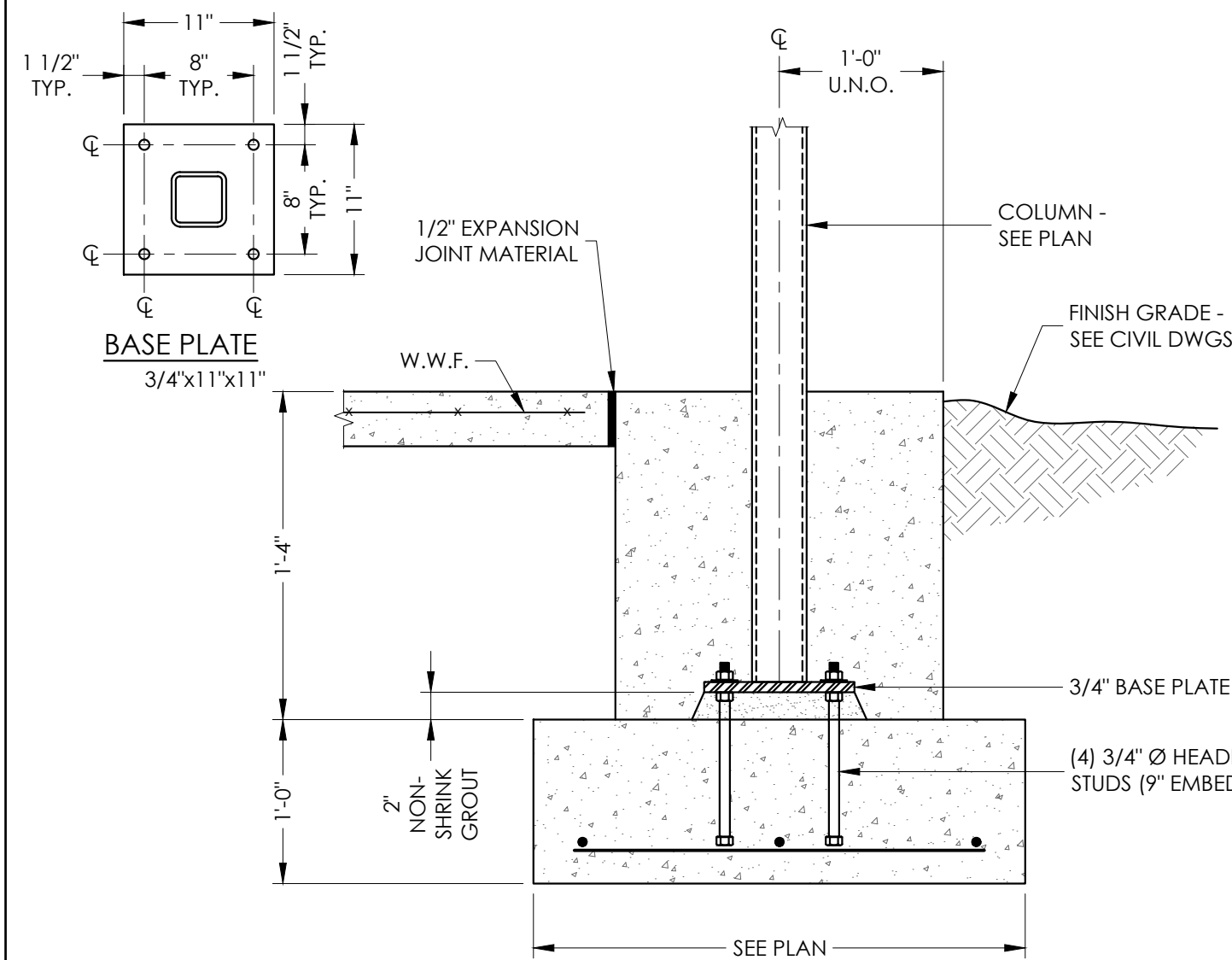
8 CHANNEL BEAM-TO-COLUMN
S-8 SCALE: NONE

2
S-8 SCALE: NONE

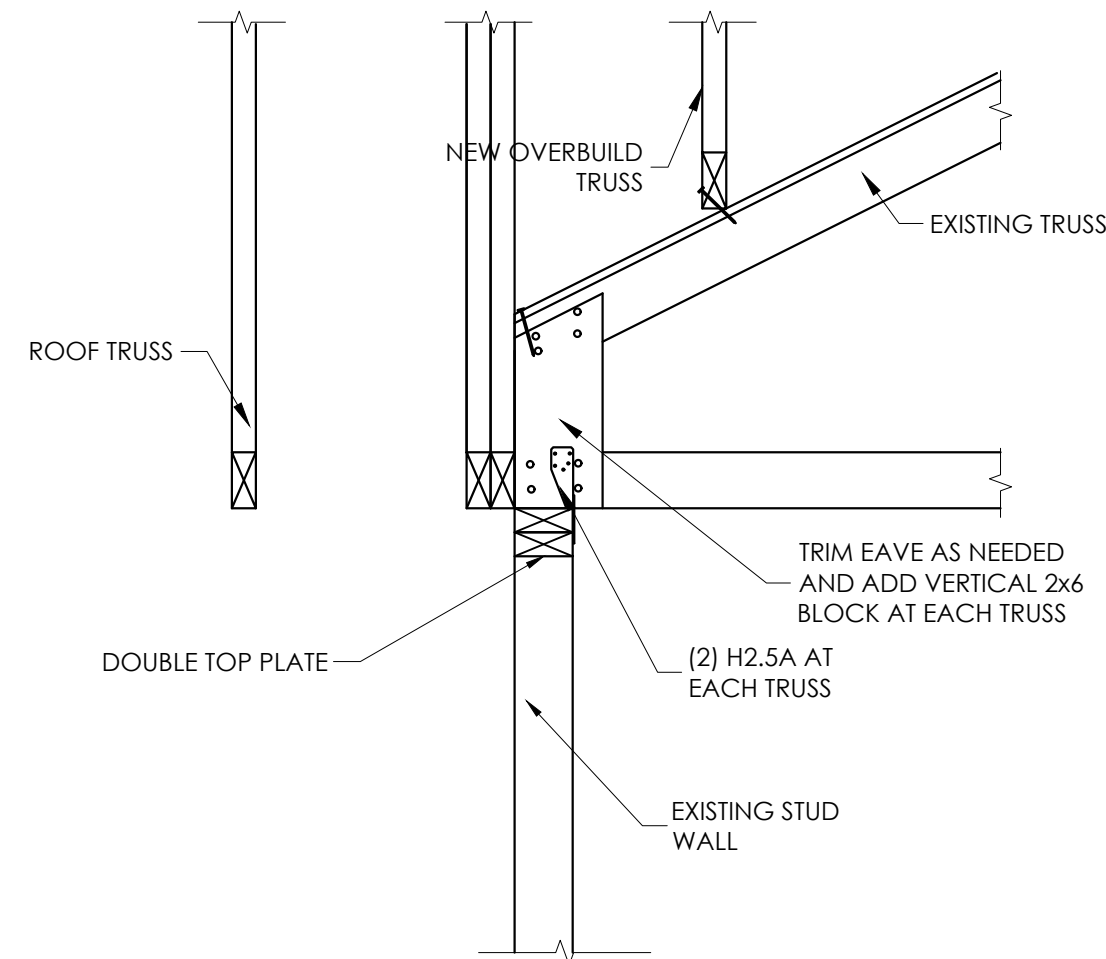


5 SHEAR CONNECTION
S-8 SCALE: NONE

9
S-8 SCALE: NONE

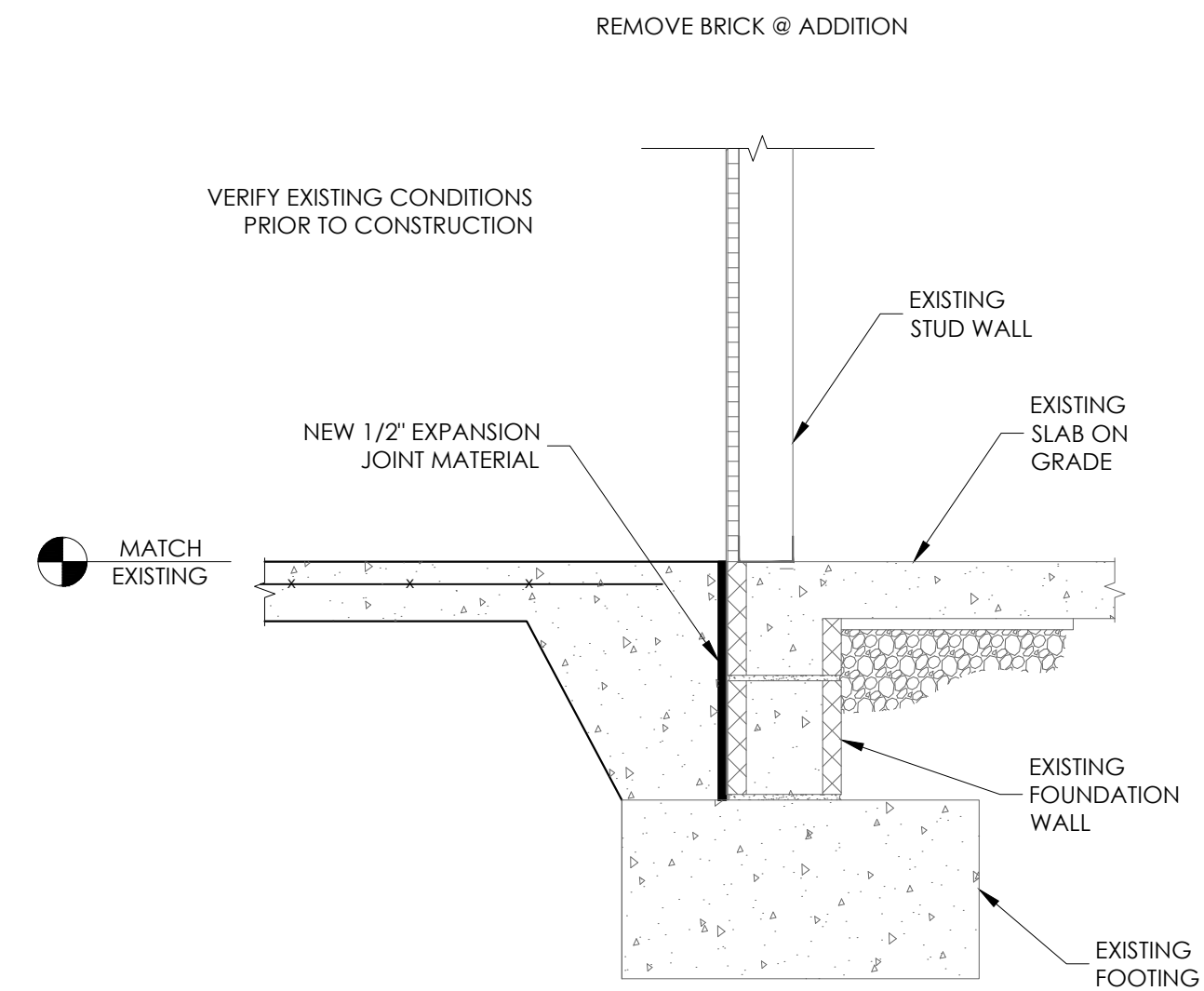


3 EXTERIOR COLUMN
S-8 SCALE: NONE

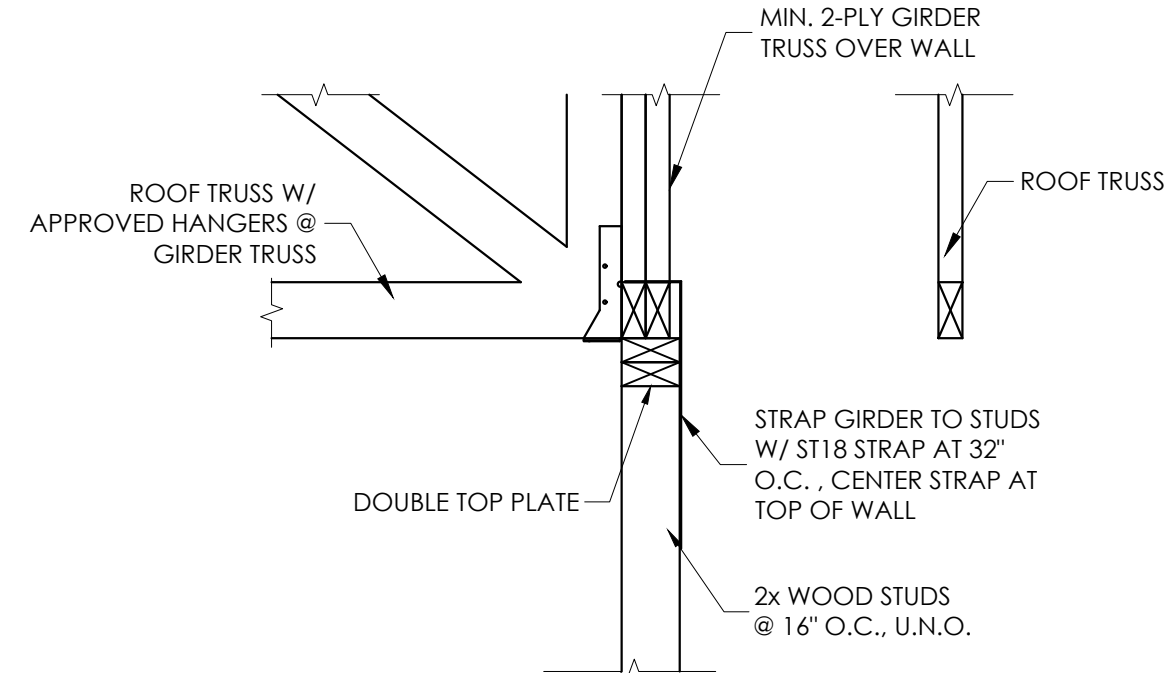


6 NEW TRUSS BEARING AT EXISTING WALL
S-8 SCALE: NONE

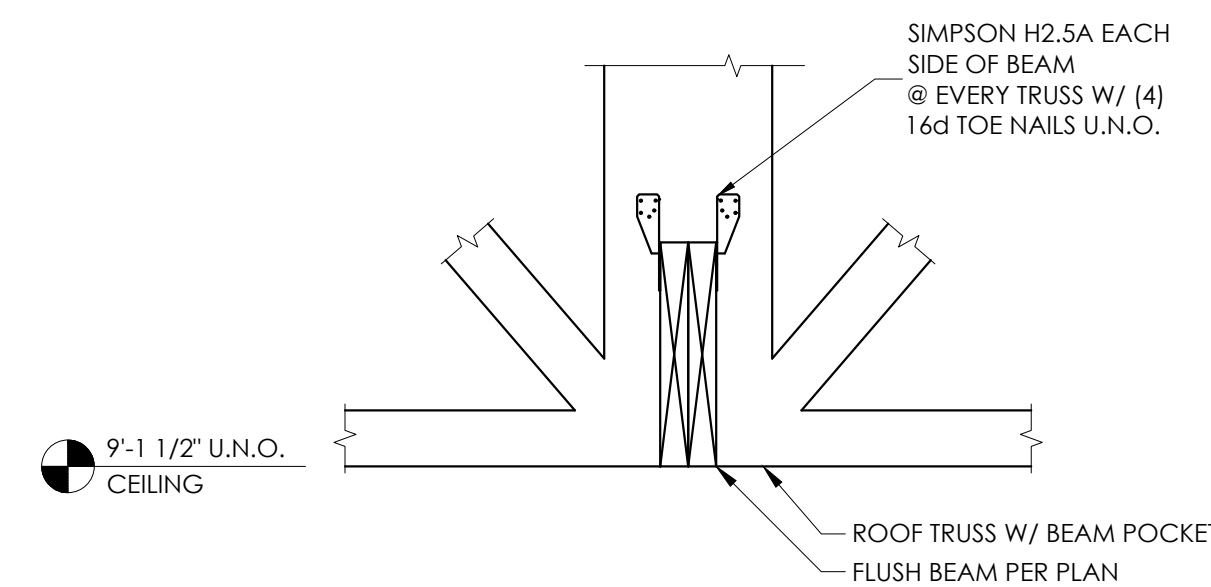
10
S-8



4 NEW TO EXISTING FOUNDATION
S-8 SCALE: NONE



7 GIRDER TRUSS CONNECTION TO WALL
S-8 SCALE: NONE



11 SECTION @ FLUSH BEAM POCKET
S-8

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HAUSER-CREECH, INC.

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PRUITT HEALTH
TOWN CENTER
HARRISBURG, NC

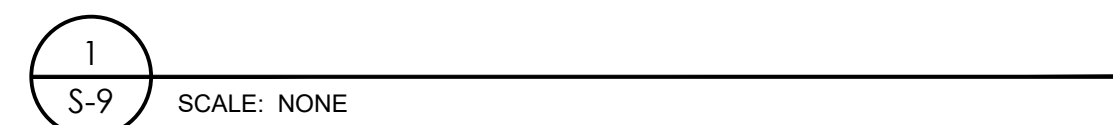
David R. Polston - Architect
3806 Park Ave. Suite 2-L, Wilmington, NC 28403
Architecture Planning Design

59 BED
ADDITION /
RENOVATION

ISSUE DATE: 06.18.2025

REV	DATE

S8



STRUCTURAL DESIGN DATA SHEET (ASCE 7-10):

RISK CATEGORY III (ASCE 7-10)
OCCUPANCY CLASSIFICATION INSTITUTIONAL GROUP I-2 (2015 IBC)

IMPORTANCE FACTORS:
I seismic 1.25
I snow 1.10

LIVE LOADS:
ROOF 20 psf
CATWALK 40 psf
FLOOR 100 psf

SNOW LOAD:
Pg 10 psf

WIND LOAD:
Basic Wind Speed 120 MPH
Exposure Category B

SEISMIC LOAD:
Spectral Response
Ss 0.229
S1 0.100
Sds 0.244
Sd1 0.160
Seismic Design Category C
Seismic Site Class D (Default)
Structural System Light framed walls sheathed w/ structural panels
R-Factor 6.5
Analysis Procedure Equivalent Lateral Force
Seismic Base Shear

SEISMIC ANCHORAGE OF NON-STRUCTURAL COMPONENTS:
Per ASCE 7 Chapter 13 all non-structural components are required to be braced against seismic sway.

LATERAL DESIGN CONTROL:
X-Direction Wind
Y-Direction Wind

SOIL BEARING PROPERTIES:
Allowable Bearing Capacity 2000 psf (ECS Project#08:16127, dated April 10, 2025)

BASE SHEAR SCHEDULE				
	WIND BASE SHEAR		SEISMIC BASE SHEAR*	
	Vx	Vy	Vx	Vy
"200" WING	7.6 K	4.0 K	0.4 K	0.4 K
"400" WING	21.0 K	9.2 K	2.4 K	2.4 K
"500" WING	28.8 K	9.2 K	3.3 K	3.3 K
"600" & "700" WINGS	63.7 K	55.4 K	11.4 K	11.4 K

*SEISMIC BASE SHEAR VALUES HAVE BEEN FACTORED BY 0.7.

STATEMENT OF SPECIAL INSPECTIONS:

Project Name: Pruitt Health - Town Center

Building Permit Number:

Project Address: Harrisburg, North Carolina

The following information is being submitted in accordance with the Special Inspection provisions of the International Building Code. Attached is the Schedule of Special Inspections (SSI) required for this project.

The Special Inspection program outlined herein does not relieve the Contractor or any other entity of contractual duties, including quality control, quality assurance or safety. The contractor is solely responsible for construction means, methods and job site safety.

Respectfully submitted,
The Structural Engineer of Record

Signature: Theodore A. Deters Date: 6/6/26

REINFORCED CONCRETE:

- ALL CONCRETE WORK SHALL CONFORM TO THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE," (ACI 318, 14)
- REINFORCING STEEL SHALL BE DEFORMED BARS ASTM A-615 (GRADE 60)
- THE COMPRESSIVE STRENGTH AT 28 DAYS OF ALL CAST IN PLACE CONCRETE SHALL BE 3000 P.S.I. (SEE CIVIL DRAWINGS FOR SITE CONCRETE) KEEP COPY OF CONC. TEST REPORTS ON SITE AT ALL TIMES.
- LAP SPLICES FOR #5 REINFORCING BARS SHALL BE 24" MIN., U.N.O.
- CLEAR CONCRETE COVER FOR REINFORCING STEEL:
MASONRY WALLS: LOCATE IN CENTER OF WALL (U.N.O.)
FOOTINGS: 2" FORMED EDGES
3" CAST AGAINST GROUND
SLAB ON GRADE: MID-HEIGHT OF SLAB
- THE LONGITUDINAL REINFORCING STEEL IN WALLS AND FOOTINGS SHALL BE CONTINUOUS AROUND CORNERS. SEE TYPICAL DETAILS.
- ALL CONCRETE SHALL BE VIBRATED BY MECHANICAL VIBRATORS.

STRUCTURAL STEEL:

- ALL STRUCTURAL STEEL WORK SHALL CONFORM TO THE A.I.S.C., "STEEL CONSTRUCTION MANUAL" 360-10.
- STRUCTURAL STEEL SHALL BE ASTM A-992.
- STRUCTURAL TUBES SHALL BE ASTM A500, GRADE B.
- STEEL FRAMING CONNECTIONS SHALL BE BOLTED OR WELDED. BOLTS SHALL BE 3/4" DIAMETER MINIMUM AND SHALL BE ASTM A-325-N U.N.O., SNUG TIGHT ALL CONNECTIONS.
- ANCHOR BOLTS SHALL BE ASTM F1554 HEADED BOLTS. MINIMUM ANCHOR BOLT EMBEDMENT LENGTH SHALL BE 12 BOLT DIAMETERS U.N.O. CLEAN ANCHOR BOLTS OF ALL GREASE, DIRT, ETC., BEFORE INSTALLATION.
- WELDS SHOWN ON THE STRUCTURAL DRAWINGS ARE THE MINIMUM REQ'D BY DESIGN. THE FABRICATOR'S DRAWINGS SHALL SHOW WELDS AND THEY SHALL CONFORM TO A.W.S. SPECIFICATIONS. ALL WELDING SHALL BE DONE WITH E-70 SERIES ELECTRODES.
- PAIN T ALL STRUCTURAL STEEL WITH ONE COAT OF RED OXIDE RUST-INHIBITIVE PRIMER 2.5 MILS IN THICKNESS. THE COMPATIBILITY OF PRIMER AND ANY TOP COAT SHALL BE VERIFIED BEFORE ANY PAINTING IS PERFORMED. TOUCH-UP ALL EXPOSED METAL AFTER FIELD INSTALLATION. ALL STRUCTURAL STEEL WHICH IS EXPOSED TO THE ELEMENTS SHALL RECEIVE TWO COATS OF EXTERIOR ENAMEL WHICH IS COMPATIBLE TO THE PRIMED SURFACE.
- THE SHOP DRAWINGS SHALL INCLUDE COMPLETE DETAILS AND SCHEDULES FOR FABRICATION AND ASSEMBLY OF STRUCTURAL STEEL MEMBERS. SUBMIT FOUR PRINTS OF EACH DRAWING. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED. CONTRACTOR TO REVIEW AND STAMP DRAWINGS PRIOR TO SUBMISSION TO THE EOR.

WOOD TRUSSES:

- ROOF TRUSSES SHALL BE DESIGNED TO SUPPORT THE DESIGN LOADS INDICATED IN THE DESIGN INFORMATION SECTION.
- IN ADDITION TO THE UNIFORM LOADING SPECIFIED FOR TRUSS DESIGN, THE TRUSS SUPPLIER SHALL INCLUDE ANY CONCENTRATED LOADS CAUSED BY ARCHITECTURAL FEATURES OR M. P&E EQUIPMENT OR MATERIALS AND BY SPRINKLER LOADS IN THE TRUSS DESIGN.
- TRUSSES SHALL BE DESIGNED BY A REGISTERED ENGINEER IN THE STATE OF NORTH CAROLINA AND SHOP DRAWINGS BEARING THE ENGINEER'S SEAL SHALL BE SUBMITTED FOR APPROVAL.
- TRUSSES SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH APPLICABLE STANDARDS OF THE TRUSS PLATE INSTITUTE TPI I-2002.
- LIMIT LL DEFLECTION TO L/360. LIMIT TL DEFLECTION TO L/240 OR 1.25" MAX.

DESIGN INFORMATION:

- ALL CONSTRUCTION SHALL CONFORM TO THE 2018 NORTH CAROLINA BUILDING CODE, 2015 INTERNATIONAL BUILDING CODE AND ASCE 7-10.
- DESIGN LOADS:
DEAD AND LIVE LOADS
ROOF LOADS
TOP CHORD DEAD 15 psf
BOTTOM CHORD DEAD 5 psf
TOP CHORD LIVE 20 psf
BOTTOM CHORD LIVE 10 psf (WITHOUT ATTIC STORAGE)
CATWALK 40 psf
FLOOR LOADS
TOP CHORD DEAD N/A
BOTTOM CHORD DEAD N/A
TOP CHORD LIVE N/A
BOTTOM CHORD LIVE N/A
RISK CATEGORY II
IMPORTANCE FACTORS
I seismic 1.25
I snow 1.10
GROUND SNOW LOAD (pg) 0 psf
DESIGN WIND SPEED 151 mph
SEISMIC DESIGN PARAMETERS
S1 5.9 %g
Ss 11.4 %g
SITE CLASS D (DEFAULT)
Sds 0.122
Sd1 0.094
SEISMIC DESIGN CATEGORY B
R 6.5
Cv 0.0235
- ADDITIONAL LIVE LOADS PRESCRIBED IN ASCE7-10 RELATED TO ROOF ATTICS AND ROOF TRUSSES, INCLUDING LIMITED ACCESS STORAGE IN ATTICS SHALL APPLY TO PRE-FABRICATED TRUSSES, AND SHALL BE CLEARLY IDENTIFIED ON THE TRUSS SHOP DRAWINGS.
- THE DESIGN ADEQUACY AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC. IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- FOR LOCATION OF MISCELLANEOUS ITEMS (SUCH AS INSERTS, ETC.) AFFECTING STRUCTURAL WORK, SEE ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS.
- THIS PROJECT CONTAINS A SERIES OF DETAILS CONSIDERED "TYPICAL DETAILS". THESE SHALL APPLY AT ALL SITUATIONS THAT ARE THE SAME OR SIMILAR AS THESE DETAILS. THESE "TYPICAL DETAILS" SHALL APPLY WHETHER OR NOT THEY ARE INDICATED OR CUT AT EACH LOCATION.
- VERIFY EXISTING CONDITIONS AND NOTIFY ARCHITECT AND ENGINEER OF ANY CONDITIONS WHICH DO NOT COMPLY WITH PLANS AND SPECIFICATIONS. STRUCTURAL DRAWINGS MUST BE WORKED WITH ARCHITECTURAL DRAWINGS.
- USE OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED. THE CONTRACTOR SHALL REVIEW AND STAMP DRAWINGS ACCORDINGLY PRIOR TO SUBMITTING TO THE ENGINEER. THE OMISSION OF ITEMS FROM SHOP DRAWINGS SHALL NOT RELIEVE CONTRACTOR OF RESPONSIBILITY OF FURNISHING AND INSTALLING ITEMS REGARDLESS OF WHETHER SHOP DRAWINGS HAVE BEEN REVIEWED AND APPROVED.

WOOD FRAMING (NOT INCLUDING PRE-FABRICATED TRUSSES):

- ALL WOOD CONSTRUCTION SHALL CONFORM TO THE FLORIDA BUILDING CODE AND TO THE NDS.
- ALL NAILING (UNLESS NOTED OTHERWISE) SHALL CONFORM TO THE NORTH CAROLINA BUILDING CODE.
- ALL STUDS, TOP PLATES AND SILL PLATES IN BEARING WALLS AND SHEARWALLS SHALL BE SPF NO. 2 OR BETTER.
- ALL STUDS, TOP PLATES AND SILL PLATES IN NON-BEARING WALLS SHALL BE SPF NO. 3 OR BETTER.
- ALL 2x NOMINAL HEADERS SHALL BE SPF NO. 2 OR BETTER OR SYP NO. 2 OR BETTER.
- ALL EXPOSED LUMBER SHALL BE PRESERVATIVE TREATED.
- FINGER JOINTED STUDS MAY BE USED IN INTERIOR APPLICATIONS PROVIDED THE STRUCTURAL PROPERTIES EQUAL OR EXCEED THAT OF THE SOLID SAWN LUMBER. FINGER JOINTED LUMBER SHALL NOT BE USED IN EXPOSED CONDITIONS.
- ALL CONNECTIONS IN EXPOSED LUMBER SHALL BE HOT DIPPED GALVANIZED OR STAINLESS STEEL.
- ALL LUMBER IN CONTACT WITH CONCRETE SHALL BE PRESERVATIVE TREATED.
- ALL MANUFACTURED LAMINATED VENEER LUMBER (LVL) SHALL HAVE A MODULUS OF ELASTICITY OF 256 psf AND A MINIMUM BENDING STRENGTH OF 2800 psf.
- UNDER NO CIRCUMSTANCE SHALL LAMINATED VENEER LUMBER BE USED IN AN EXPOSED CONDITION. WHERE MANUFACTURER LUMBER IS REQUIRED IN AN EXPOSED CONDITION THE CONTRACTOR MUST USED PRESERVATIVE TREATED GLU-LAMINATED LUMBER (GLB).
- ALL GLU-LAMINATED LUMBER SHALL BE GRADED ACCORDING TO THE PLANS. IF NO GRADE IS SPECIFIED A MINIMUM GADE OF 4VF2400 SHALL BE USED.

FOUNDATION NOTES:

- FOUNDATION DESIGN IS BASED UPON SOIL VALUES PROVIDED BY ECS SOUTHEAST, LLC (PROJECT #08:16127, DATED APRIL 10, 2025). CONTRACTOR/OWNER SHALL VERIFY PRIOR TO CONSTRUCTION. FOOTINGS ARE DESIGNED TO BEAR ON UNIFORM SUITABLE SOIL CAPABLE OF SUPPORTING 2000 PSF.
- IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO FOLLOW RECOMMENDATIONS BY A LICENSED GEOTECHNICAL ENGINEER TO ACHIEVE 2000 PSF AND LESS THAN 1" ANTICIPATED SETTLEMENT.
- THE SOIL BEARING CAPACITY AND CONSISTENCY SHALL BE VERIFIED FOR THE BUILDING LIMITS BY A REGISTERED GEO-TECHNICAL ENGINEER WHEN FOUNDATION EXCAVATIONS HAVE BEEN CARRIED DOWN TO THE PROPOSED ELEVATIONS. THE BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A 1'-6" MINIMUM BELOW FINISHED SLAB (U.N.O.)
- WHERE FOOTING EXCAVATIONS ARE TO REMAIN OPEN AND MAY BE EXPOSED TO RAINFALL, THE EXCAVATIONS SHALL BE UNDERCUT AND A 3" THICK MUD MAT OF 2000 PSI CONCRETE SHALL BE PLACED OR CLEAN GRAVEL SHALL BE PLACED IN THE BOTTOM TO PROTECT THE BEARING SOILS.
- WHERE FOOTING STEPS ARE NECESSARY, THEY SHALL BE NO STEEPER THAN 1 VERTICAL TO 2 HORIZONTAL, UNLESS SHOWN OTHERWISE ON PLANS.
- IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY FOR PREPARING THE BUILDING PAD PER THE GEOTECHNICAL ENGINEER OF RECORD'S RECOMMENDATIONS.

CONCRETE MASONRY:

- CONCRETE MASONRY SHALL CONFORM TO THE NATIONAL CONCRETE MASONRY ASSOCIATION SPECIFICATIONS, AND HAVE A DENSITY OF 125 P.C.F. AND SHALL HAVE A MINIMUM PRISM STRENGTH (Fm) OF 1500 P.S.I.
- GROUT FOR FILLING CONCRETE MASONRY CELLS SHALL CONFORM TO STANDARD SPECIFICATIONS FOR "GROUT FOR MASONRY", ASTM C-476-02, AND SHALL HAVE A COMPRESSIVE PRISM STRENGTH (Fm) OF 3000 P.S.I. AT 28 DAYS. THE SLUMP SHALL BE BETWEEN 9" AND 11". WHERE THE MINIMUM DIMENSION OF ANY CONTINUOUS VERTICAL CELL IS 3" OR LESS, USE FINE GROUT, OTHERWISE USE COARSE (PEA GRAVEL) GROUT.
- MORTAR FOR CONCRETE MASONRY SHALL BE TYPE "S" AND SHALL CONFORM TO ASTM C-270-04. 4. GROUT PROCEDURES AND REBAR INSTALLATION SHALL PER ASTM ACI 530 1-99. PROVIDE 36" LAP SPLICES IN REBAR IN 12" CMU FIRE WALL.

SCHEDULE OF SPECIAL INSPECTIONS:

Project Name: Pruitt Health - Town Center

Construction divisions which require inspections for this project are as follows:

INSPECTION TASK	CONTINUOUS (C) OR PERIODIC (P) INSPECTIONS		SPECIAL INSPECTION FIRM	NOTES & SCOPE
	C	P		
1. VERIFICATION OF SOILS (Table 1704.7)				
Verify materials below shallow Foundations are adequate to achieve the design bearing capacity.		P	Testing Agency (TA)	Testing Agency shall provide soils report
Verify excavations are extended to proper depth.		P	Testing Agency (TA)	
Perform Classification and testing of compacted fill materials.		P	Testing Agency (TA)	
Verify use of proper materials, densities and lift thickness during placement and compaction of compacted fill.		C	Testing Agency (TA)	
Prior to placement of compacted fill, observe sub-grade and verify that site has been prepared properly.		P	Testing Agency (TA)	
2. REINFORCED CONCRETE (Table 1704.4)				
Inspection of reinforcing steel, including prestressing tendons, and placement. ACI 318:3.5, 7.1-7.7		P	Testing Agency (TA)	ACI 318: 3.5,7.1-7.7 IBC: 1913.4
Verifying use of required design mix: ACI 318: Ch. 4, 5.2-5.4		P	Testing Agency (TA)	ACI 318: Ch. 4, 5.2-5.4 IBC: 1904.2.2, 1913.2, 1913.3
At the time fresh concrete is sampled to fabricate specimens for strength tests, slump, air content, and temperature of concrete.		C	Testing Agency (TA)	ASTM C 172, C 31 ACI: 318: 5.6, 5.8 IBC: 1913.10
2. REINFORCED CONCRETE (Table 1704.4)				
Inspect OSB nailing patterns per structural plans. Inspect roof truss and top plate ties, hold downs, and anchorage per structural plans		P	Special Inspector (SI)	

SCHEDULE OF SPECIAL INSPECTIONS (Continued):

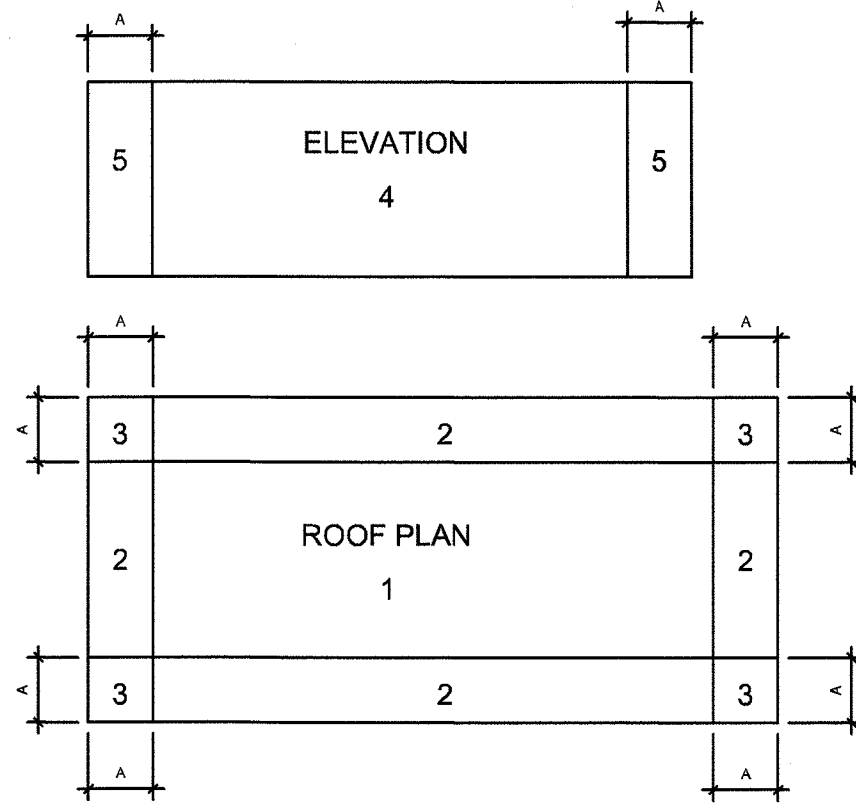
Project Name: Pruitt Health - Town Center

Construction divisions which require inspections for this project are as follows:

INSPECTION TASK	CONTINUOUS (C) OR PERIODIC (P) INSPECTIONS		SPECIAL INSPECTIONS FIRM	NOTES & SCOPE
	C	P		
3. STRUCTURAL STEEL (Table 1704.3)				
Material verification of high strength bolts, nuts and washers.		P	Special Inspector (SI)	AISC 360, A3.3
Inspection of high strength bolting, snug tight joints		P	Special Inspector (SI)	AISC 360, M2.5 IBC 1704.3.3
Material verification of structural steel.		P	Special Inspector (SI)	Fabricator's bill of materials verification is acceptable.
All field welding.		P	Special Inspector (SI)	AWS D1.1 IBC 1704.3.1
4. RETAINING WALLS (Table 1704.12)				
Inspect all retaining walls over 5 feet in height.		P	Testing Agency (TA)	
5. MASONRY				
As masonry construction begins, the following shall be verified to ensure compliance: (A) Proportions of site mixed mortar, (B) Construction of mortar joints, (C) Location of reinforcement and connectors.		P	Testing Agency (TA)	ACI 318: 3.5.7.1-7.7 IBC: 1913.4
The inspection program shall verify: (A) Size and location of structural elements, (B) Size, grade, type of reinforcement, (C) Protection of masonry during cold weather (temperature below 40 degrees F) or hot weather (temperature above 90 degrees F)		P	Testing Agency (TA)	Sec. 2108.9.2.11, Item 2, Sec. 2104.3, 2104.4, ACI Sec. 1.15.4, 2.1.2, Sec. 1.12, Sec 2.1.8.6, 2.1.8.6.2, ACI 3.3.G, Art 2.4.3.4, Art 1.8
Prior to grouting, the following shall be verified to ensure compliance: (A) Grout space is clean, (B) Placement of reinforcement and connectors, (C) Proportions of site-prepared grout, (D) Construction of mortar joints		P	Testing Agency (TA)	Sec. 1.12, Art. 3.2D, Art 3.4, Art. 2.6B, Art. 3.3B
Grout Placement shall be verified to ensure compliance with code and construction provisions.		P	Testing Agency (TA)	Art. 3.5

WIND LOAD SCHEDULE					
COMPONENTS & CLADDING	ROOF WIND LOAD			WALL WIND LOADS	
	ROOF AREA			WALL AREA	
	1	2	3	4	5
PRESSURE (PSF)	+10.5	+10.5	+10.5	+25.5	+25.5
SUCTION (PSF)	-21.5	-52.3	-58.8	-27.7	-33.8

- CORNER DISTANCE, A=5 FEET, ROOF = 100 SF, WALL = 13 S.F. C&C
- VALUES ARE NOT FACTORED. ASD LOAD FACTOR IS 0.6 FOR WIND.
- DP FOR WINDOW AND DOOR CAN CONSERVATIVELY USE NEGATIVE PRESSURES AT WALL AREA 5.



HAUSER-CREECH, INC.
PROJECT #: 25-001-001



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