

**NOTES:**

1. PROVIDE 4" CONCRETE SLAB ON GRADE REINFORCED W/ WWF 6x6-W1.4xW1.4 OVER 10 MIL POLY VAPOR BARRIER (LAP EDGES 6" MIN.) OVER 4" POROUS BASE ALL DIMENSIONS REFERENCED TO CENTERLINE OF COLUMNS, FACE OF EXTERIOR VENEER, AND CENTERLINE OF INTERIOR BEARING WALLS. SEE ARCHITECTURAL AND STRUCTURAL SECTIONS TO DETERMINE EDGE OF SLAB. VERIFY DIMENSIONS PRIOR TO CONSTRUCTION.
2. TOP OF EXTERIOR FTG. = F.F.E. -1'-4" AND FIN. GRADE -1'-0" (MIN.)
3. SEE ARCH. DWGS. FOR DIMENSIONS NOT SHOWN.
4. SEE DETAIL 5/S-3 FOR RECESSED SLAB DETAILS.
5. SEE DETAIL 1/S-3 FOR SLAB CONTROL JOINTS (CJ), ALTERNATE LAYOUT PLANS MAY BE SUBMITTED FOR APPROVAL.
6. SEE ARCHITECTURAL DRAWINGS. FOR LOCATIONS OF RECESSED AND/OR SLOPED SLAB AREAS. PROVIDE POSITIVE DRAINAGE FROM ALL PERIMETER WALLS TO FLOOR DRAIN. COORDINATE W/ PLUMBING DWGS. SEE DETAIL 5/S-3.
7. LOCATE CONTROL JOINTS UNDERNEATH NON-BEARING WALLS WHERE POSSIBLE.
8. PROVIDE (4) 2x6 @ EXT. WALLS, (5) 2x4 @ INT. WALLS BEARING (MIN.) AT ALL GIRDER TRUSSES BEARING POINTS AND SHEARWALL END POSTS W/ SIMPSON HTT4 AT STUD BASE.
9. REFER TO ARCHITECTURAL DRAWINGS FOR RATED WALL LOCATIONS.
10. SEE FOOTING SCHEDULE FOR SIZES AND REINFORCING.
11. OMITTED
12. ALL EXTERIOR STUDS SHALL BE 2x6 SPF NO. 2 STUDS AT 16" O.C. ALL INTERIOR STUDS AT BEARING WALLS AND SHEAR WALLS SHALL BE (2) 2x4 SPF NO. 2 STUDS AT 16" O.C.
13. PROVIDE (2) 6'-0" LONG #5 BARS AT RE-ENTRANT CORNERS, PLACE AT MID-DEPTH OF SLAB.
14. INTERIOR FOOTING DIMENSIONS SHOULD NOT BE USED TO LOCATE INTERIOR WALLS. REFER TO ARCHITECTURAL DRAWINGS FOR ALL INTERIOR WALL DIMENSIONS.

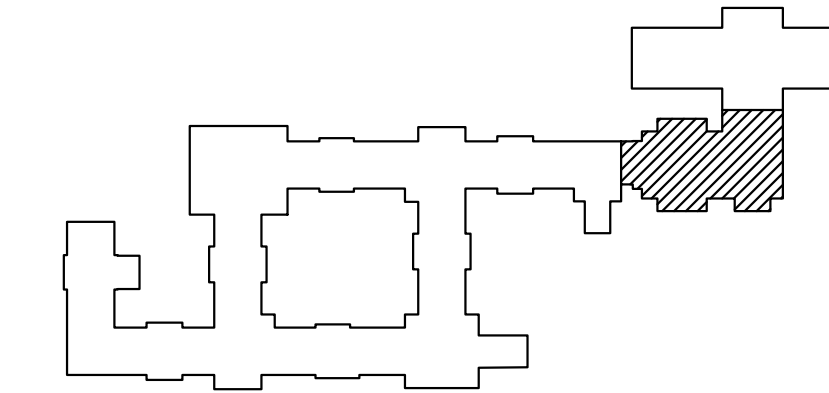
"SHEARWALL" DESIGNATES INTERIOR 2X4 STUDS SHEATHED W/ MINIMUM 7/16" OSB. PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED JOINTS. EDGE NAIL WITH 8d COMMONS AT 4" O.C. AND FIELD NAIL WITH 8d COMMONS AT 12" O.C.

WRAP ALL EXTERIOR WALLS WITH MINIMUM 7/16" OSB. PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED JOINTS. EDGE NAIL WITH 8d COMMONS AT 4" O.C. AND FIELD NAIL WITH 8d COMMONS AT 12" O.C.

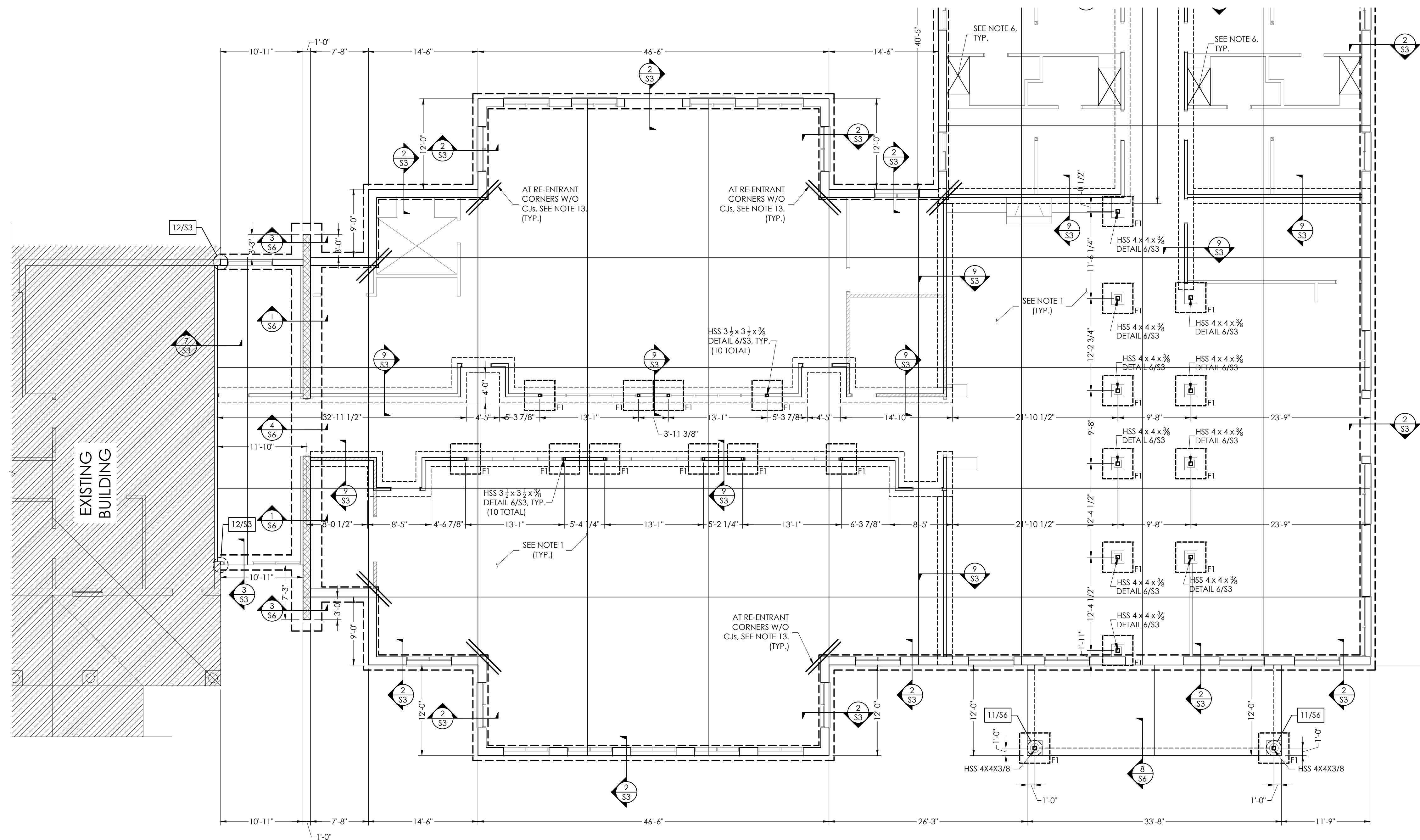
\*\*\*EXISTING CONFIGURATIONS MUST BE VERIFIED BY THE CONTRACTOR PRIOR TO DEMOLITION AND CONSTRUCTION

\*\*\*CONTRACTOR IS RESPONSIBLE FOR TEMPORARY SHORING OF ALL SUPPORTS DURING CONSTRUCTION. TEMPORARY WALLS OR BEAMS WITH SCREW JACKS ARE ACCEPTABLE METHODS OF SHORING

FOOTING SCHEDULE		
TYPE	SIZE	REBAR
F1	4'-0" X 4'-0" X 1'-0"	(4) #5s (3'-6" LONG) E.W.

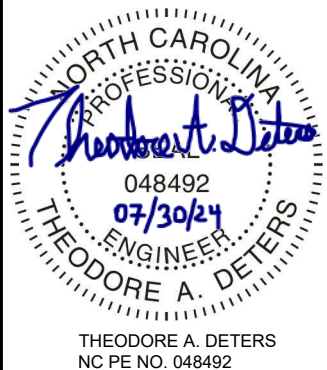


**KEY PLAN**



**FOUNDATION PLAN**

SCALE: 1/8"=1'-0"



HAUSER-CREECH, INC.  
 P. 919.817.7579  
 P. 919.817.7676  
 F. 919.404.2427  
 4506 PEARCES RD.  
 ZEBULON, NC  
 27597

UNI HEALTH  
 POST-ACUTE CARE  
 RALEIGH, NC

David R. Polston - Architect  
 3806 Park Ave. Suite 2-L, Wilmington, NC 28403  
 Architecture Planning Design

24 BED  
 ADDITION

ISSUE DATE: 7.30.2024

REV DATE

REV	DATE

S1.1

**NOTES:**

1. PROVIDE 4" CONCRETE SLAB ON GRADE REINFORCED W/ WWF 6x6-W1.4xW1.4 OVER 10 MIL POLY VAPOR BARRIER (LAP EDGES 6" MIN.) OVER 4" POROUS BASE ALL DIMENSIONS REFERENCED TO CENTERLINE OF COLUMNS, FACE OF EXTERIOR VENEER, AND CENTERLINE OF INTERIOR BEARING WALLS. SEE ARCHITECTURAL AND STRUCTURAL SECTIONS TO DETERMINE EDGE OF SLAB. VERIFY DIMENSIONS PRIOR TO CONSTRUCTION.
2. TOP OF EXTERIOR FTG. = F.F.E., -1'-4" AND FIN. GRADE -1'-0" (MIN.)
3. SEE ARCH. DWGS. FOR DIMENSIONS NOT SHOWN.
4. SEE DETAIL 5/S-3 FOR RECESSED SLAB DETAILS.
5. SEE DETAIL 1/S-3 FOR SLAB CONTROL JOINTS (CJ), ALTERNATE LAYOUT PLANS MAY BE SUBMITTED FOR APPROVAL.
6. SEE ARCHITECTURAL DRAWINGS, FOR LOCATIONS OF RECESSED AND/OR SLOPED SLAB AREAS. PROVIDE POSITIVE DRAINAGE FROM ALL PERIMETER WALLS TO FLOOR DRAIN. COORDINATE W/ PLUMBING DWGS. SEE DETAIL 5/S-3.
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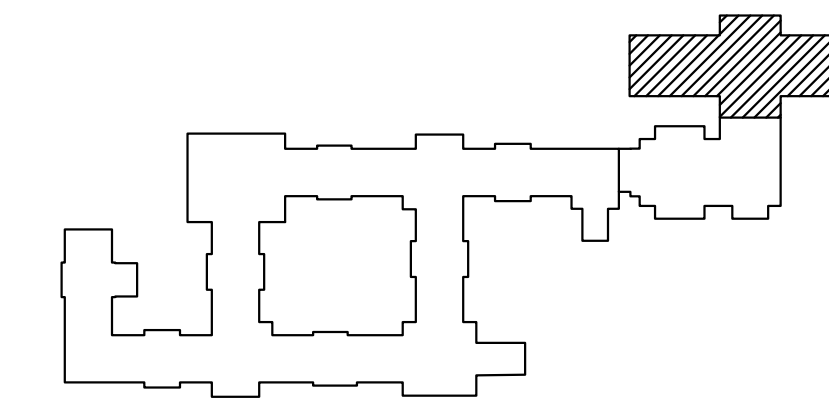
INTERIOR WALLS LABELED "SHEARWALL" DESIGNATES INTERIOR 2X4 STUDS SHEATHED W/ WITH MINIMUM 7/16" OSB ON ONE SIDE OF WALL. PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED JOINTS. PROVIDE 8d NAILS AT 6" O.C. AT ALL PANEL EDGES, 12" O.C. @ INTERMEDIATE FRAMING.

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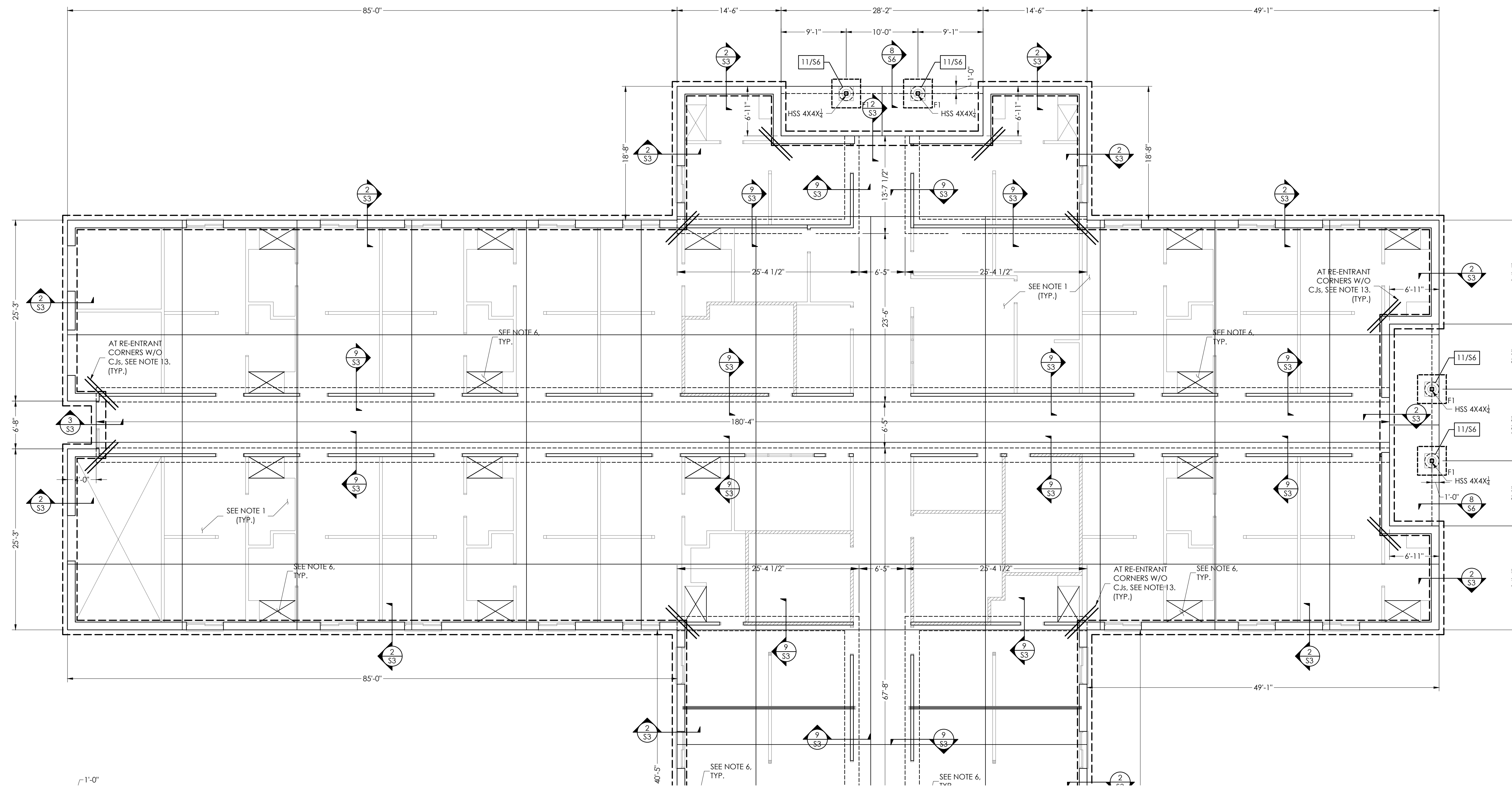
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FOOTING SCHEDULE		
TYPE	SIZE	REBAR
F1	4'-0" X 4'-0" X 1'-0"	(4) #5s (3'-6" LONG) E.W.

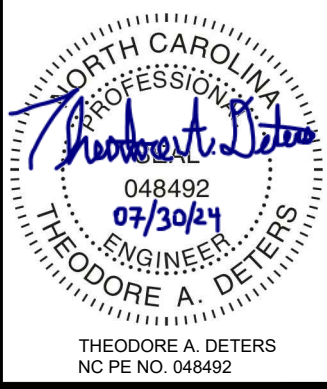


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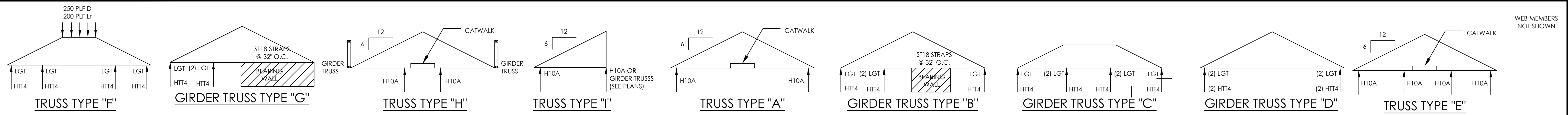
David R. Polston - Architect  
 3806 Park Ave. Suite 2-L, Wilmington, NC 28403  
 Architecture Planning Design

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HEADER AND BEAM SCHEDULE		
TYPE	SIZE	NOTES
H1	(2) 2x8	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S5
H2	(2) 2x10	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S5
H3	(2) 2x12	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S5
H4	(3) 2x8	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S5
H5	(3) 2x10	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S5
H6	(3) 2x12	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S5
H7	(2) 1 3/4" x 11 7/8" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0. STRAP EACH FACE OF HEADER TO FACE OF JACK STUD W/ CS16 24" LONG. PROVIDE HT4 HOLDDOWN AT STUD BASE.
H8	(3) 1 3/4" x 11 7/8" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0
H9	(2) 1 3/4" x 14" LVL FLUSH BEAM. BOTTOM OF BEAM FLUSH WITH BOTTOM OF ROOF TRUSSES	Fb=2850 PSI, E=2.0. STRAP ENDS OF BEAM TO STUD GROUP BELOW W/ (2) H6 TIES. PROVIDE HT4 HOLDDOWN AT STUD BASE.
H10	(2) 1 3/4" x 9 1/4" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0
H11	(3) 1 3/4" x 9 1/4" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0
B1	W8x18 STEEL BEAM. T.O.S = 8'-10 1/2" AFF	
B2	W8x28 STEEL BEAM. T.O.S = 8'-10 1/2" AFF, U.N.O.	
B3	W8x40 STEEL BEAM. T.O.S = 8'-10 1/2" AFF	
B4	W12x50 STEEL BEAM. T.O.S = 8'-10 1/2" AFF	
B5	C12x20.7 STEEL BEAM. T.O.S = 8'-10 1/2" AFF, U.N.O.	
B6	(2) 8" DEEP BOND BEAMS	PROVIDE (2) #5 CONTINUOUS BARS IN EACH BOND BEAM

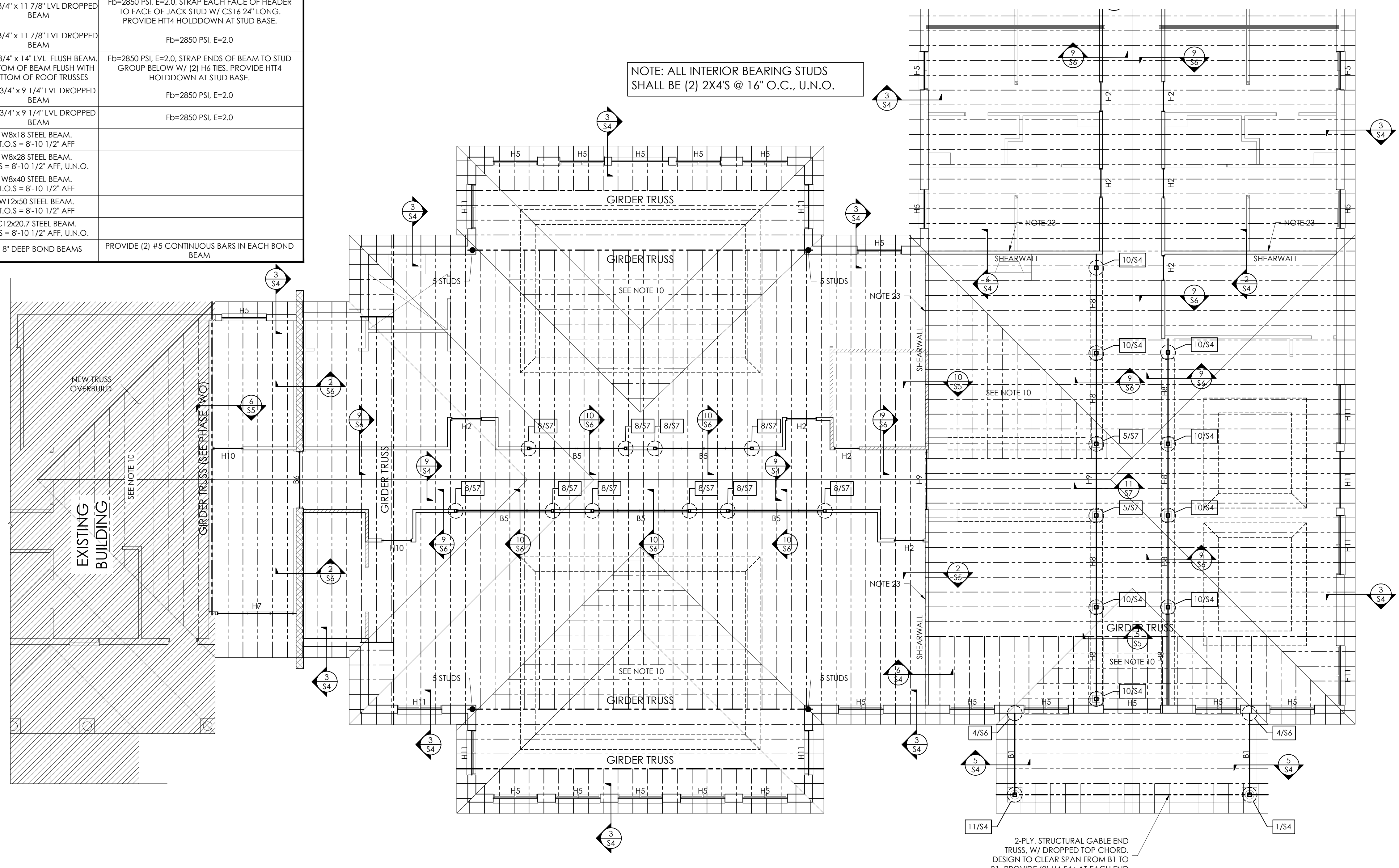
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 \* BOTTOM CHORD RECESSES ARE NOT SHOWN FOR CLARITY. SEE PLANS FOR LOCATIONS OF RECESSES.

INTERIOR WALLS LABELED "SHEARWALL" DESIGNATES INTERIOR 2X4 STUDS SHEATHED W/ WITH MINIMUM 7/16" OSB ON ONE SIDE OF WALL. PROVIDE HORIZONTAL 2x BLOCKS AT ALL UNSUPPORTED JOINTS. PROVIDE 8d NAILS AT 6" O.C. AT ALL PANEL EDGES, 12" O.C. @ INTERMEDIATE FRAMING.

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- NOTES:**
- ALL TRUSS SPACING IS AT 2'-0" O.C. UNLESS NOTED OTHERWISE. SPACE TRUSSES AT ATTIC ACCESS DOORS TO ALLOW FOR PROPER INSTALLATION.
  - TRUSS FABRICATOR SHALL VERIFY ALL DIMENSIONS, LAYOUTS AND COORDINATE WITH BEARING WALL AND BEAM LOCATIONS. ALTERNATE LAYOUT PLANS MAY BE SUBMITTED FOR APPROVAL.
  - THE CONTRACTOR MUST VERIFY THAT ALL LATERAL BRACING REQUIRED FOR TRUSS WEBS IS INSTALLED PER THE TRUSS SHOP DRAWINGS AND DETAIL 4/S-5.
  - REFER TO FOUNDATION PLAN FOR DIMENSIONS AND TO ARCHITECTURAL PLANS FOR DIMENSIONS NOT SHOWN.
  - DESIGN ROOF TRUSSES FOR ADDITIONAL MECHANICAL, SPRINKLER, AND ARCHITECTURAL LOADS AS REQUIRED.
  - ALL TRUSS TO TRUSS CONNECTIONS SHALL BE SPECIFIED BY THE TRUSS DESIGNER AND SHALL BE CLEARLY INDICATED ON THE TRUSS SHOP DRAWINGS.
  - SEE DETAIL 7/S-4 OR 8/S-4 FOR ROOF DECK NAILING PATTERN.
  - PROVIDE 1x4x4x1/2" MIN. LOOSE LAID BRICK UNTEL WITH 4" BEARING ABOVE ALL OPENINGS UP TO 8'-0" WHERE OPENINGS EXCEED 8'-0" PROVIDE 1x6x4x1/2" UNTEL BOLTED DIRECTLY TO HEADER WITH 1/2" Ø THRU BOLTS.
  - VERIFY LOCATIONS AND AMOUNTS OF ALL HEADERS.
  - PRE-FABRICATED TRUSS OVER-BUILD FRAMING. ROOF SHEATHING SHALL BE CONTINUOUS BENEATH TRUSS OVERBUILD. PROVIDE ATTACHMENT OF OVERBUILD FRAMING TO ROOF SHEATHING AND TRUSSES BELOW ACCORDING TO TRUSS MANUFACTURER.
  - SEE ARCH. DWGS. FOR LOCATIONS OF FIRE/SMOKE WALLS AND DRAFT PARTITIONS. TRUSSES MUST BE COORDINATED WITH FIRE/SMOKE WALLS. WHERE ARCHITECTURAL PLANS REQUIRE SMOKE/FIRE WALLS TO EXTEND TO UNDER SIDE OF ROOF SHEATHING, THE TRUSSES MUST BE STOP AT THE FACE OF THE WALL.
  - BOTTOM CHORD RAISED TWO FEET FOR RECESSED CEILING - DASHED LINE SHOWS APPROXIMATE LOCATION. VERIFY ALL LOCATIONS WITH ARCH DWGS.
  - VERIFY ATTIC ACCESS LOCATIONS W/ ARCH. DWGS. SPACE TRUSSES AS REQUIRED FOR PROPER INSTALLATION. WHERE TRUSS SPACING EXCEEDS 24" O.C., LADDER BLOCK BETWEEN CHORDS WITH 2x BLOCKING @ 24" O.C.
  - SEE DETAIL 12/S5 FOR TOP PLATE SPLICE DETAIL.
  - SEE DETAILS 3/S-5 AND 4/S-5 FOR PERMANENT ROOF TRUSS BRACING.
  - DESIGN ROOF TRUSSES TO INCORPORATE FIXED WINDOW INSTALLATION. COORDINATE WITH ARCHITECTURAL DRAWINGS.
  - PROVIDE (4) 2x6 AT EXTERIOR WALL AND (5) 2x4 @ INTERIOR WALL BELOW ALL GIRDER TRUSS BEARING POINTS PROVIDE LGT TIE DOWN WITH HT4 AT STUD BASE.
  - SMOKE WALLS EXTEND THROUGH TRUSS OVERBUILD TO ROOF SHEATHING. BREAK TRUSS OVERBUILD ON BOTH SIDES OF WALL.
  - TRUSS CLIPS AT ENDS OF TRUSSES HAVE BEEN DESIGNED TO TRANSFER LATERAL SHEAR LOAD AND UPLIFT INTO THE WALLS. ANY SUBSTITUTIONS MUST BE APPROVED BY THE EOR. H10A TIE DOWNS AT EXTERIOR WALLS MUST BE APPLIED OVER THE EXTERIOR WALL OSB SHEATHING.
  - REFER TO ARCHITECTURAL PLANS FOR LOCATION OF DORMERS ON MAIN ROOF. DORMERS SHALL BE FRAMED USING 2x4 STUDS AT 16" O.C. WITH 2X4 RAFTERS AND COLLAR TIES AT 24" O.C. PROVIDE 2x6 LADDER BLOCKING BETWEEN TRUSSES FOR ATTACHMENT OF THE DORMER WALL SILL PLATE. PROVIDE A MINIMUM OF (1) 1/2" X 3" WOOD SCREW AT 24" O.C. FROM DORMER SILL TO BLOCKING BETWEEN TRUSSES. THE MAIN ROOF SHEATHING MUST EXTEND BELOW DORMER. IF REQUIRED CUT A MAXIMUM 20"x36" HOLE IN THE MAIN ROOF SHEATHING BELOW THE DORMER FOR VENTILATION.
  - COORDINATE WITH MP AND E DRAWINGS FOR THE ROOF TOP PLATFORM AND EXTERIOR LADDER LOCATION. SEE SHEET S7 FOR DETAILS.
  - OMITTED
  - ALIGN DRAG TRUSS WITH SHEAR WALL PER DETAIL 6/S4. DESIGN DRAG TRUSS TO TRANSFER 200 PLF LATERAL LOAD FROM TOP CHORD TO BOTTOM CHORD. LATERAL LOAD IS RESISTED BY SHEAR WALL BELOW.
  - PROVIDE DOUBLE DROPPED STRUCTURAL GABLE END TRUSS AT END OF PORCH ROOF. HANG SOFFIT FRAMING FROM BOTTOM CHORD OF TRUSSES. TRUSS DESIGNER TO DESIGN GABLE END TO SUPPORT AN ADDITIONAL 150 PLF DEAD LOAD AT THE BOTTOM CHORD.
  - SEE DETAIL 10 ON SHEET S9 FOR ROOF TOP CURB ATTACHMENTS.
  - TRUSS MANUFACTURER TO COORDINATE FIXED WINDOW OPENINGS IN GABLE END TRUSSES - SEE ARCH ELEVATIONS.
  - OMITTED

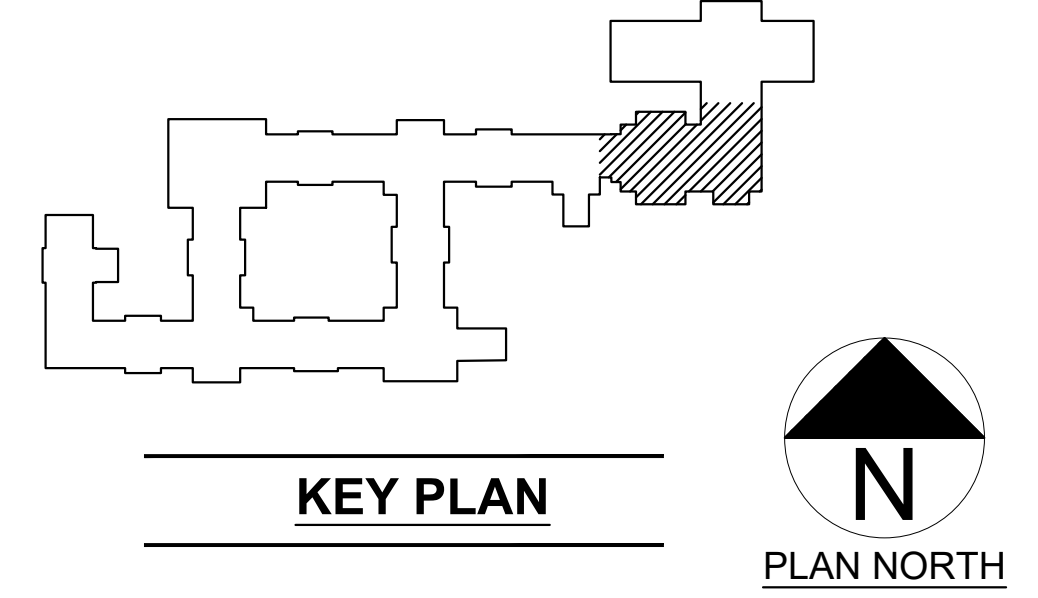
NOTE: ALL INTERIOR BEARING STUDS SHALL BE (2) 2X4S @ 16" O.C., U.N.O.



2-PLY. STRUCTURAL GABLE END TRUSS, W/ DROPPED TOP CHORD. DESIGN TO CLEAR SPAN FROM B1 TO B1. PROVIDE (2) H4.5AS AT EACH END

**PHASE ONE PARTIAL FRAMING PLAN**

SCALE: 1/8"=1'-0"



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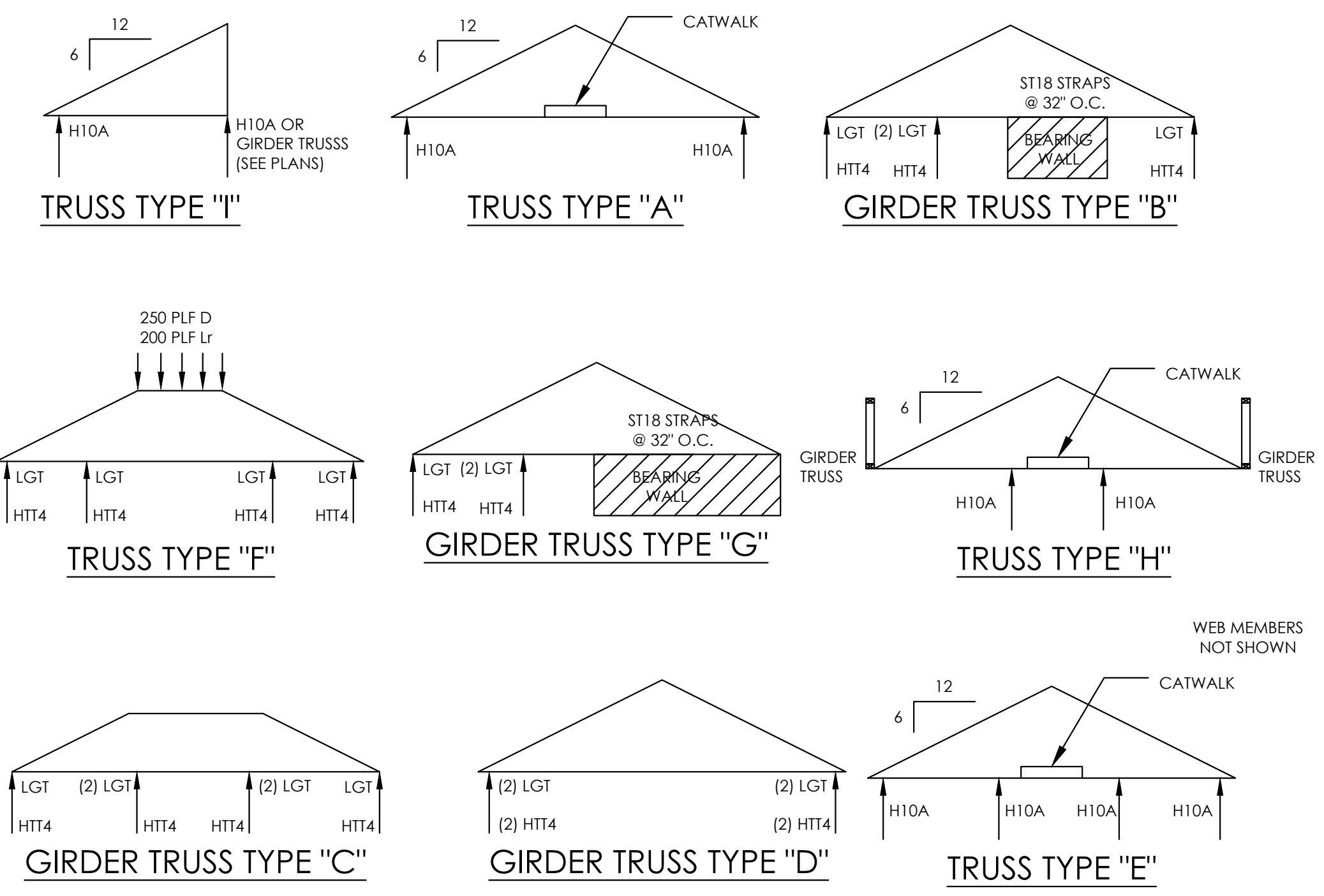
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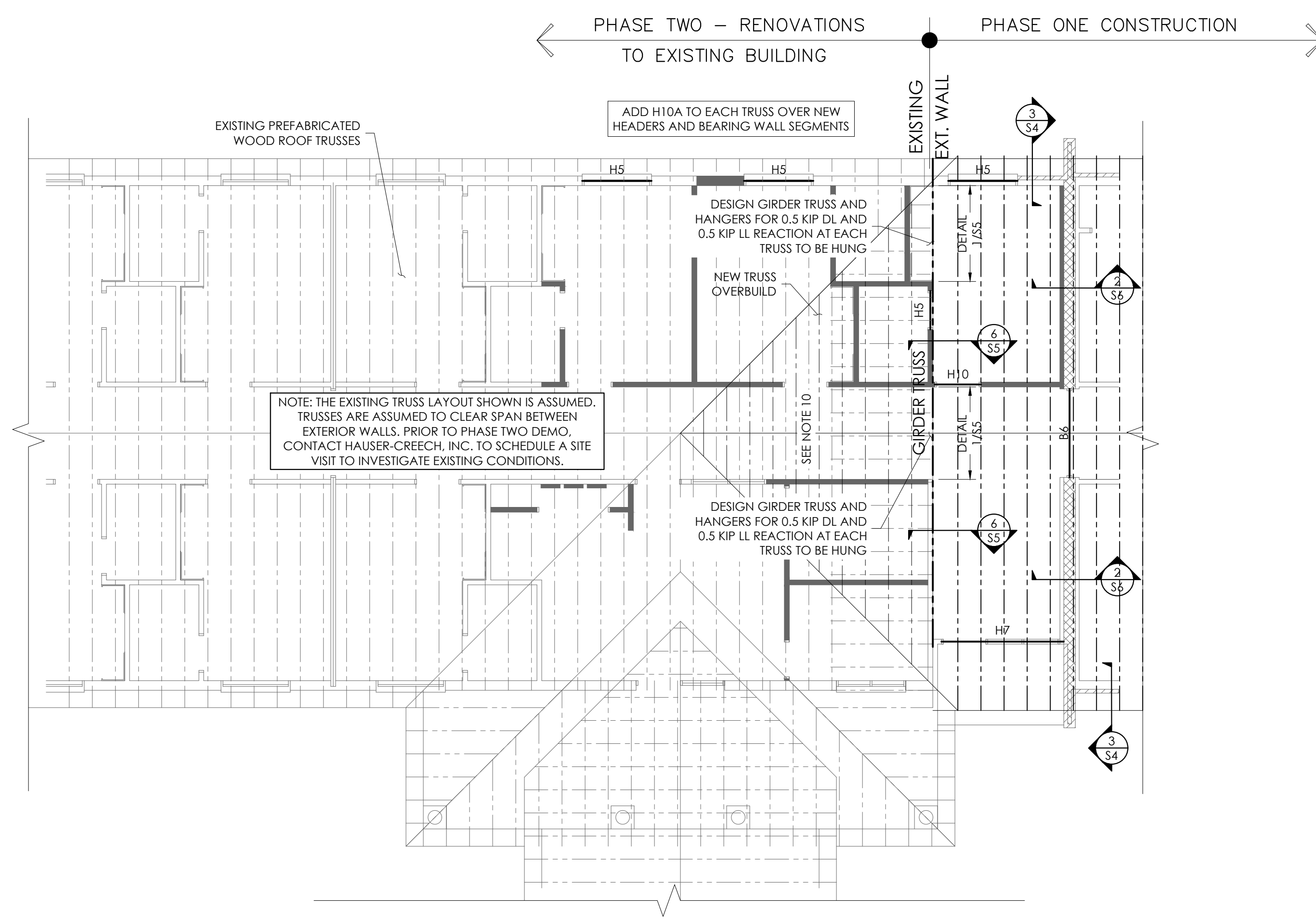
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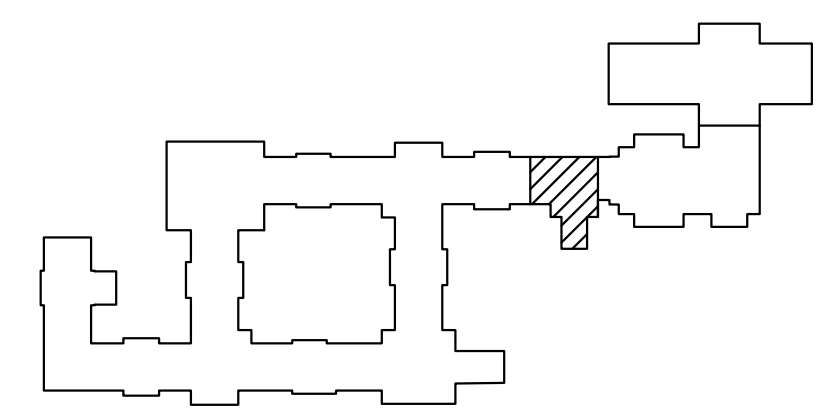
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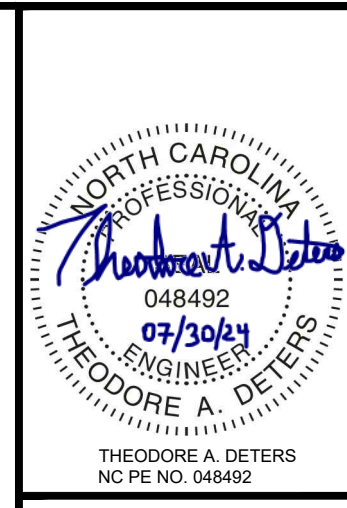
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H2	(2) 2x10	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S5
H3	(2) 2x12	W/ (1) 1/2" PLYWOOD SPACER. SEE 7/S5
H4	(3) 2x8	W/ (2) 1/2" PLYWOOD SPACERS. SEE 7/S5
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H7	(2) 1 3/4" x 11 7/8" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0. STRAP EACH FACE OF HEADER TO FACE OF JACK STUD W/ C116 24" LONG. PROVIDE HTT4 HOLDDOWN AT STUD BASE.
H8	(3) 1 3/4" x 11 7/8" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0
H9	(2) 1 3/4" x 14" LVL FLUSH BEAM. BOTTOM OF BEAM FLUSH WITH BOTTOM OF ROOF TRUSSES	Fb=2850 PSI, E=2.0. STRAP ENDS OF BEAM TO STUD GROUP BELOW W/ (2) H6 TIES. PROVIDE HTT4 HOLDDOWN AT STUD BASE.
H10	(2) 1 3/4" x 9 1/4" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0
H11	(3) 1 3/4" x 9 1/4" LVL DROPPED BEAM	Fb=2850 PSI, E=2.0
B1	W8x18 STEEL BEAM. T.O.S = 8'-10 1/2" AFF	
B2	W8x28 STEEL BEAM. T.O.S = 8'-10 1/2" AFF. U.N.O.	
B3	W8x40 STEEL BEAM. T.O.S = 8'-10 1/2" AFF	
B4	W12x50 STEEL BEAM. T.O.S = 8'-10 1/2" AFF	
B5	C12x20.7 STEEL BEAM. T.O.S = 8'-10 1/2" AFF. U.N.O.	
B6	(2) 8" DEEP BOND BEAMS	PROVIDE (2) #5 CONTINUOUS BARS IN EACH BOND BEAM



**PHASE TWO FRAMING PLAN**  
 SCALE: 1/8"=1'-0"



**KEY PLAN**



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**UNI HEALTH**  
**POST-ACUTE CARE**  
 RALEIGH, NC

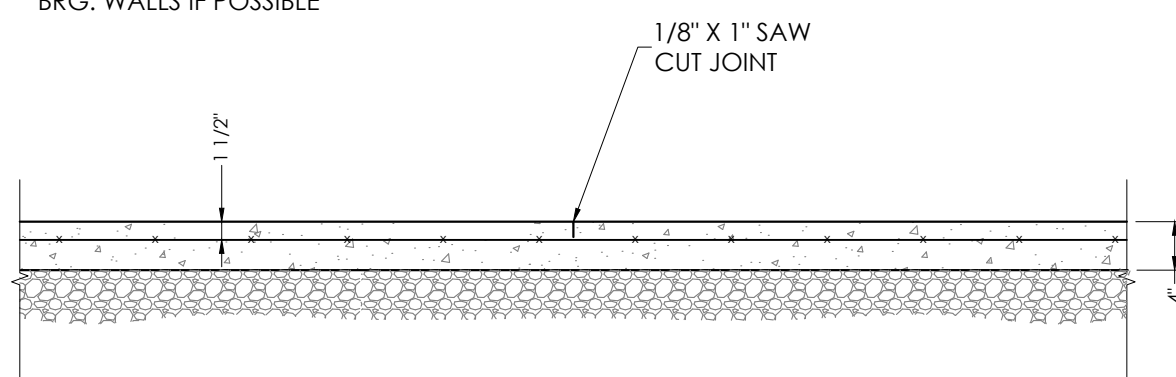
**David R. Polston - Architect**  
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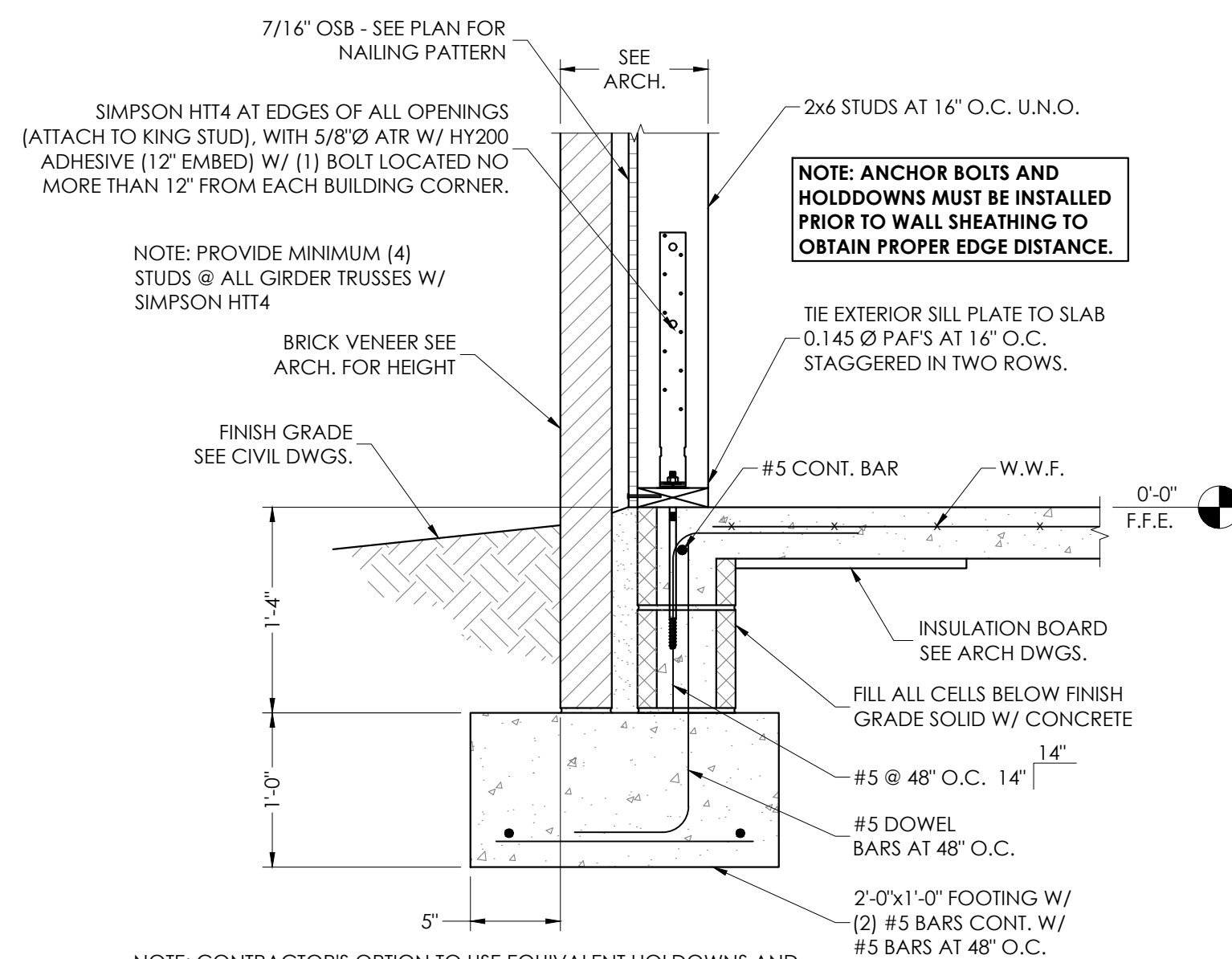
**S2.3**



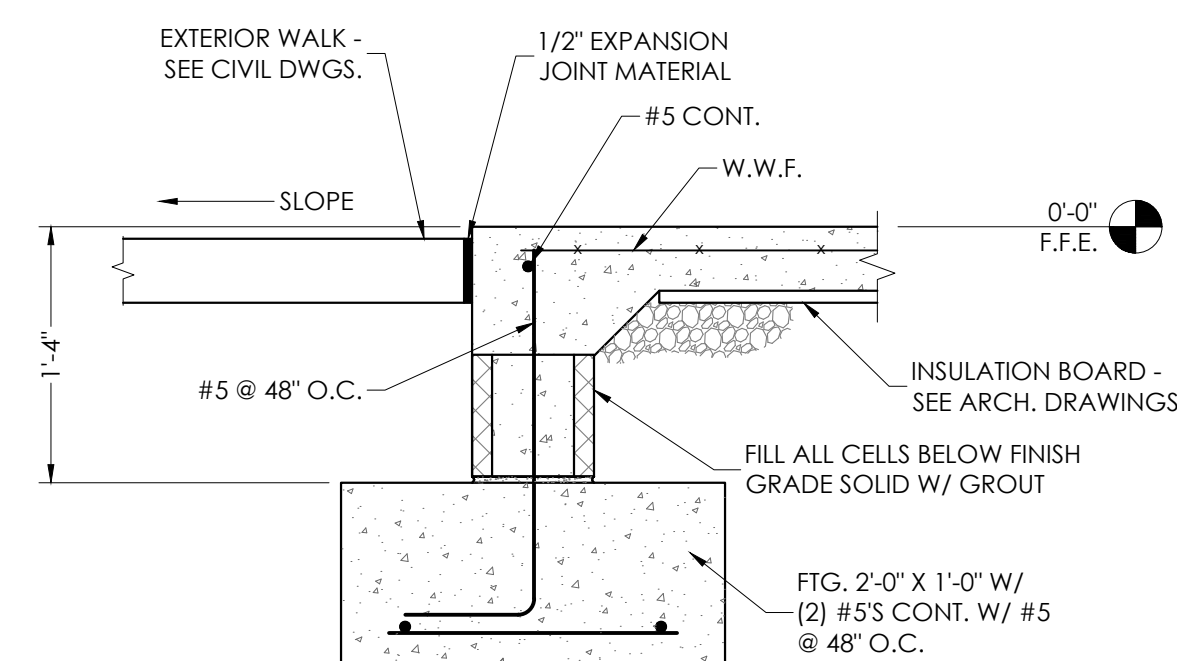
NOTE:  
MAXIMUM JOINT SPACING SHALL  
BE 16 FT. IN EACH DIRECTION  
UNLESS SHOWN OTHERWISE ON PLAN  
LOCATED UNDER NON-LOAD  
BRG. WALLS IF POSSIBLE



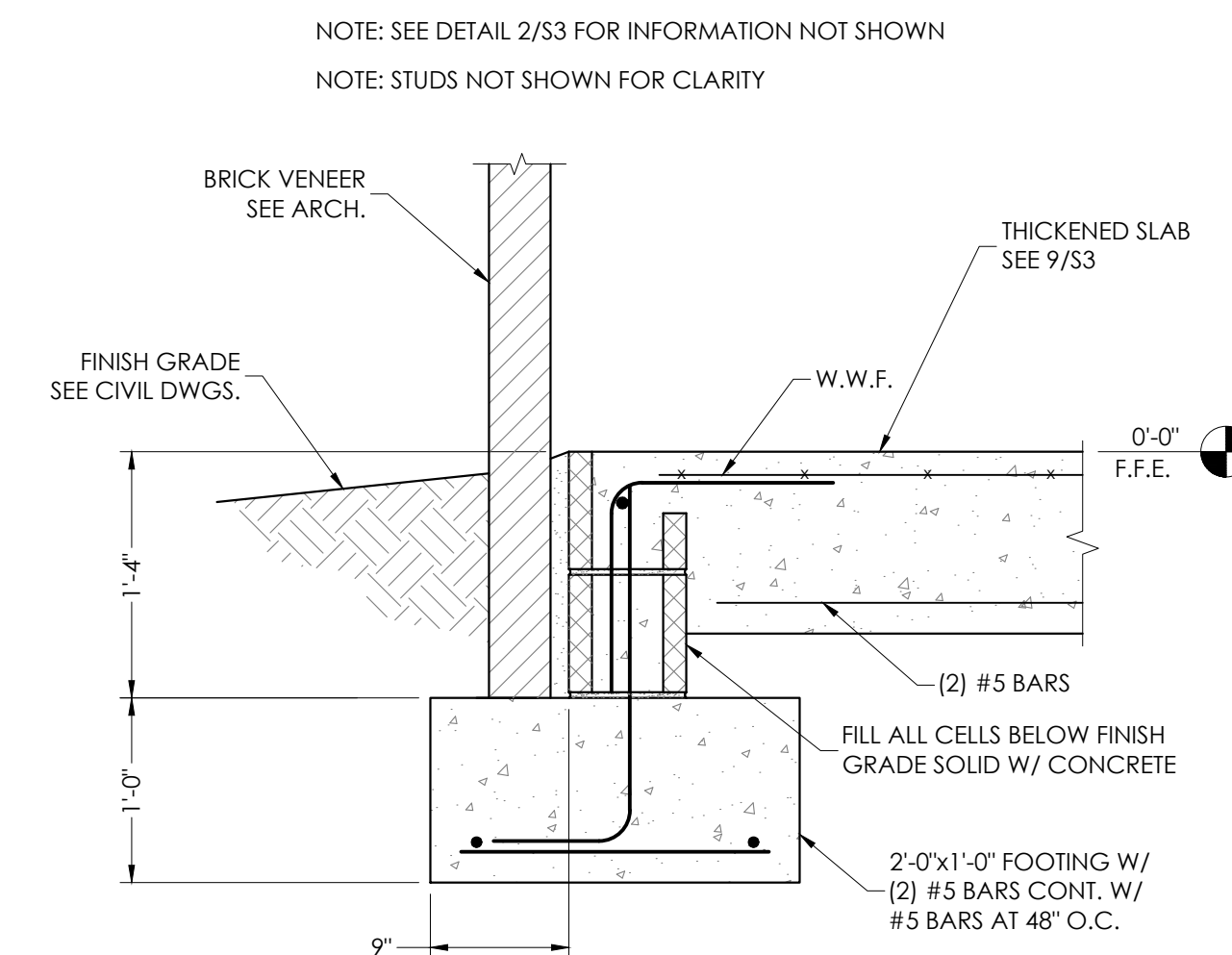
**1**  
**S-3** **SLAB ON GRADE JOINTS**  
SCALE: NONE



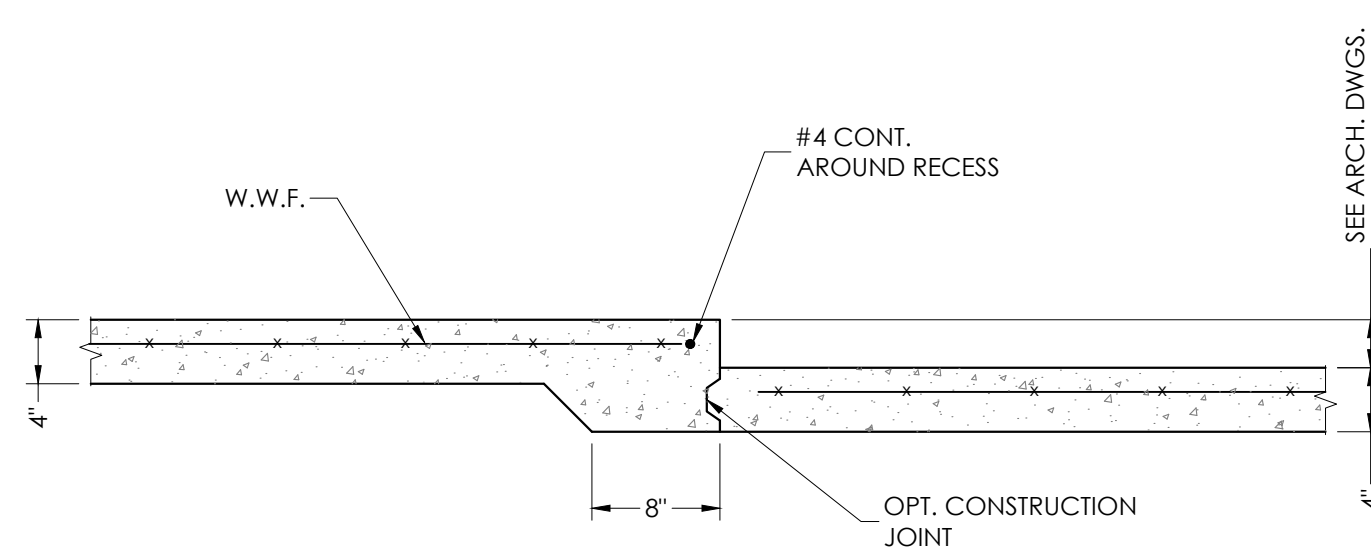
**2**  
**S-3** **EXTERIOR WALL SECTION**  
SCALE: NONE



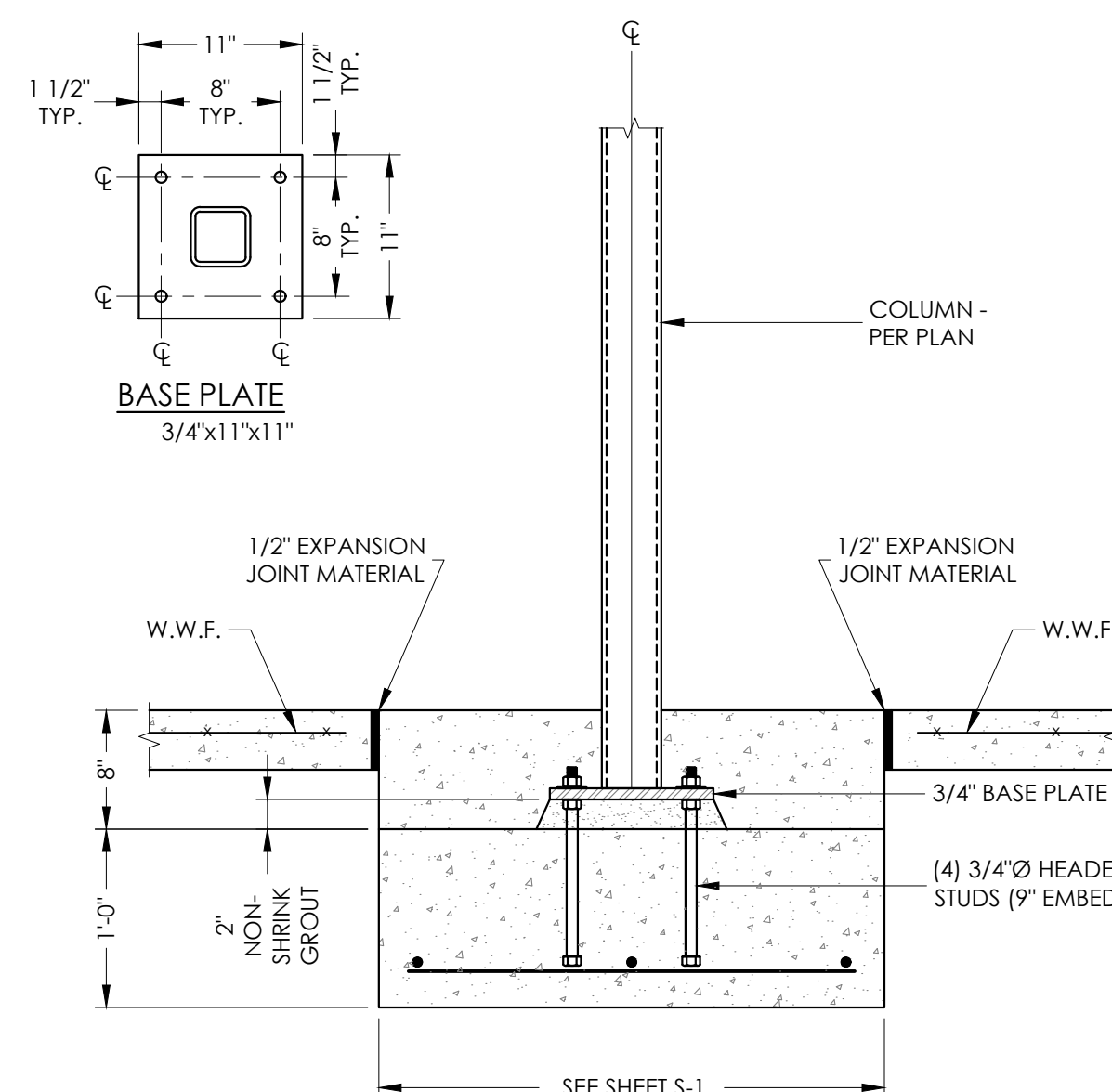
**3**  
**S-3** **EXTERIOR DOOR SECTION**  
SCALE: NONE



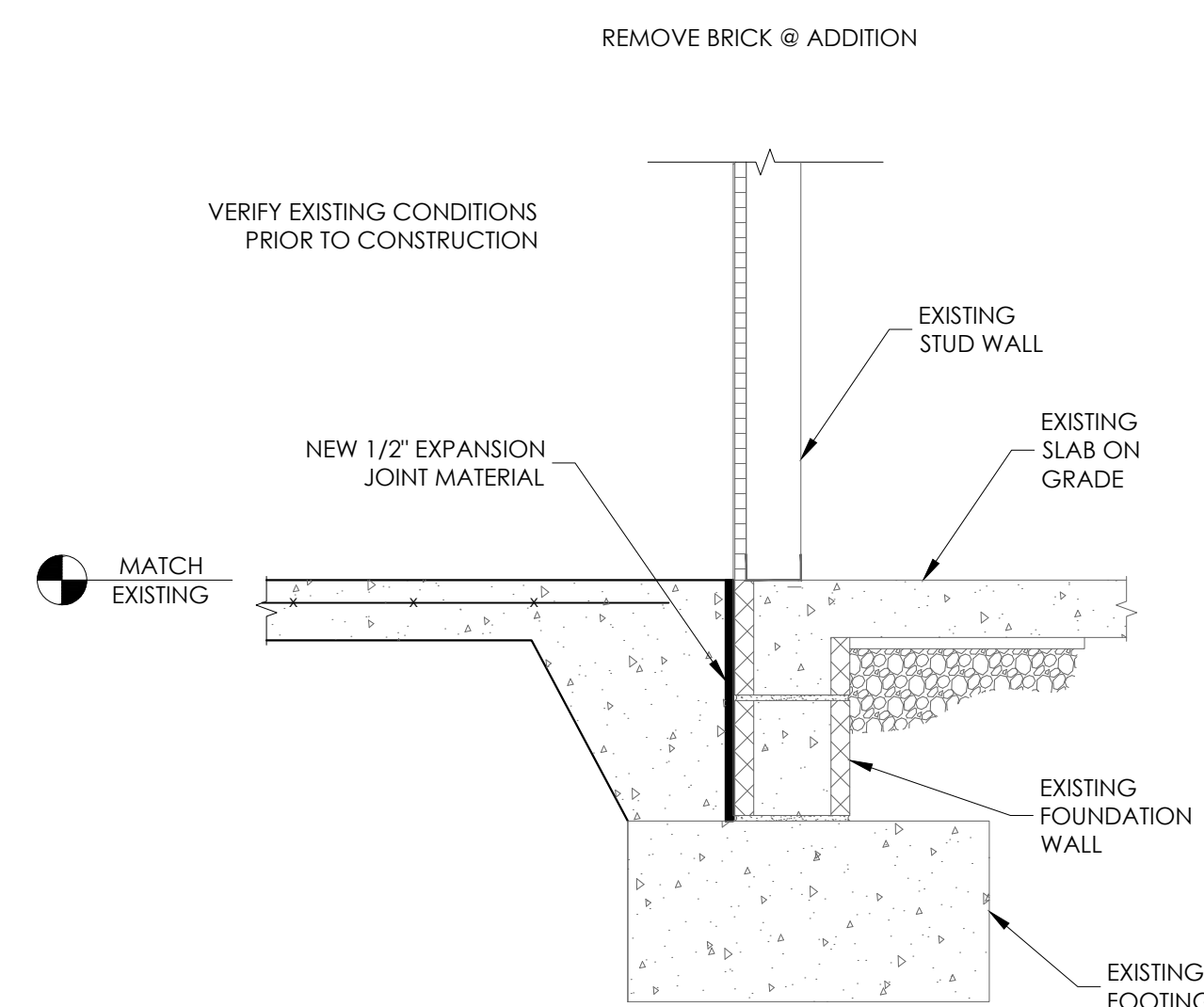
**4**  
**S-3** **SECTION @ THICKENED SLAB**  
SCALE: NONE



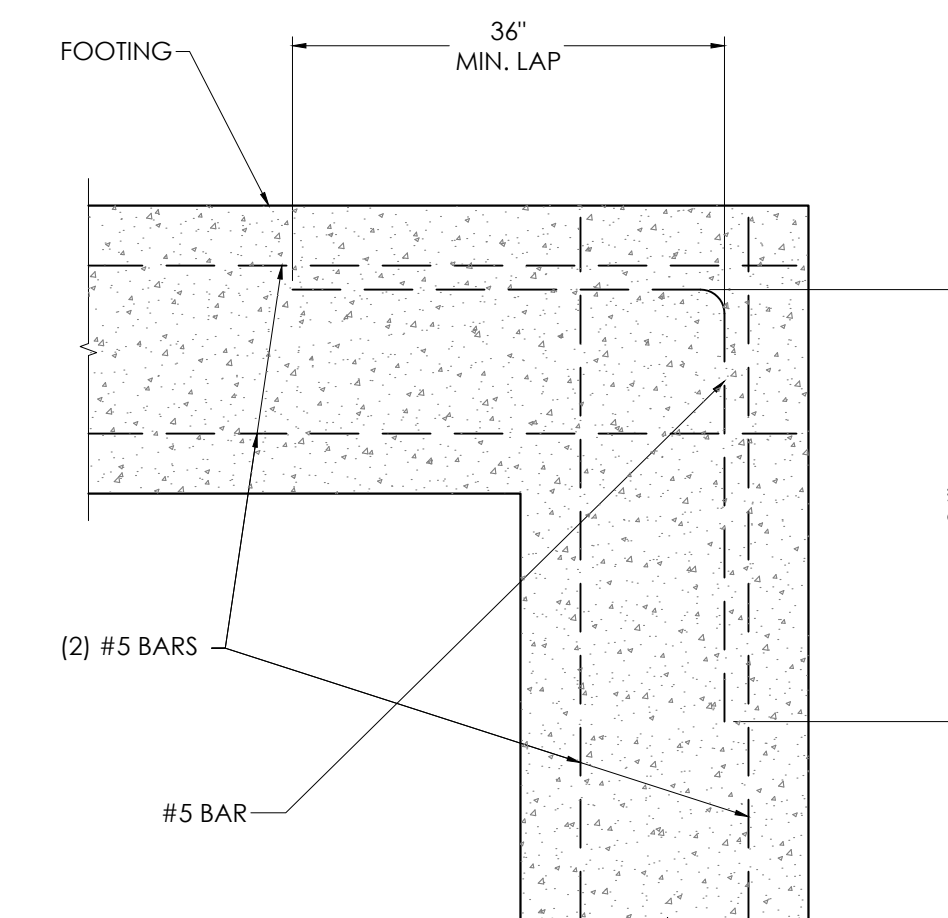
**5**  
**S-3** **TYP. RECESSED SLAB**  
SCALE: NONE



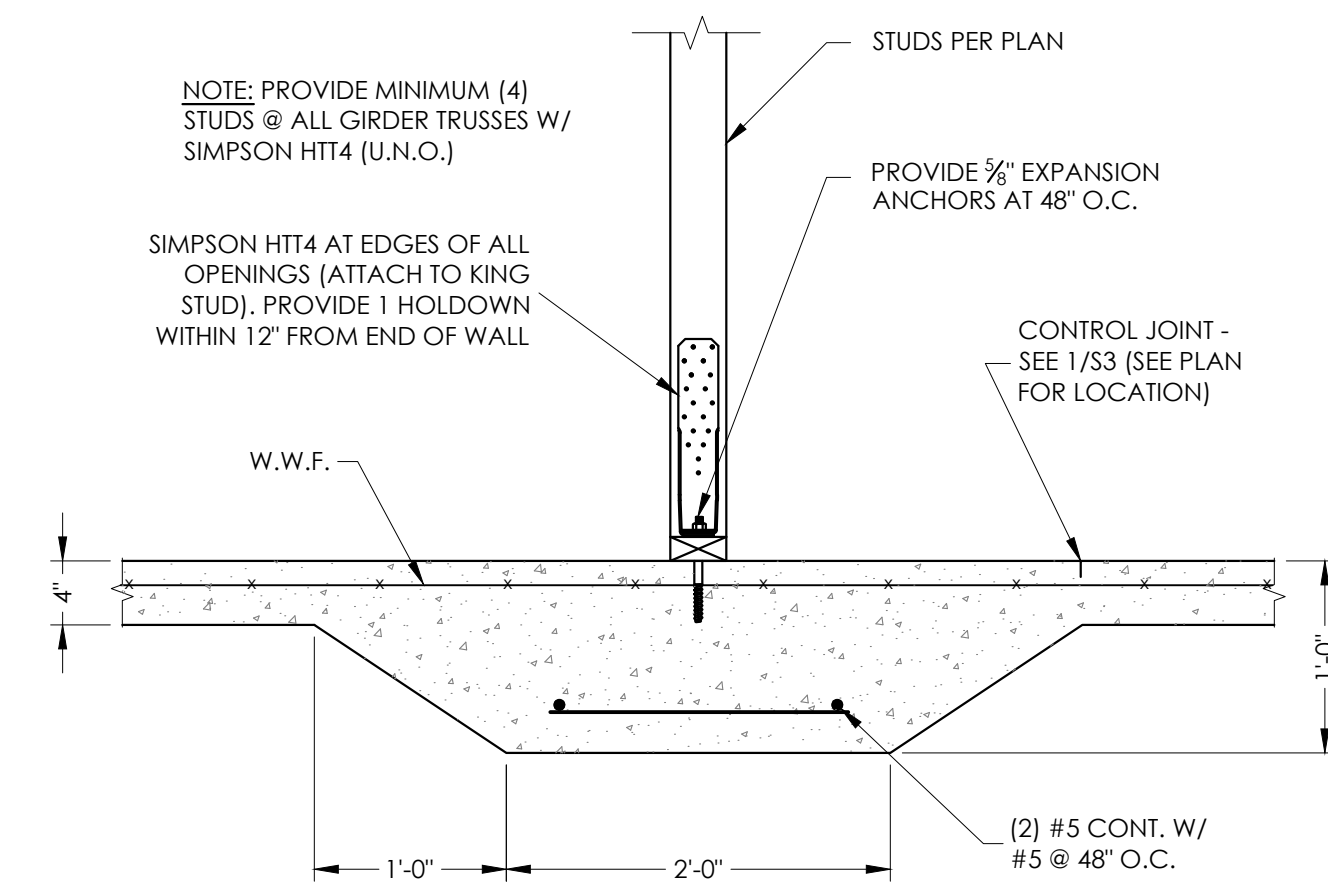
**6**  
**S-3** **SECTION @ INTERIOR COLUMN**  
SCALE: NONE



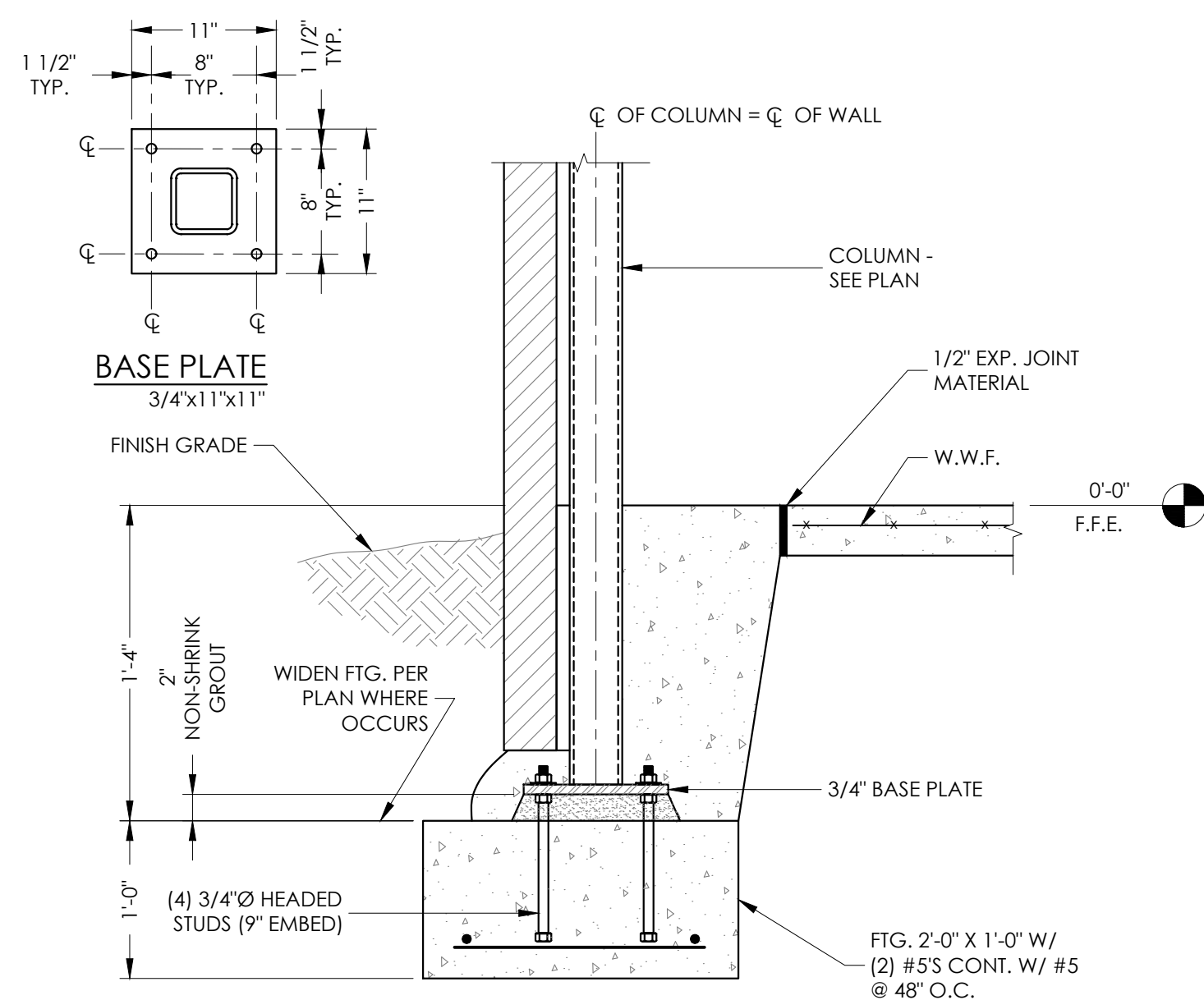
**7**  
**S-3** **NEW TO EXISTING FOUNDATION**  
SCALE: NONE



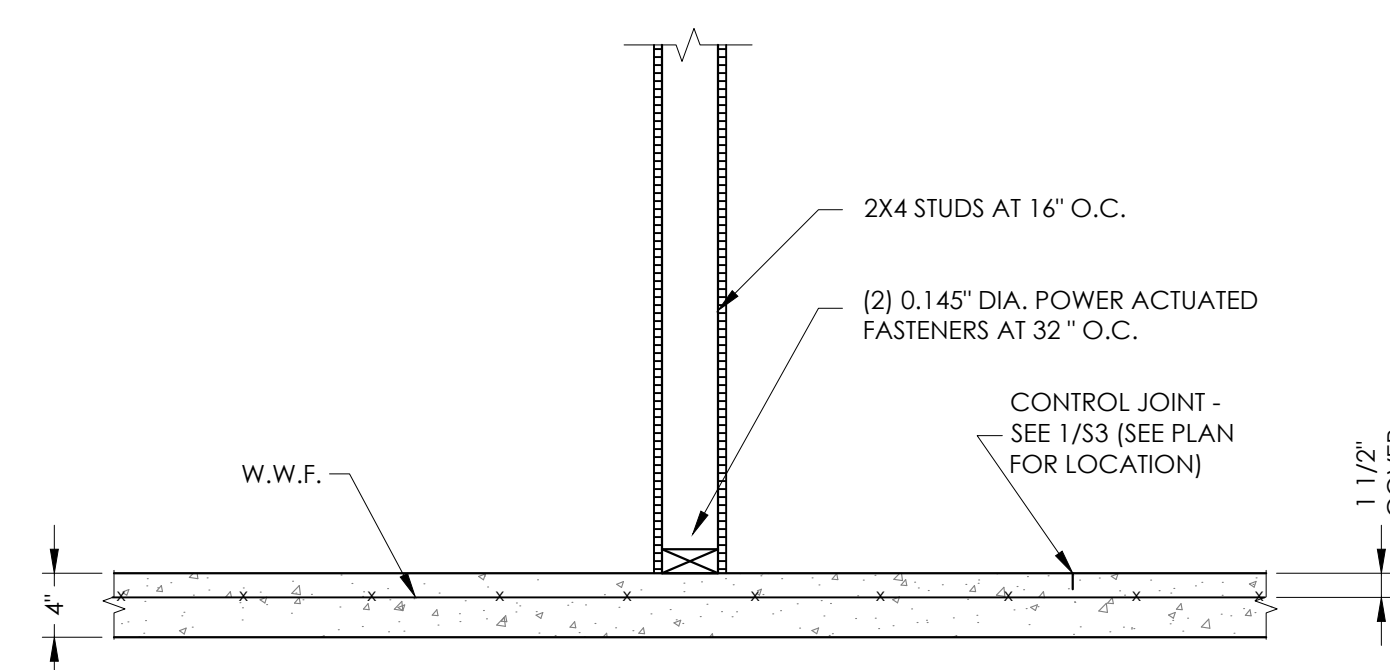
**8**  
**S-3** **TYP. CONTINUITY CORNER**  
SCALE: NONE



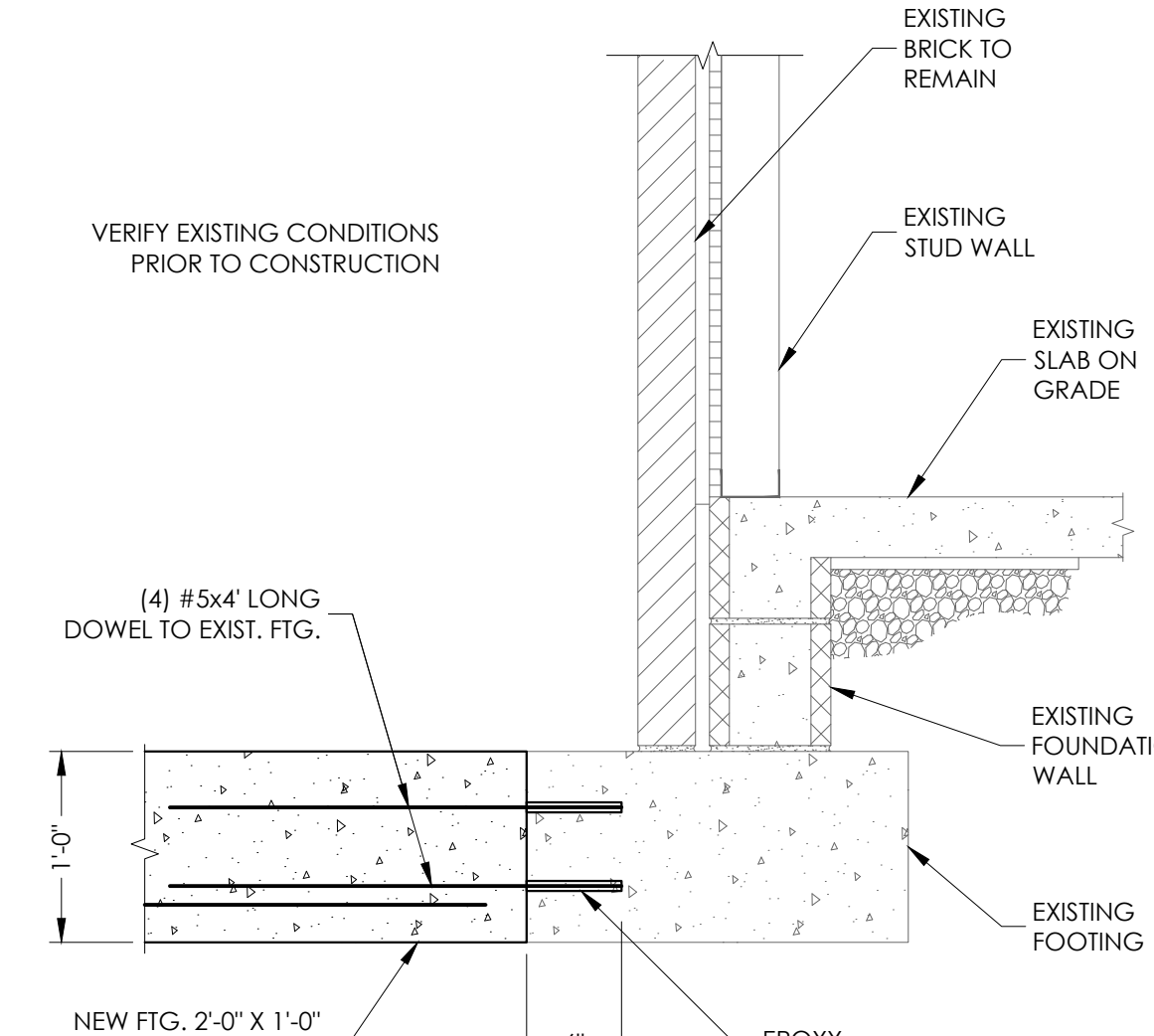
**9**  
**S-3** **THICKENED SLAB**  
SCALE: NONE



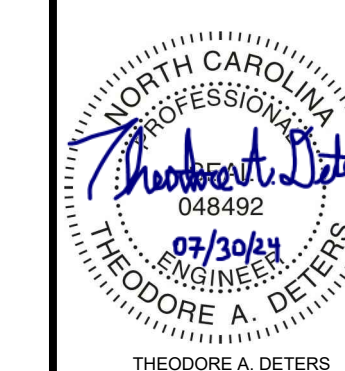
**10**  
**S-3** **SECTION @ EXTERIOR WALL**  
SCALE: NONE



**11**  
**S-3** **SECTION @ NON-LOAD BEARING STUDS**  
SCALE: NONE



**12**  
**S-3** **NEW TO EXISTING FOUNDATION**  
SCALE: NONE



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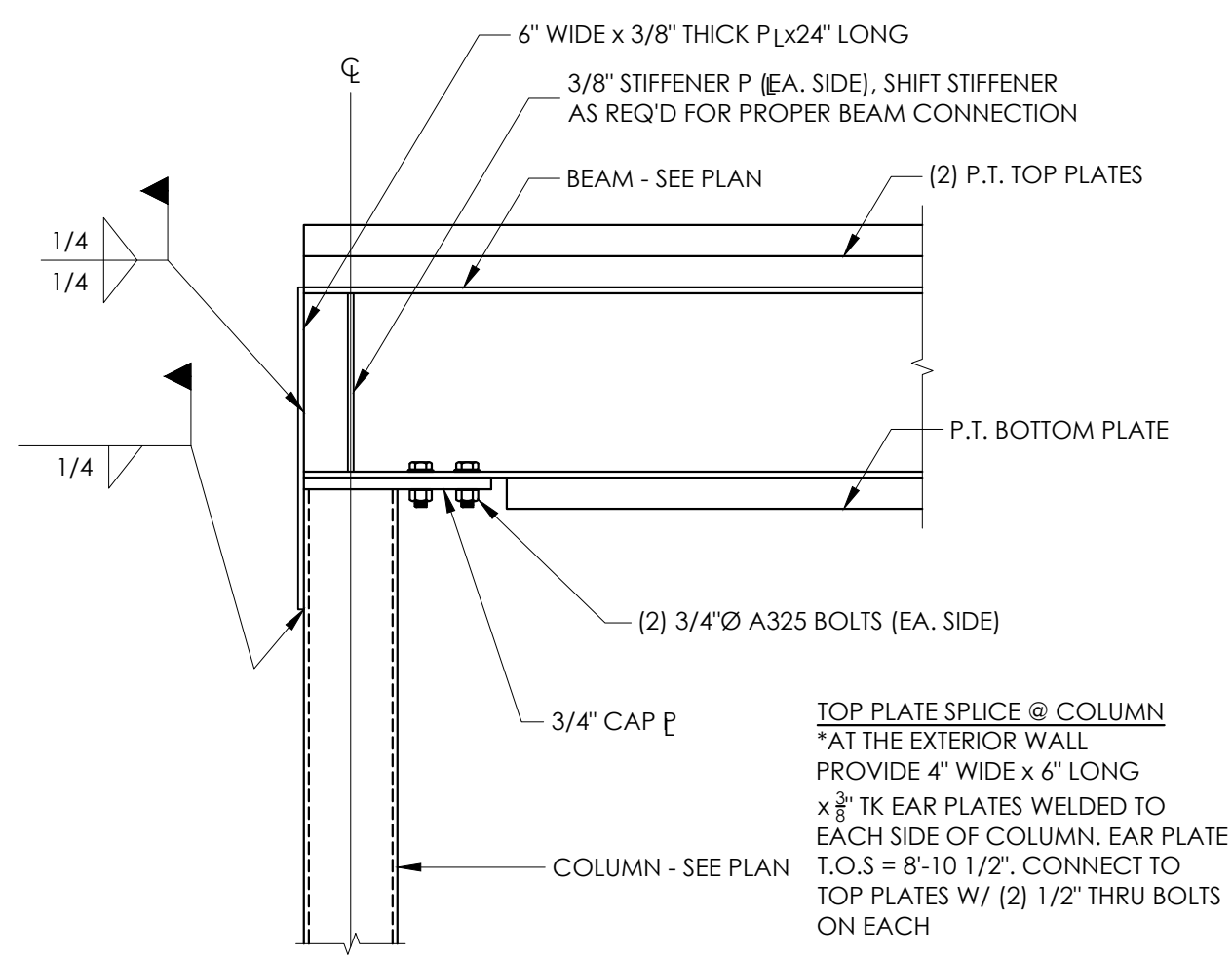
**24 BED**  
**ADDITION**

ISSUE DATE: 7.30.2024

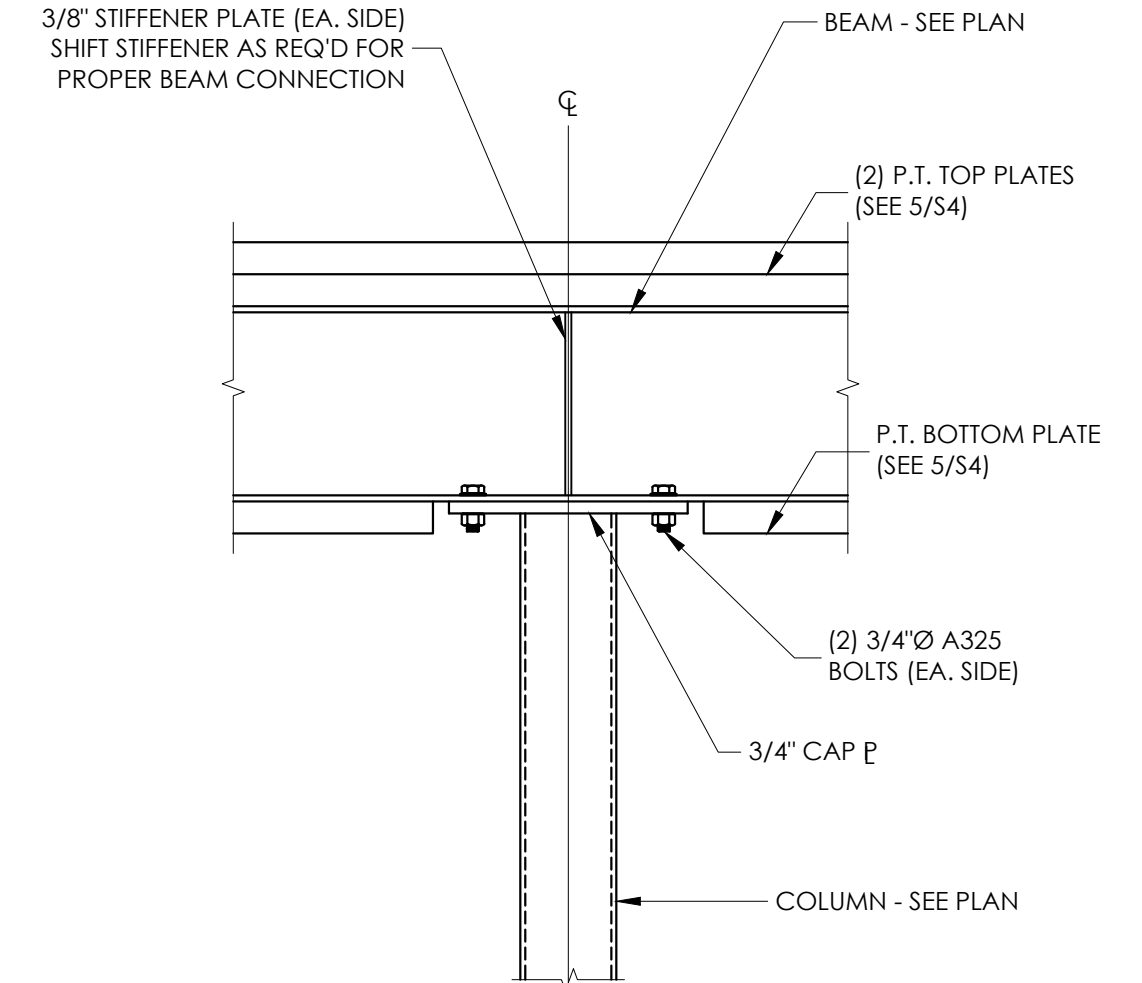
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**S3**

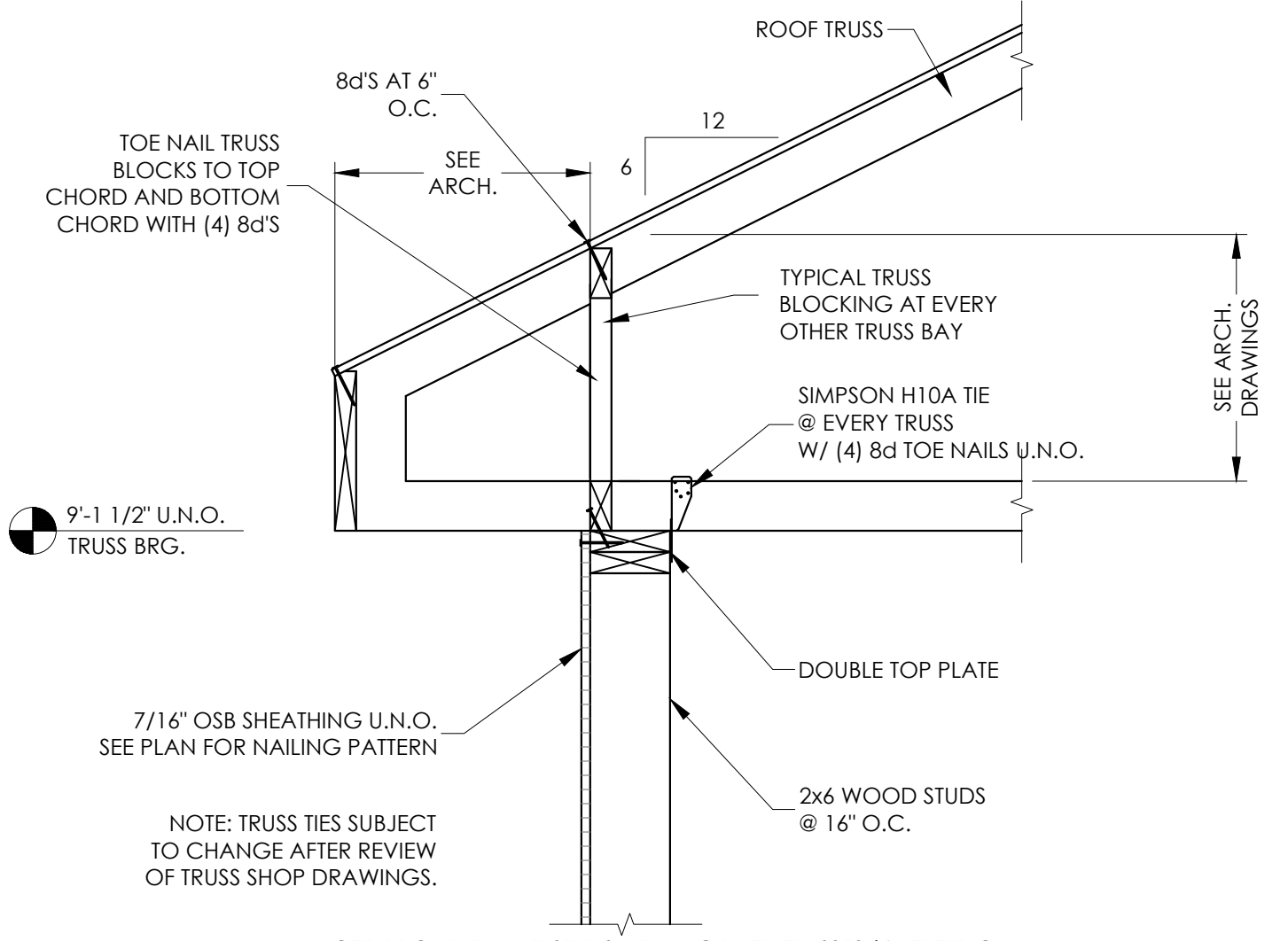




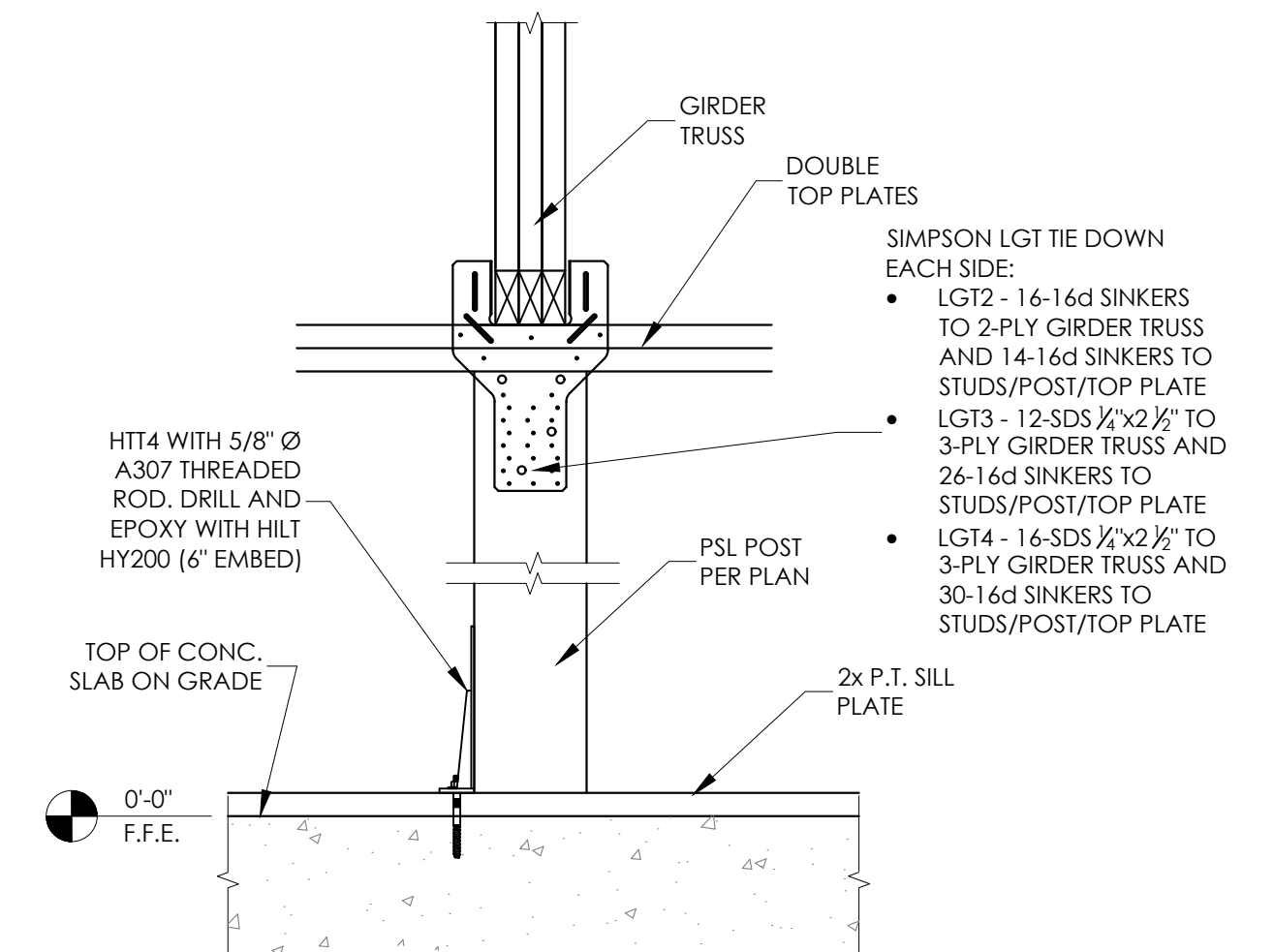
**1** BEAM DETAIL @ COLUMN  
S-4 SCALE: NONE



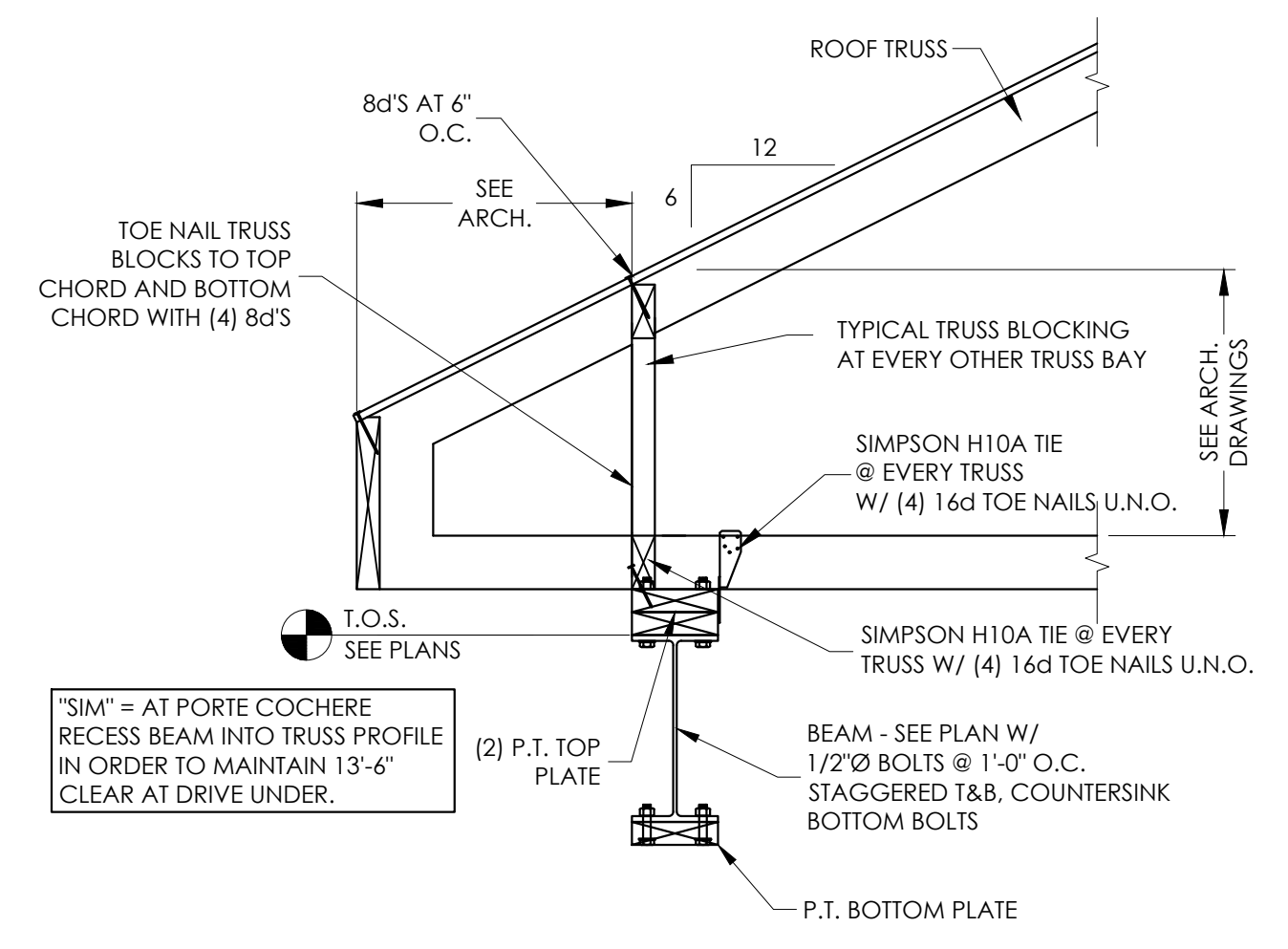
**2** BEAM DETAIL @ COLUMN  
S-4 SCALE: NONE



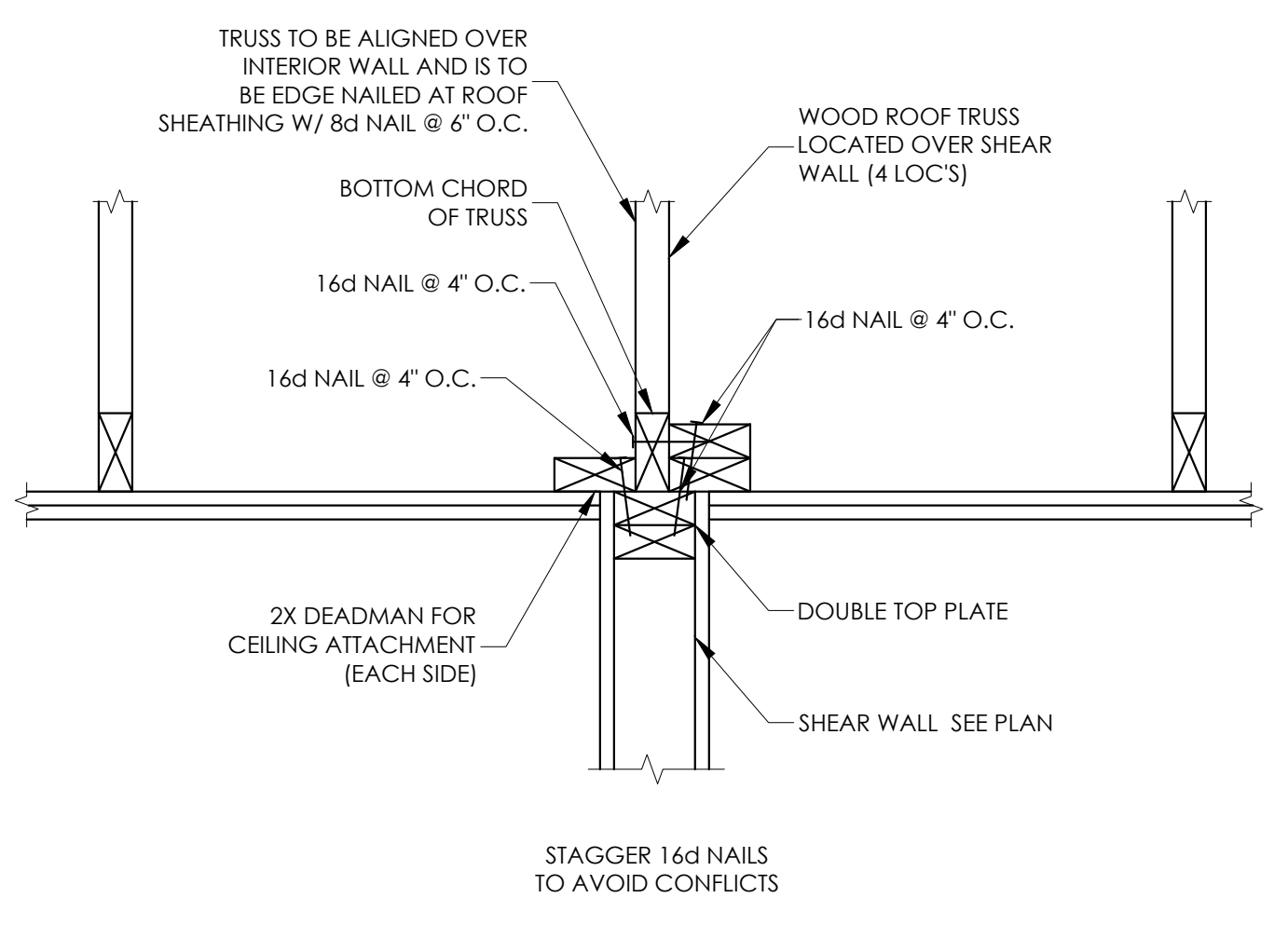
**3** PREFAB. TRUSS @ EXT. WALL  
S-4 SCALE: NONE



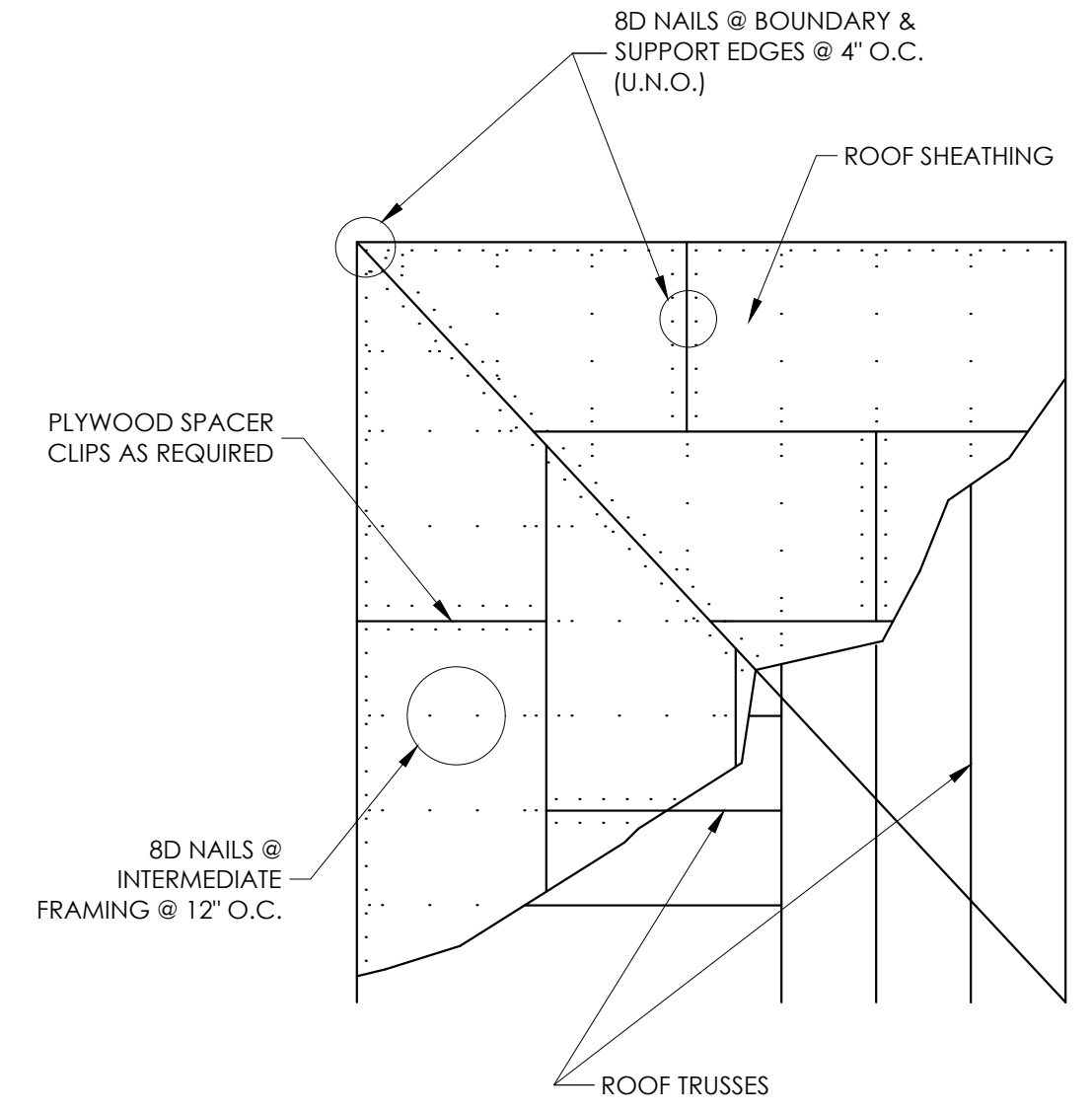
**4** GIRDER TIE DOWN  
S-4 SCALE: NONE



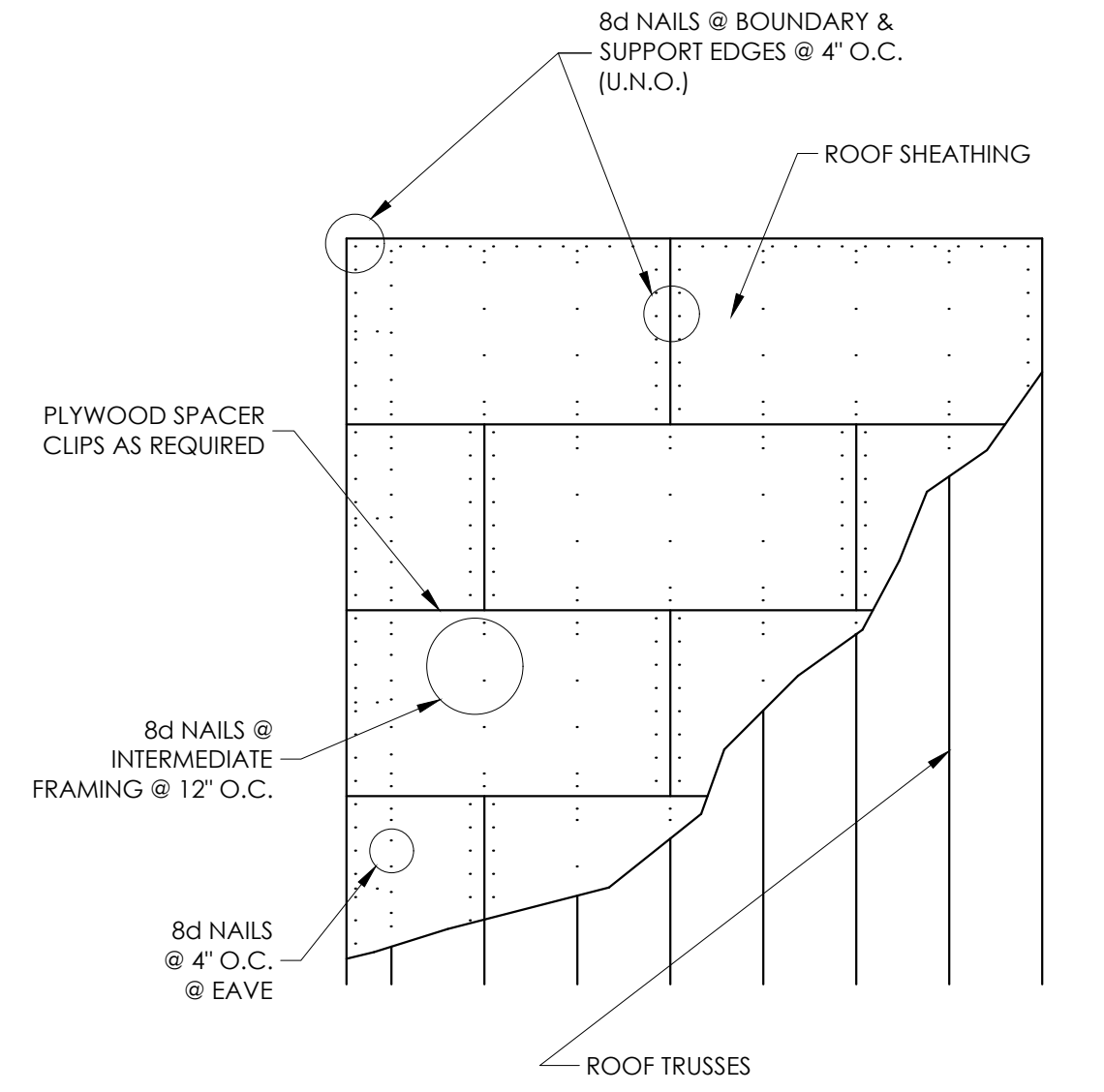
**5** TRUSS BEARING @ BEAM  
S-4 SCALE: NONE



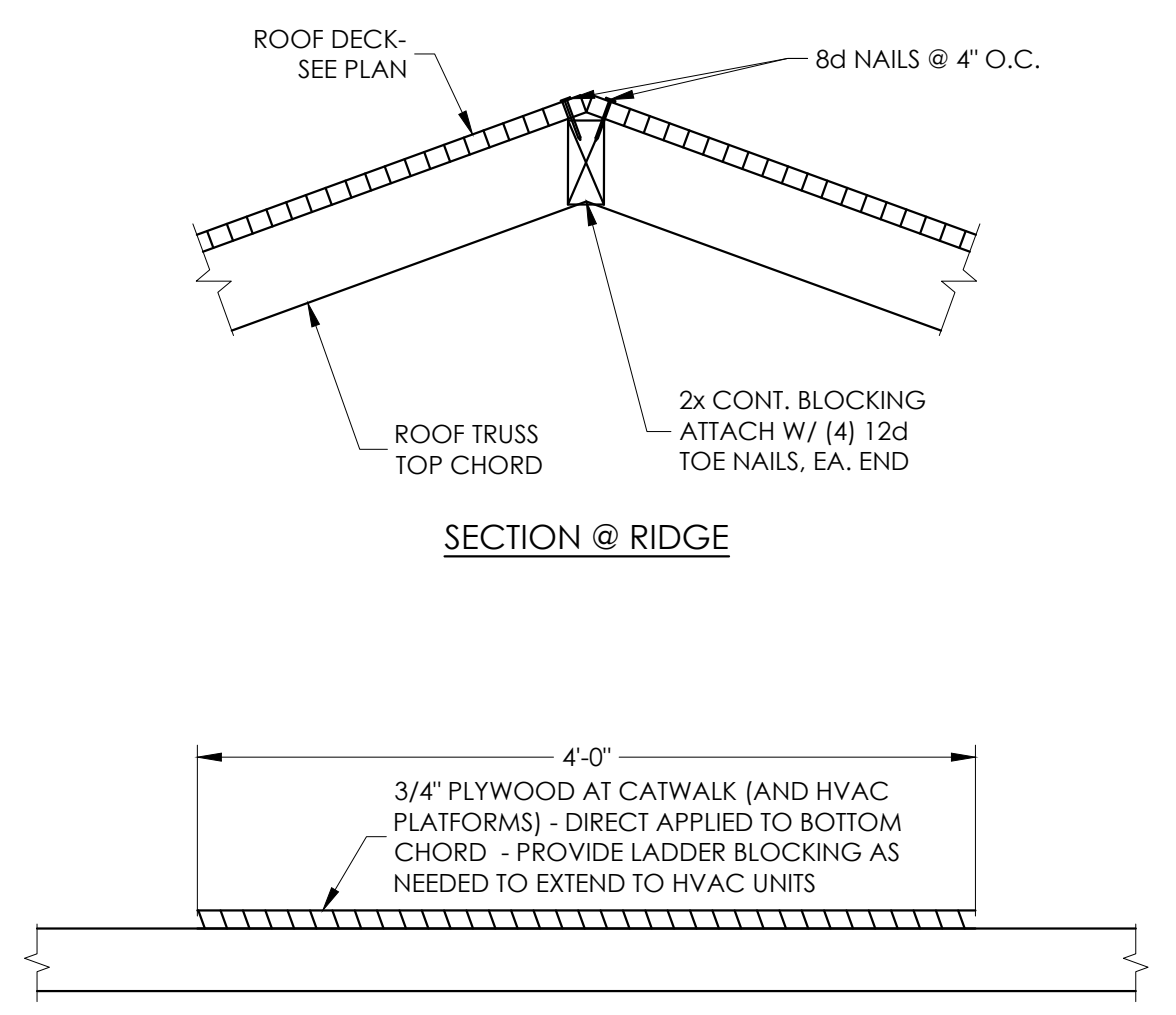
**6** SHEARWALL AT DRAG TRUSS  
S-4 SCALE: NONE



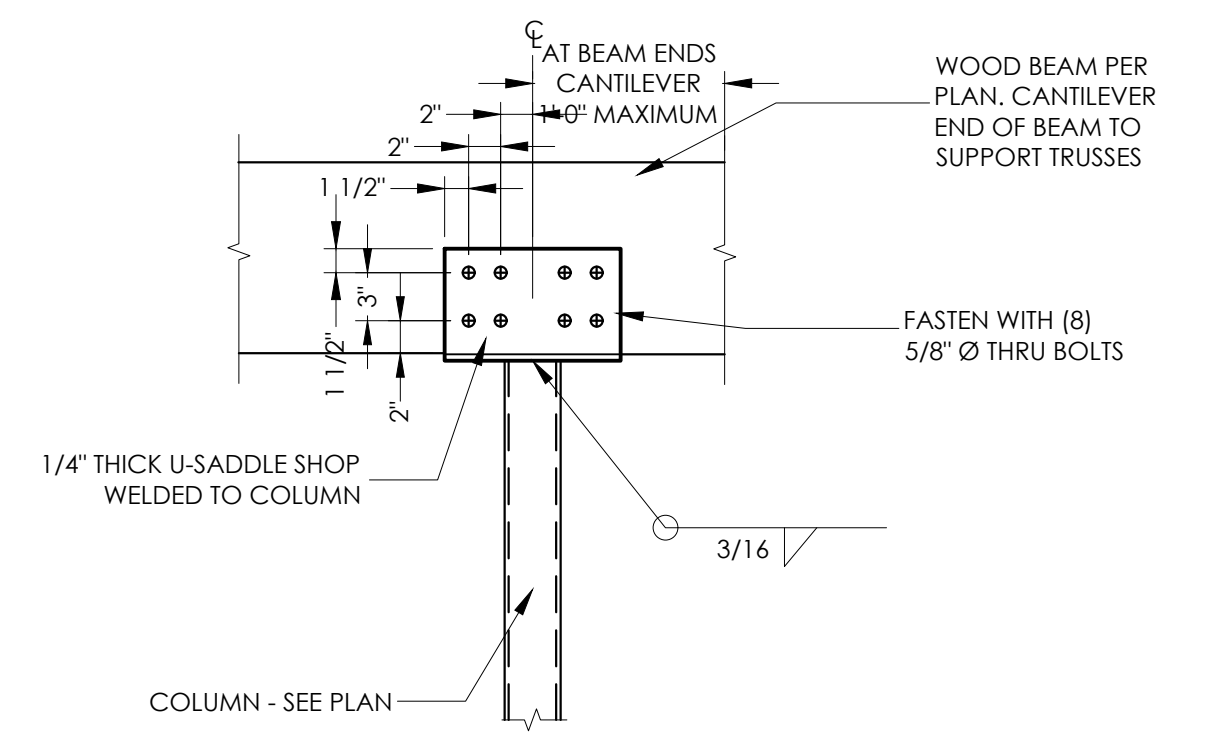
**7** ROOF DECK NAILING PATTERN  
S-4 SCALE: NONE



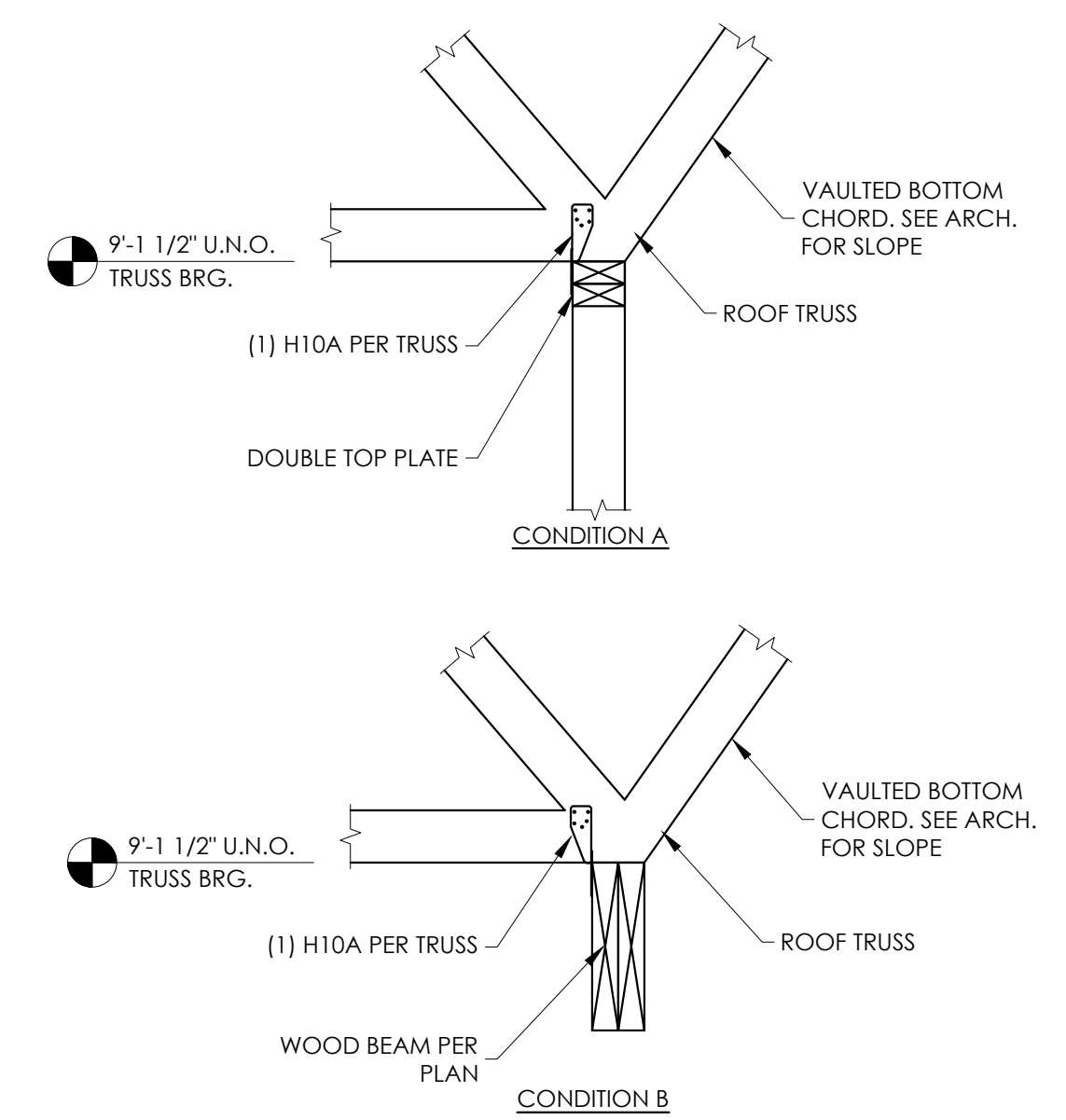
**8** ROOF DECK NAILING PATTERN  
S-4 SCALE: NONE



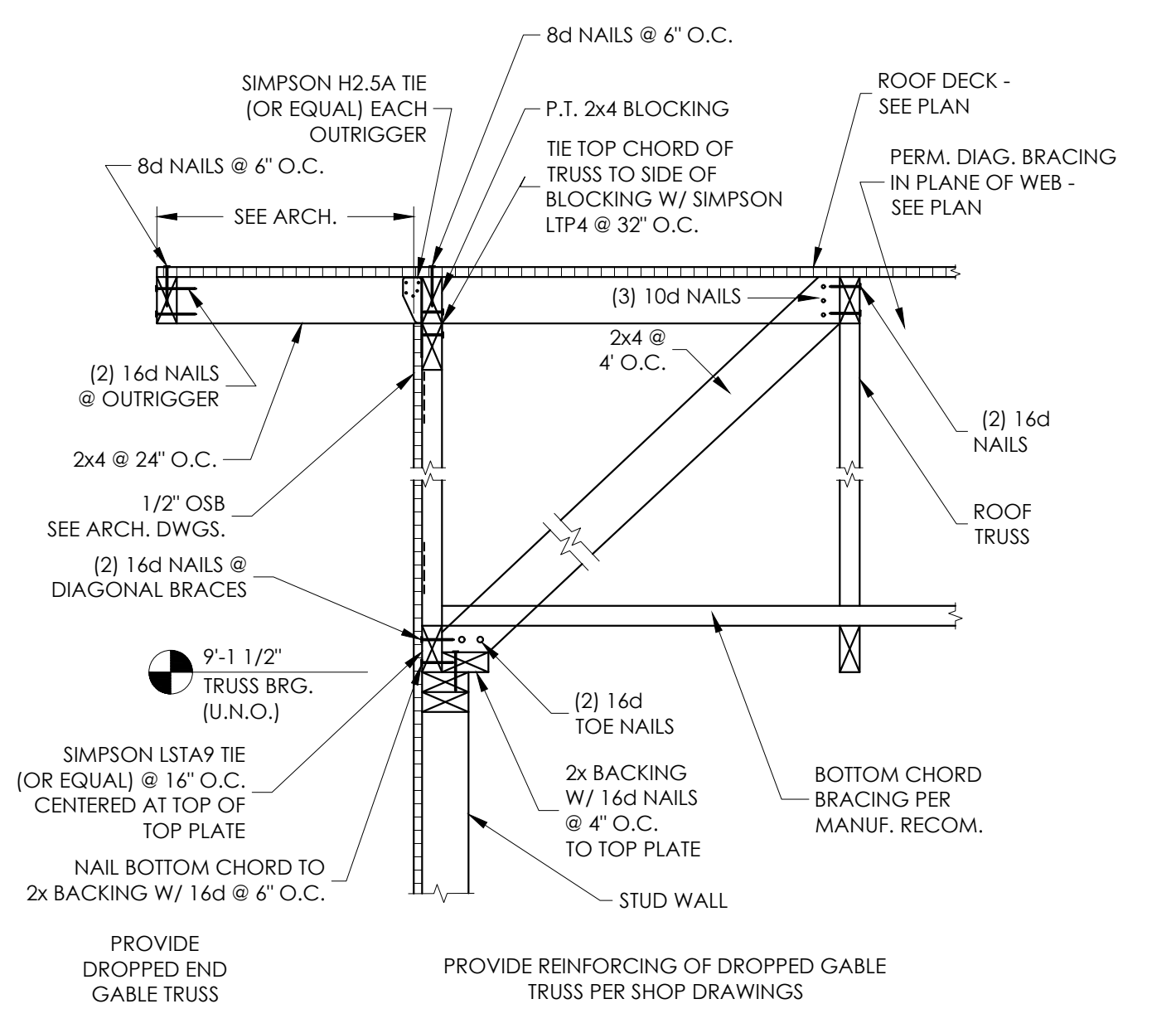
**9** SECTION AT RIDGE AND CATWALK  
S-4 SCALE: NONE



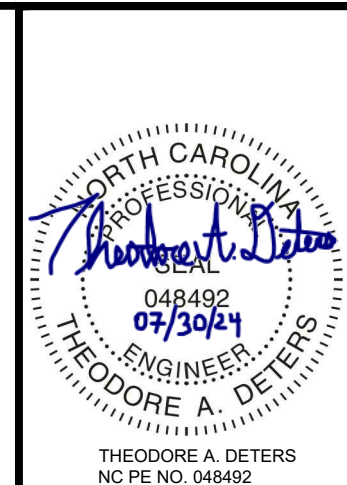
**10** WOOD BEAM ON STEEL COLUMN  
S-4 SCALE: NONE



**11** TRUSS @ INTERIOR BEAM BEARING  
S-4 SCALE: NONE



**12** RAKE SECTION @ EXT. WALL  
S-4 SCALE: NONE



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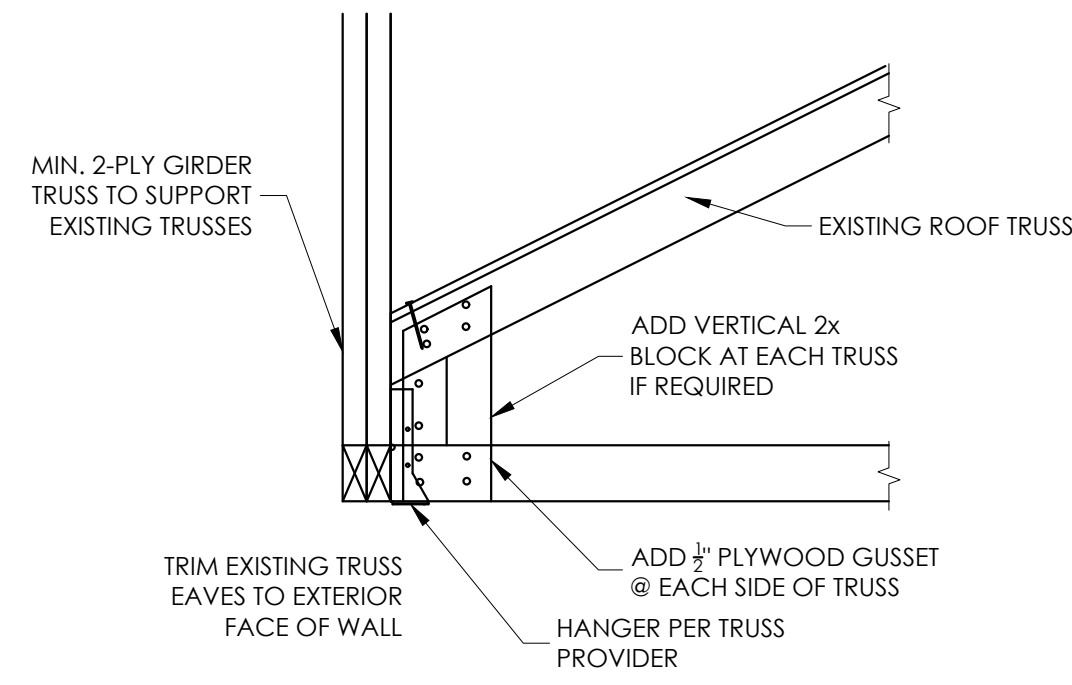
**David R. Polston - Architect**  
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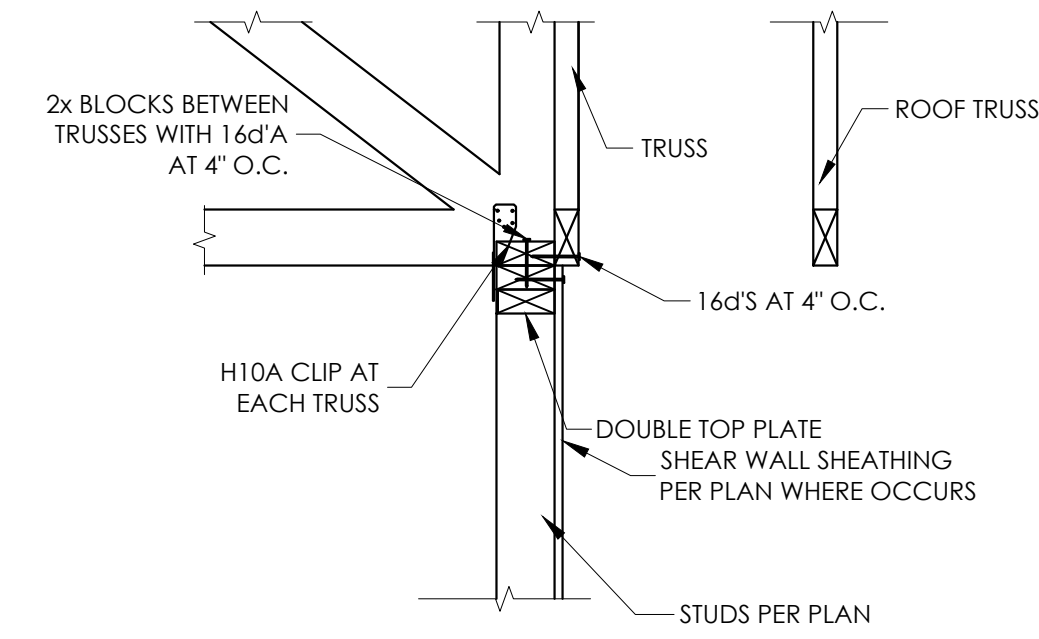
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**S4**

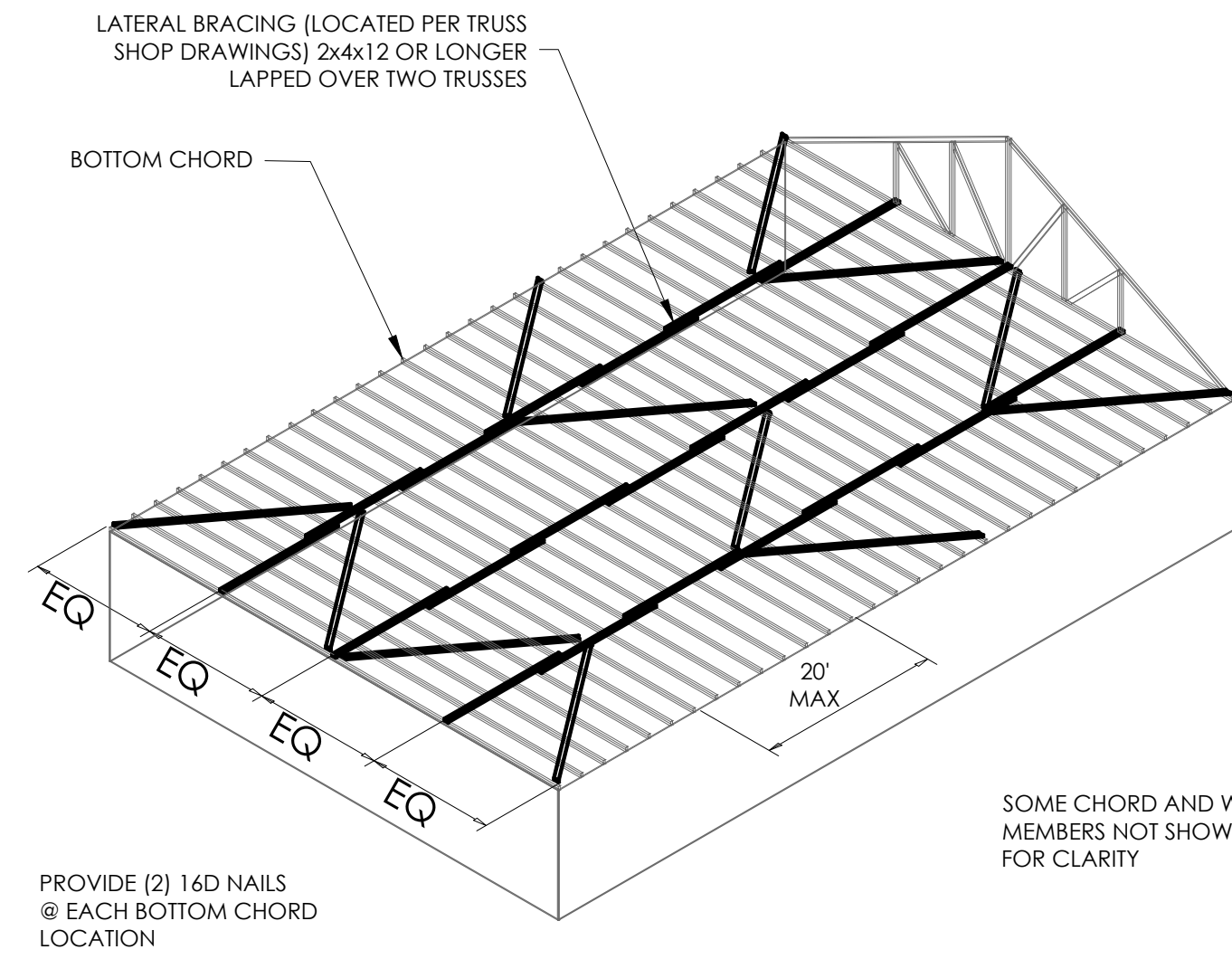




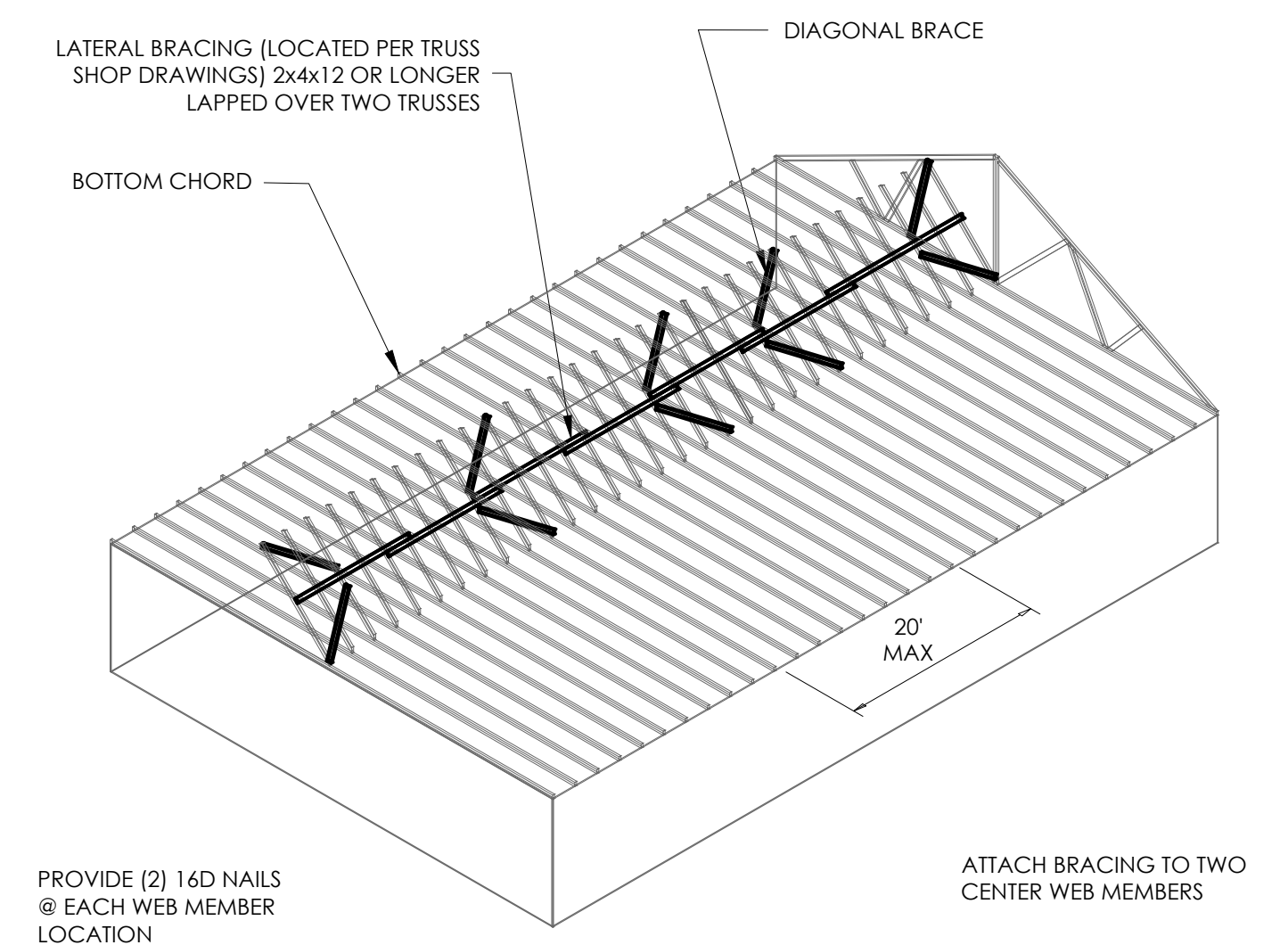
1 GIRDER TRUSS AT EXISTING  
S-5 SCALE: NONE



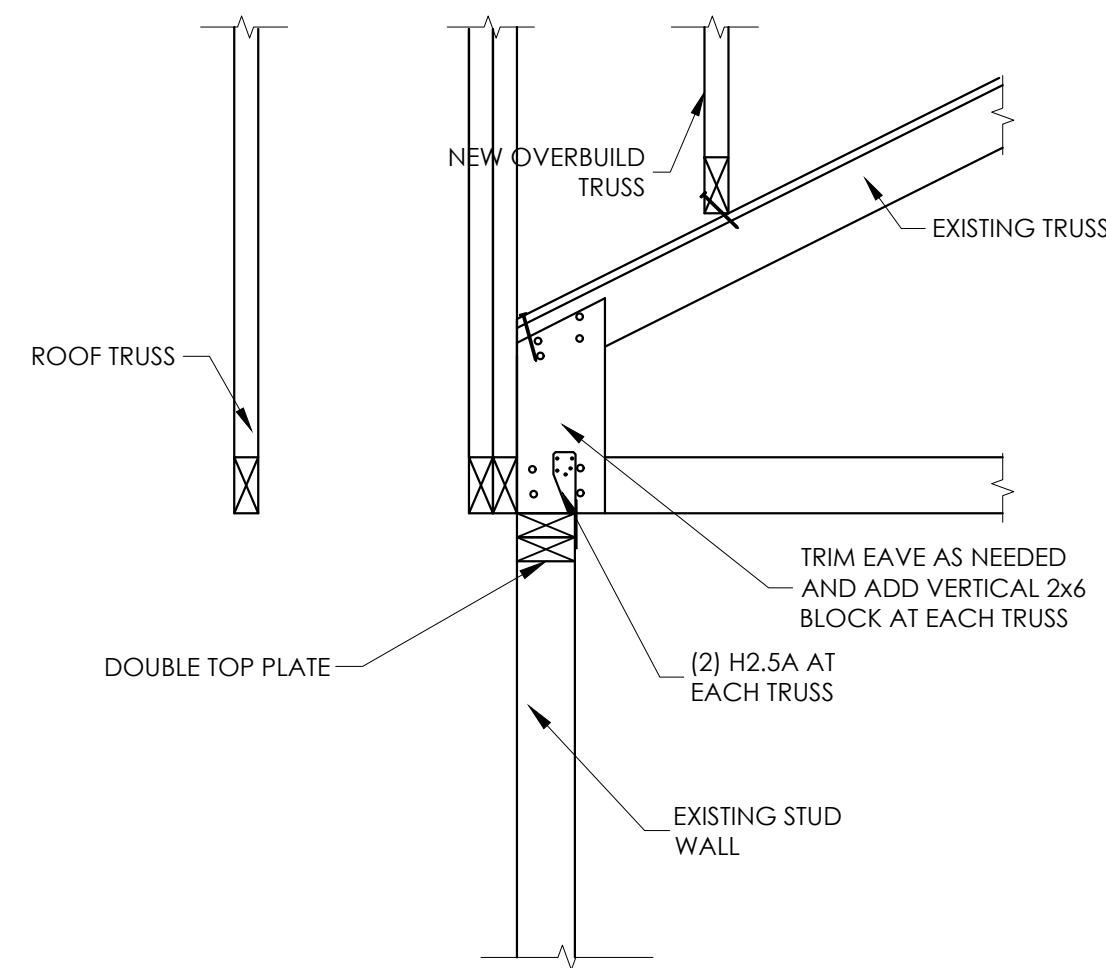
2 SECTION  
S-5 SCALE: NONE



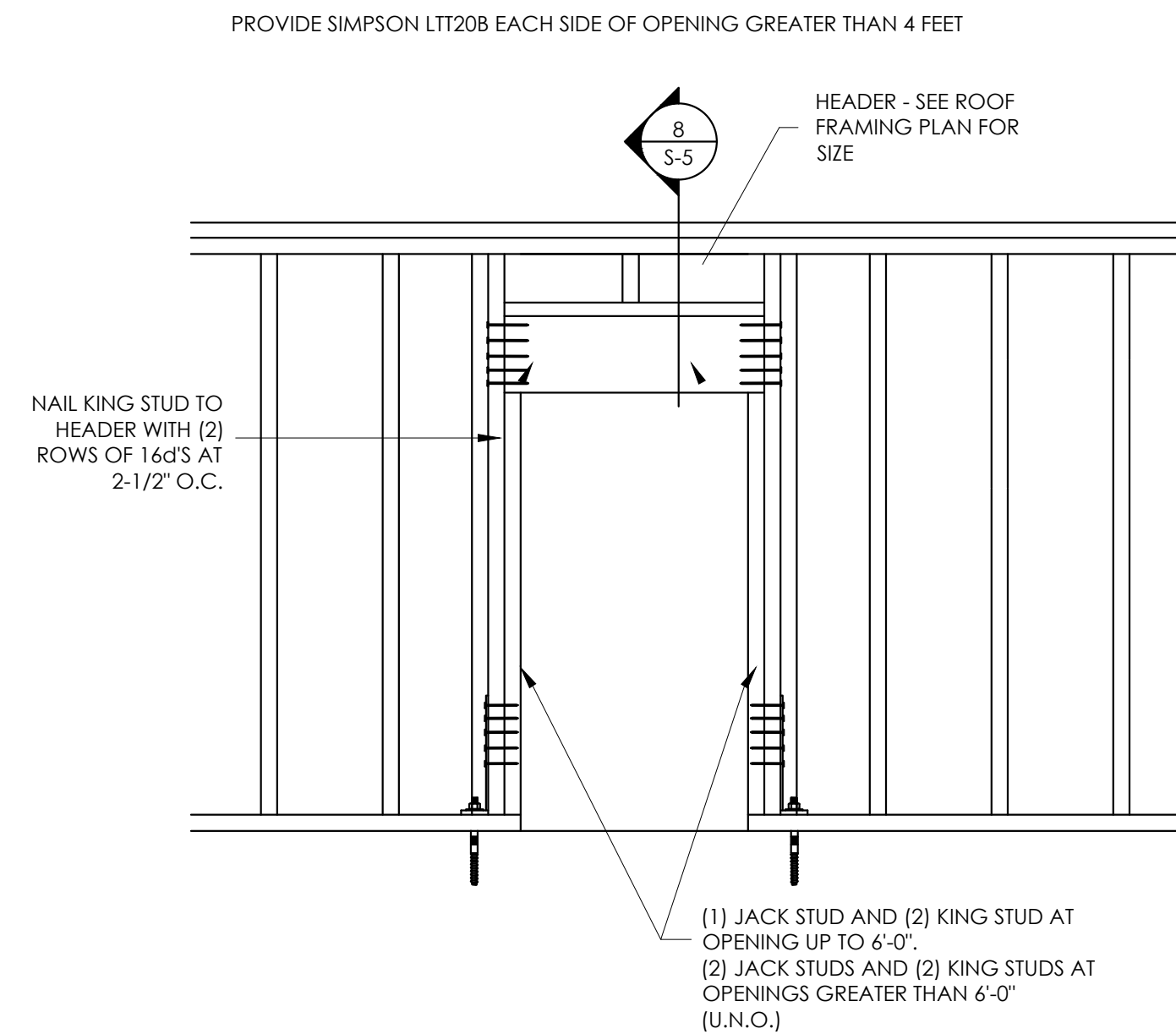
3 PERM. TRUSS BRACING DETAIL  
S-5 SCALE: NONE



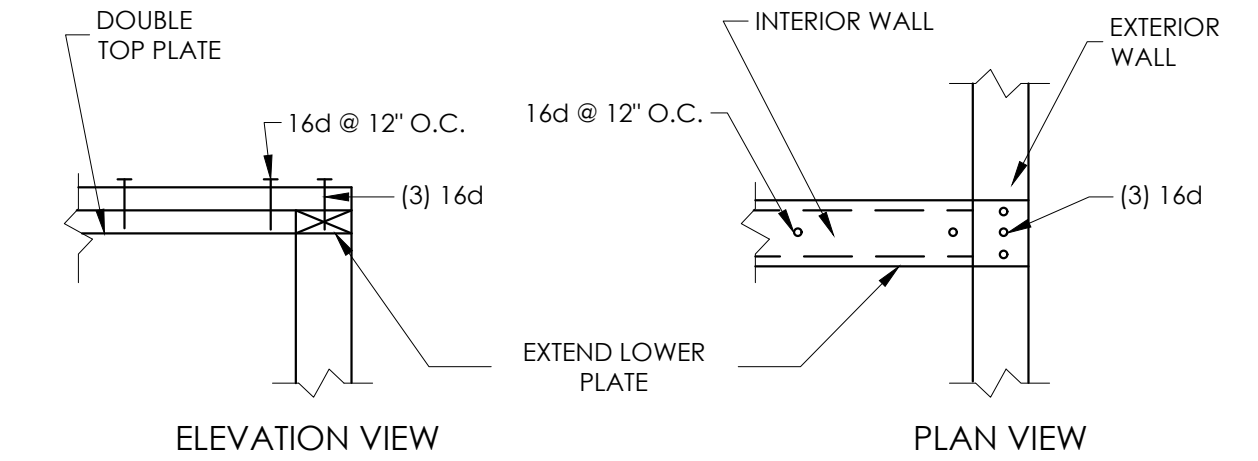
4 PERM. TRUSS BRACING DETAIL  
S-5 SCALE: NONE



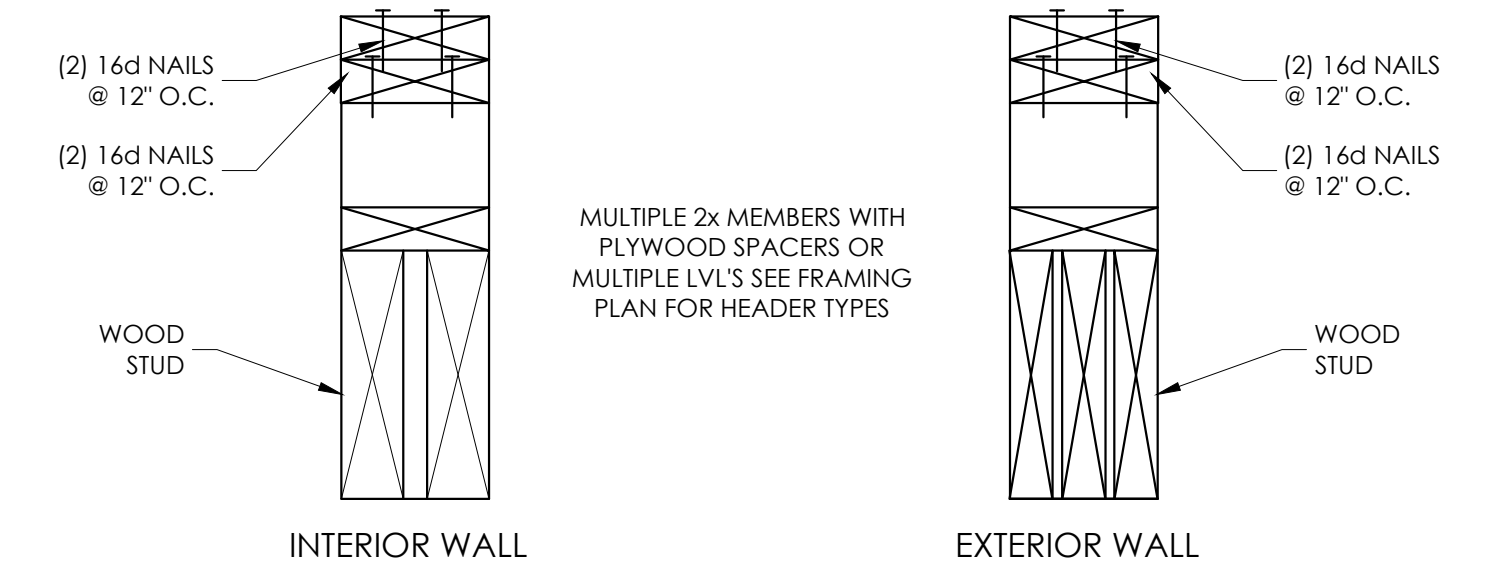
5 NEW TRUSS BEARING AT EXISTING WALL  
S-5 SCALE: NONE



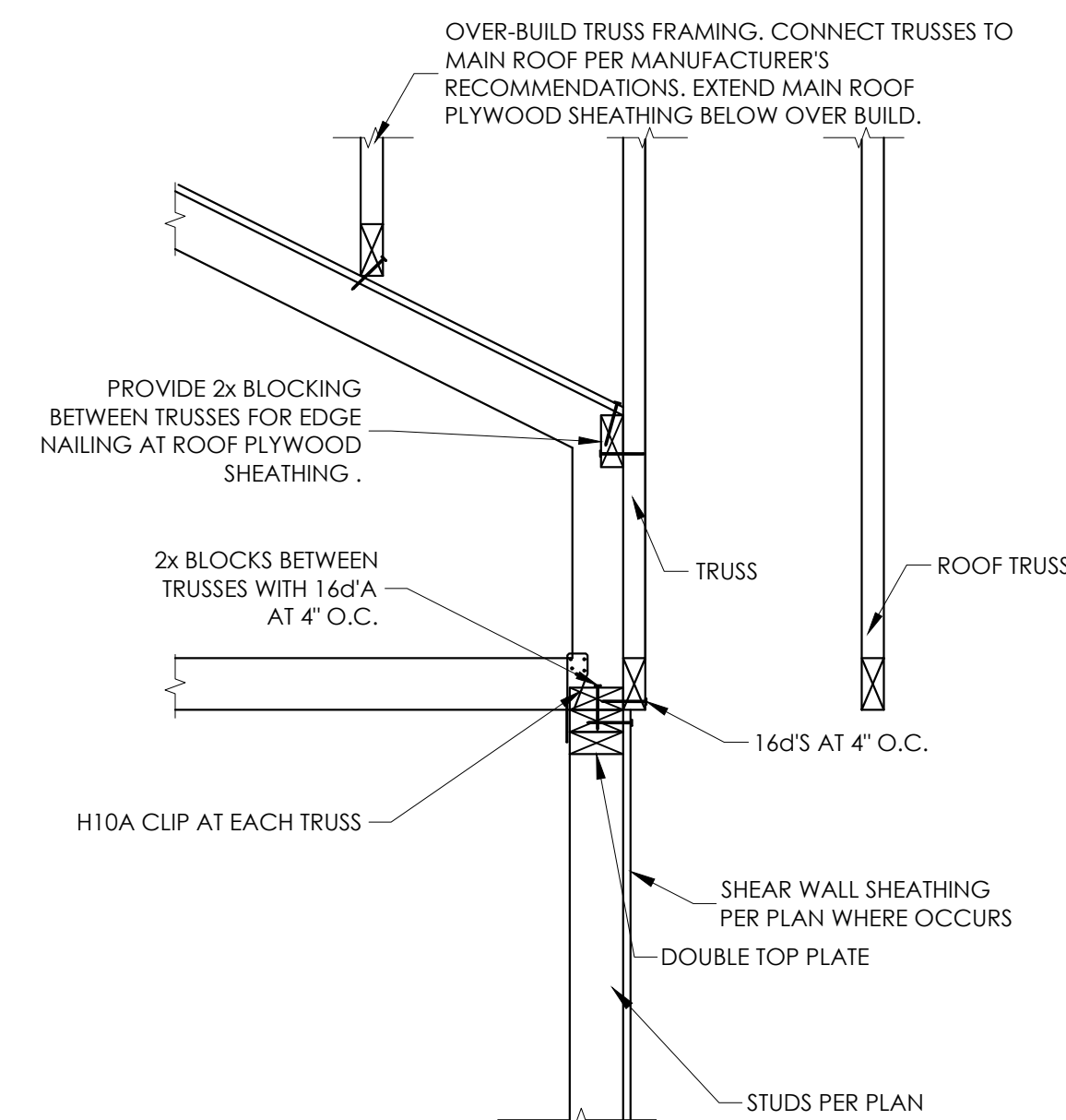
6 HEADER ELEVATION  
S-5 SCALE: NONE



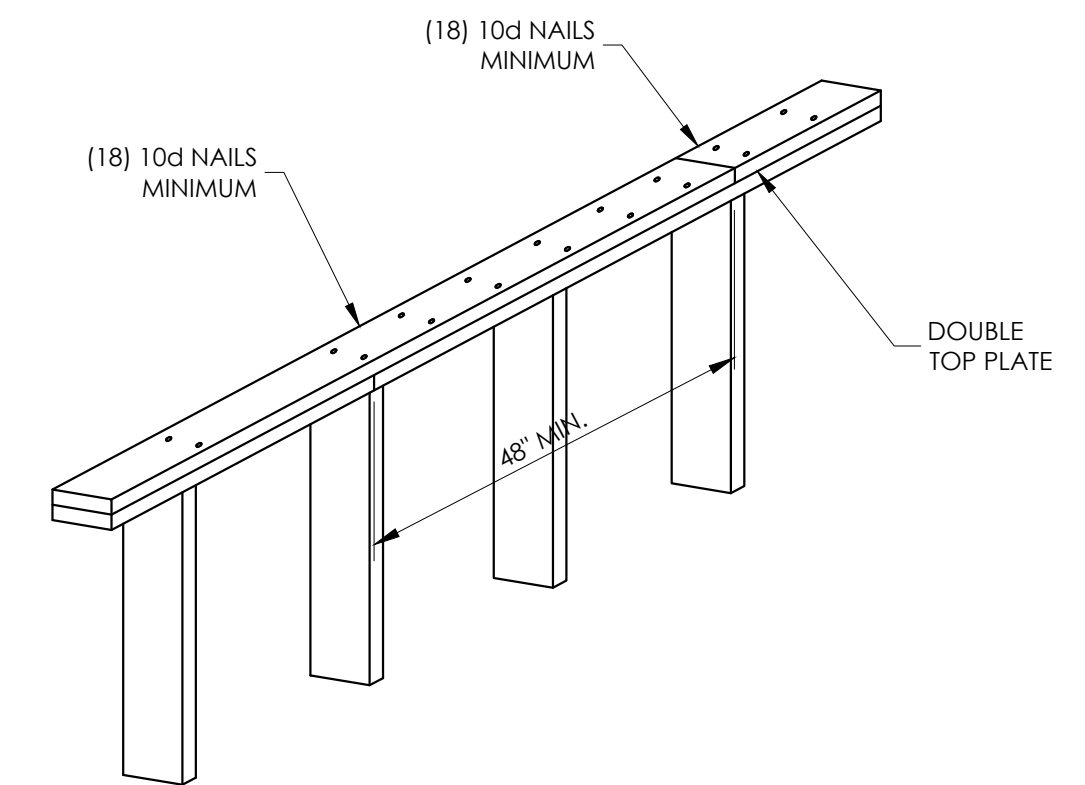
7 INTERSECTION OF INTERIOR AND EXTERIOR WALLS  
ATTACH SHEATHING AND WALLBOARD TO STUDS @ 7\"/>



8 HEADER & TOP PLATE DETAIL  
S-5 SCALE: NONE



9 INTERIOR TRUSS BEARING  
S-5 SCALE: NONE



10 TOP PLATE SPLICE DETAIL  
S-5 SCALE: NONE



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**24 BED ADDITION**

REV	DATE

**S5**

5 SCALE: NONE

9 SCALE: NONE

10 SCALE: NONE



NOTES:

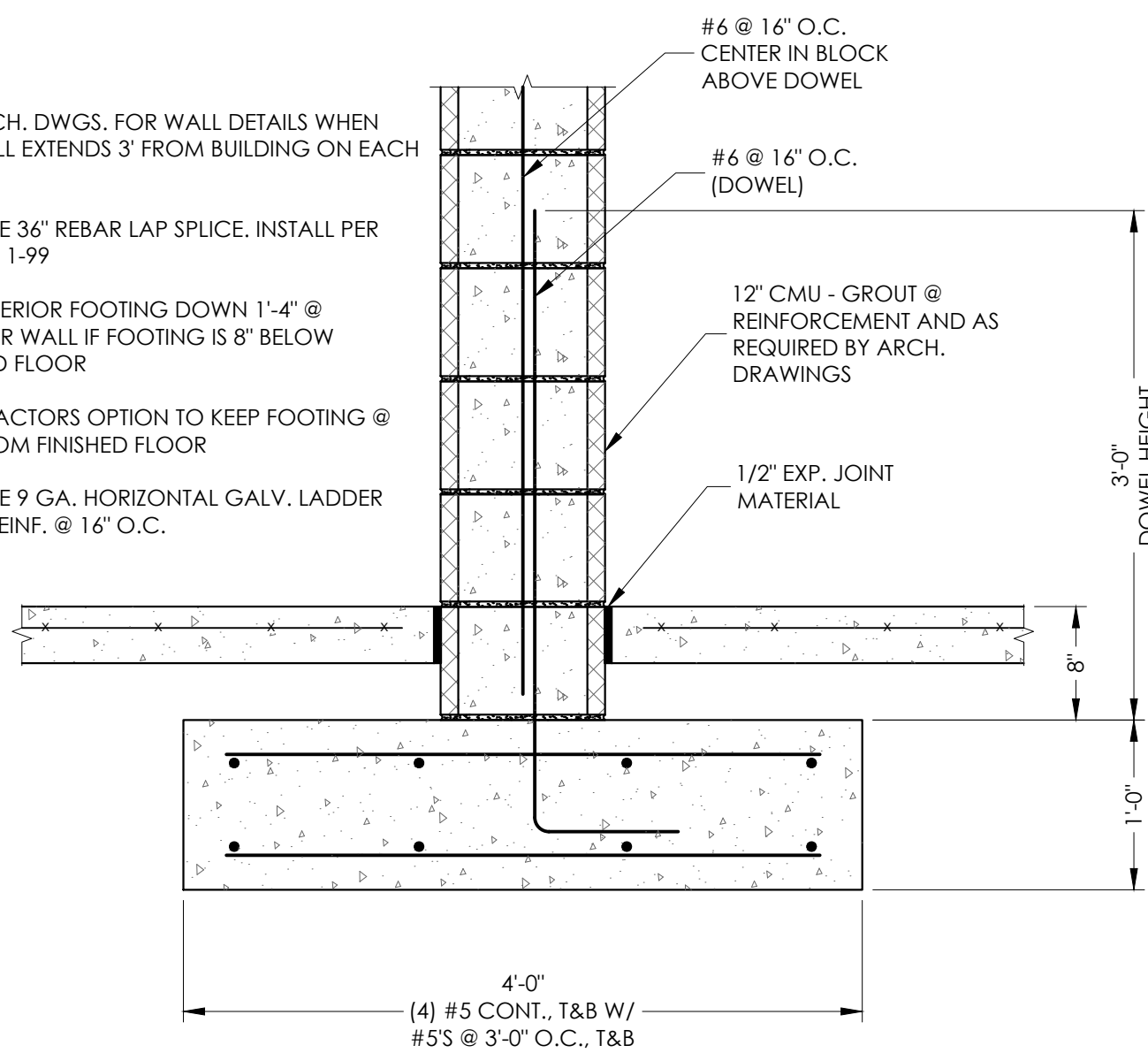
SEE ARCH. DWGS. FOR WALL DETAILS WHEN FIREWALL EXTENDS 3' FROM BUILDING ON EACH SIDE.

PROVIDE 36" REBAR LAP SPLICE. INSTALL PER ACI 530 1-99

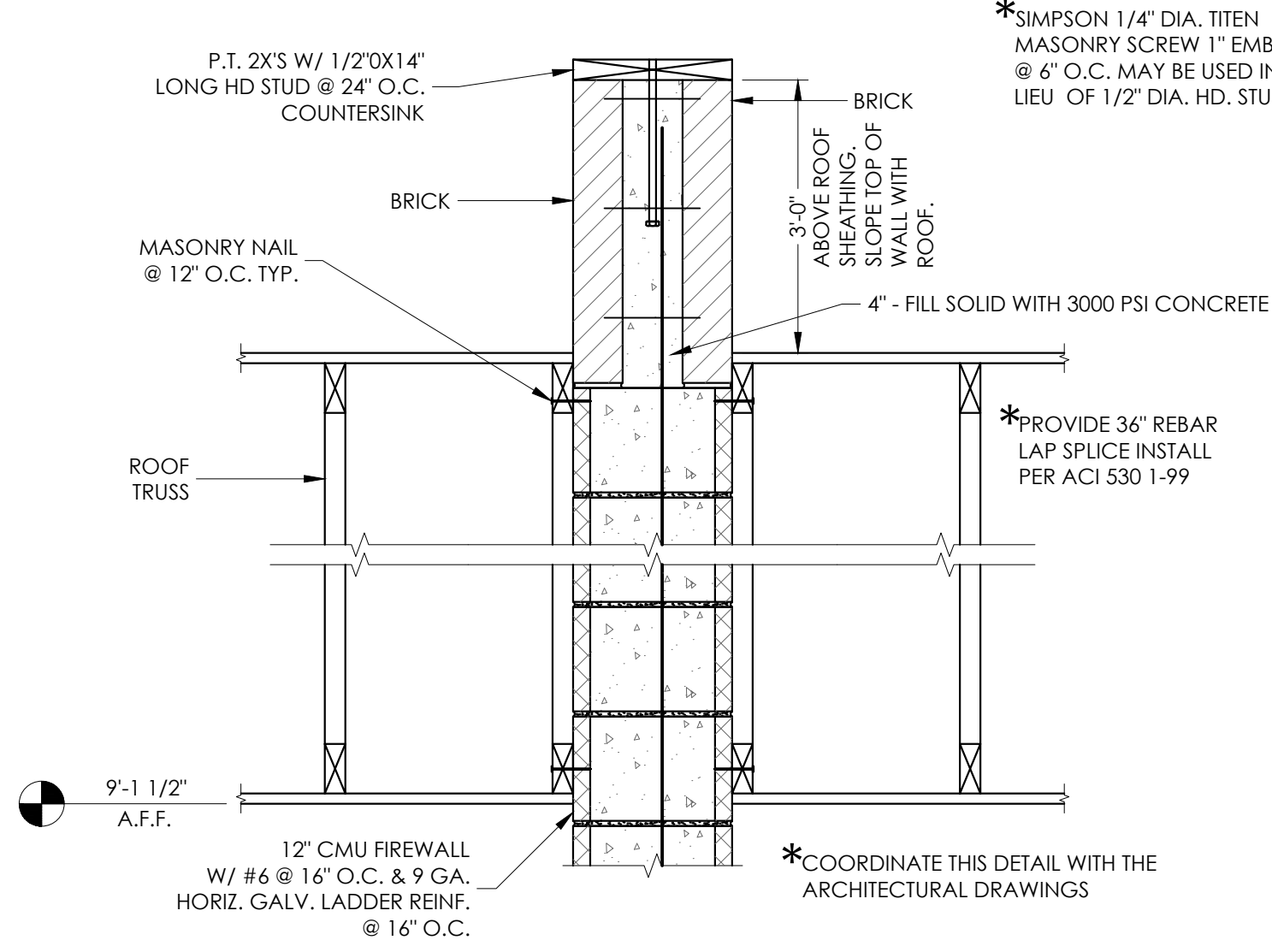
STEP INTERIOR FOOTING DOWN 1'-4" @ EXTERIOR WALL IF FOOTING IS 8" BELOW FINISHED FLOOR

CONTRACTORS OPTION TO KEEP FOOTING @ 1'-4" FROM FINISHED FLOOR

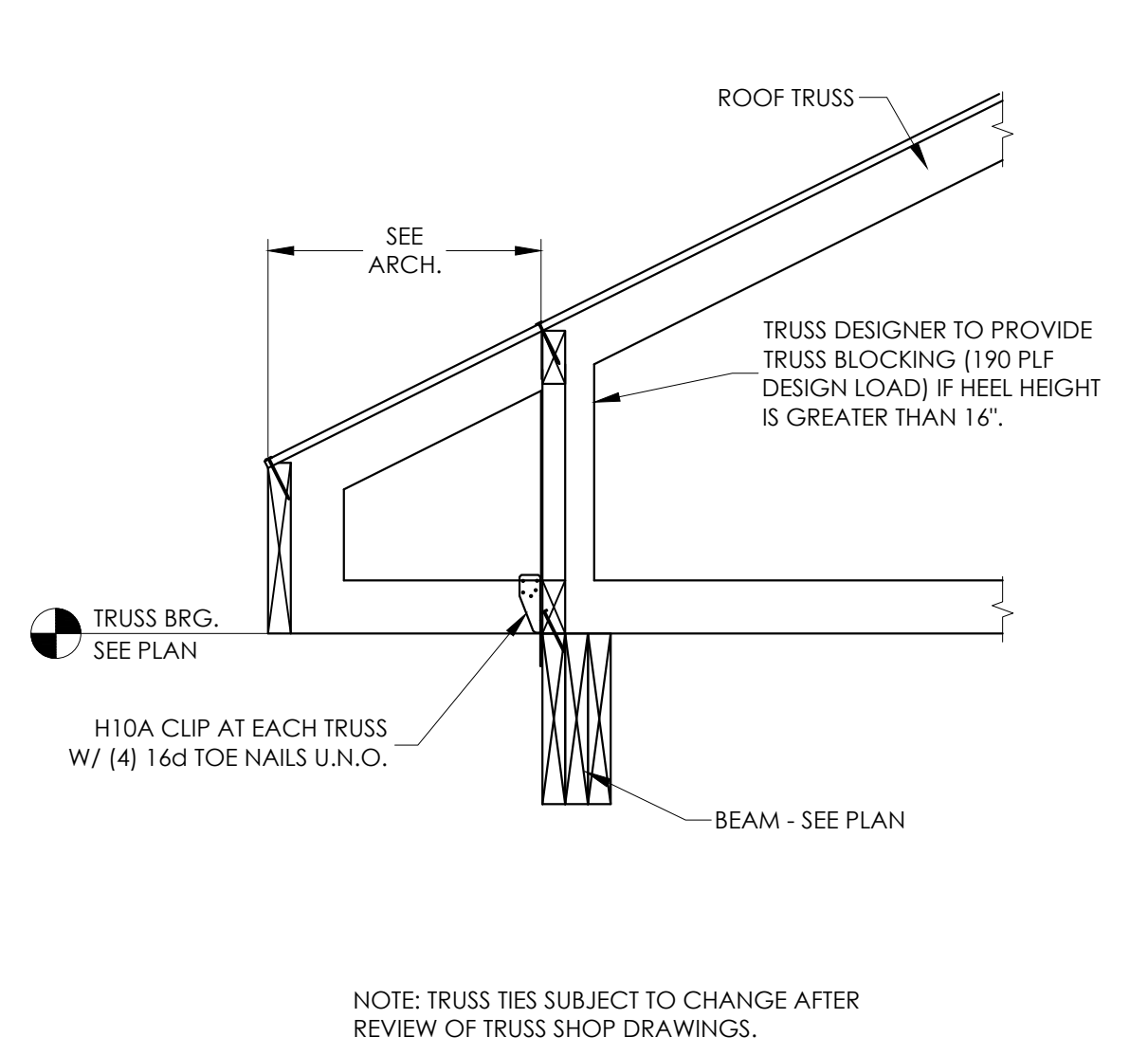
PROVIDE 9 GA. HORIZONTAL GALV. LADDER JOINT REINF. @ 16" O.C.



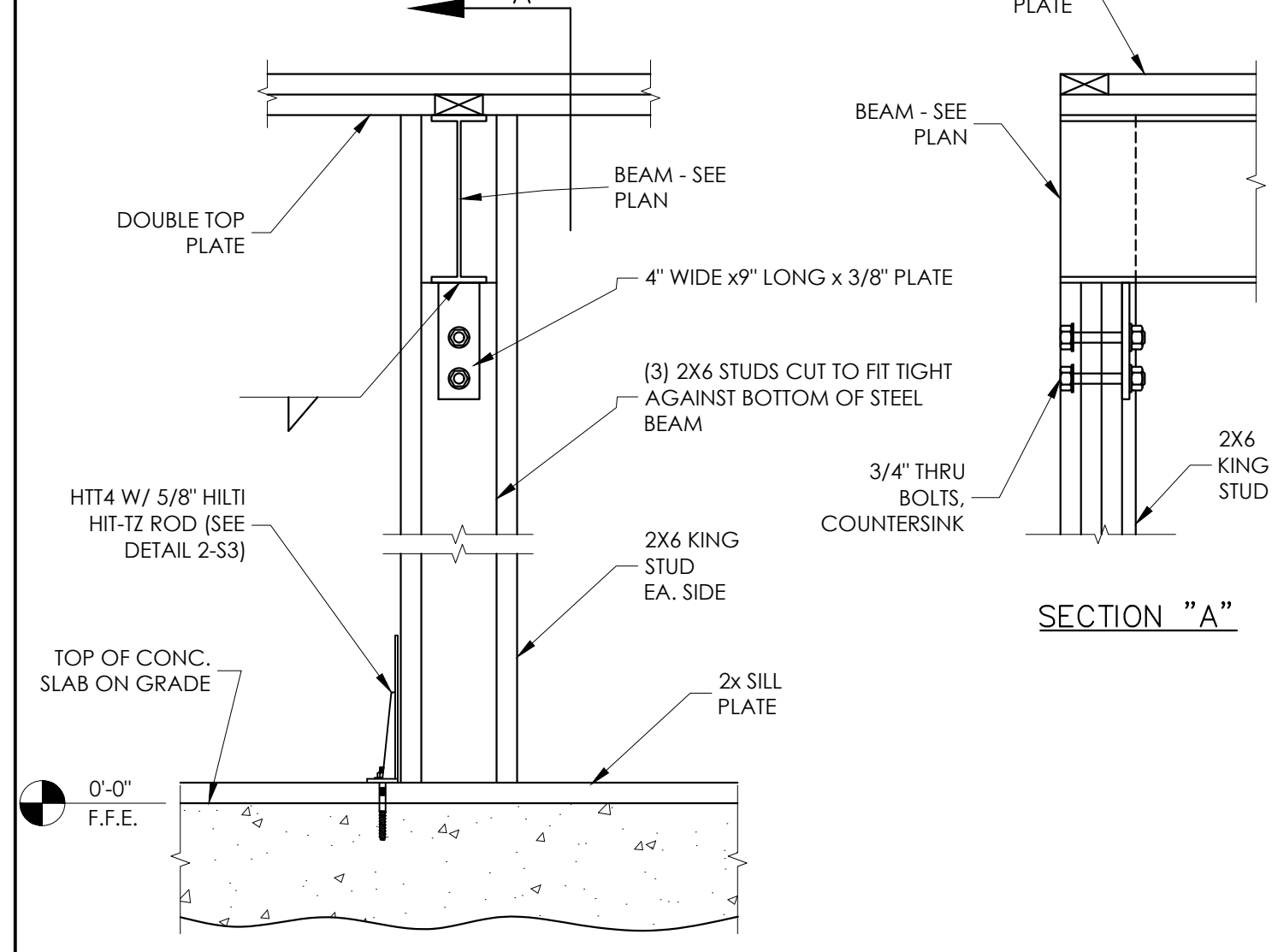
1 SECTION @ FIREWALL FOUNDATION  
S-6 SCALE: NONE



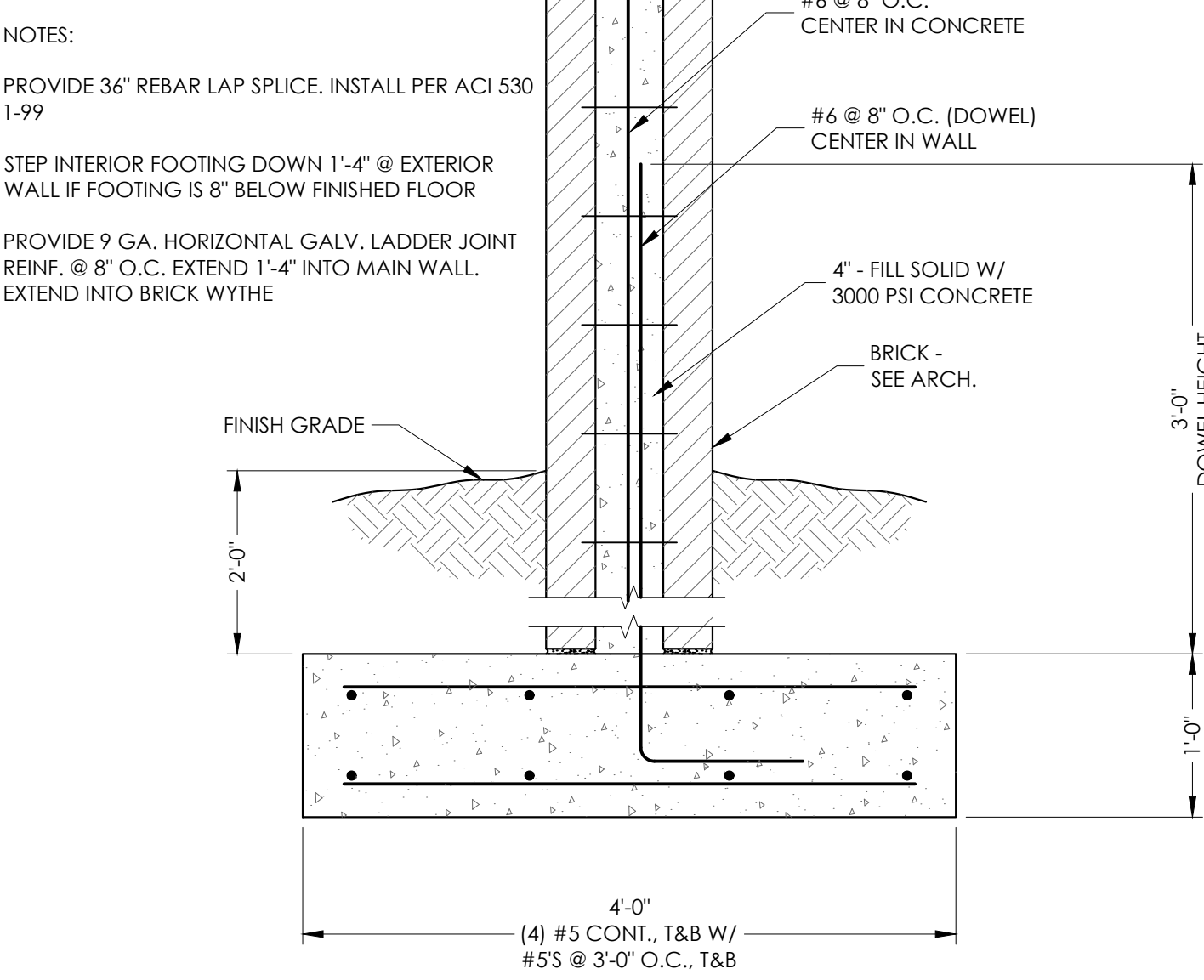
2 SECTION @ TOP OF FIREWALL  
S-6 SCALE: NONE



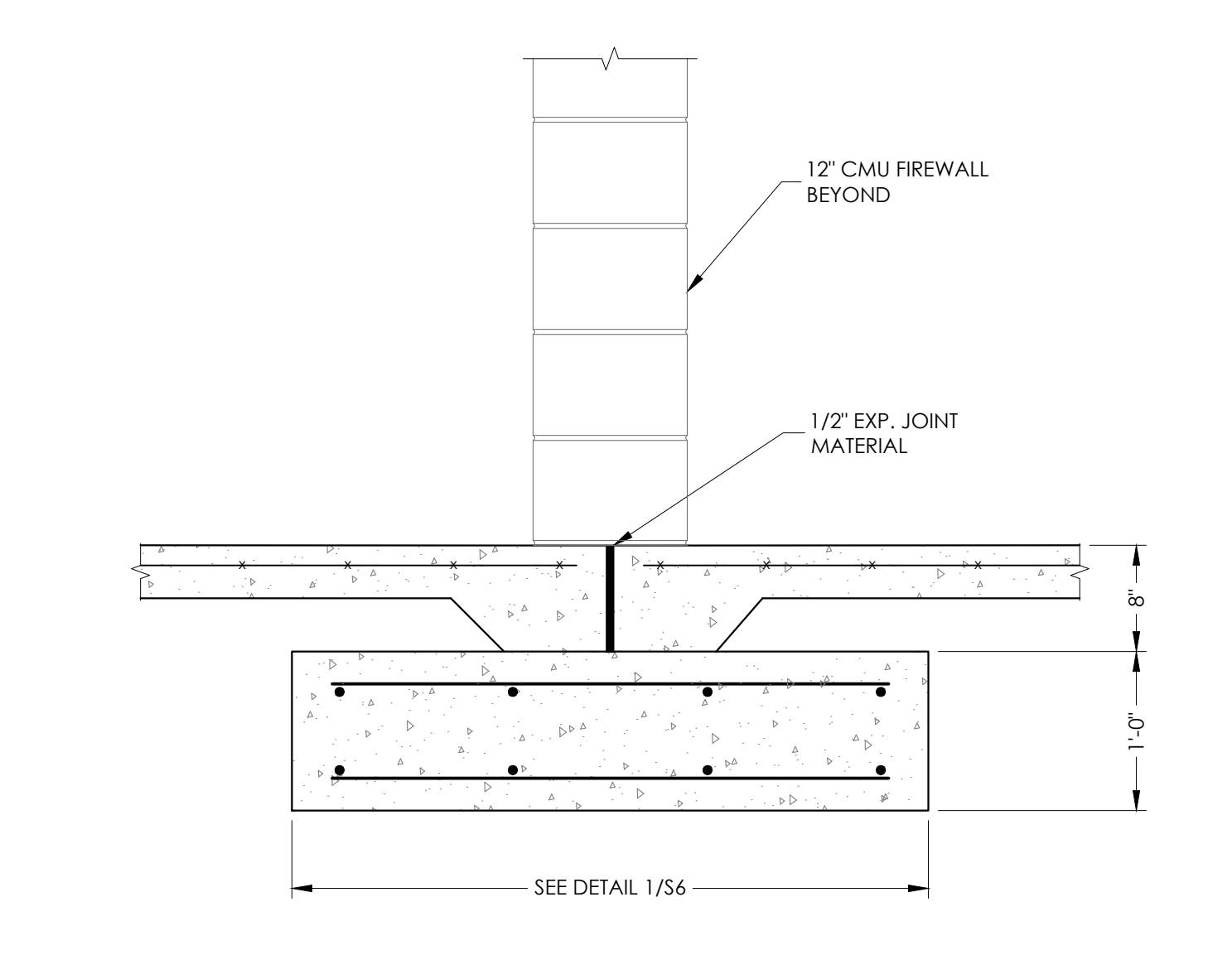
3 TRUSS BEARING @ BEAM  
S-6 SCALE: NONE



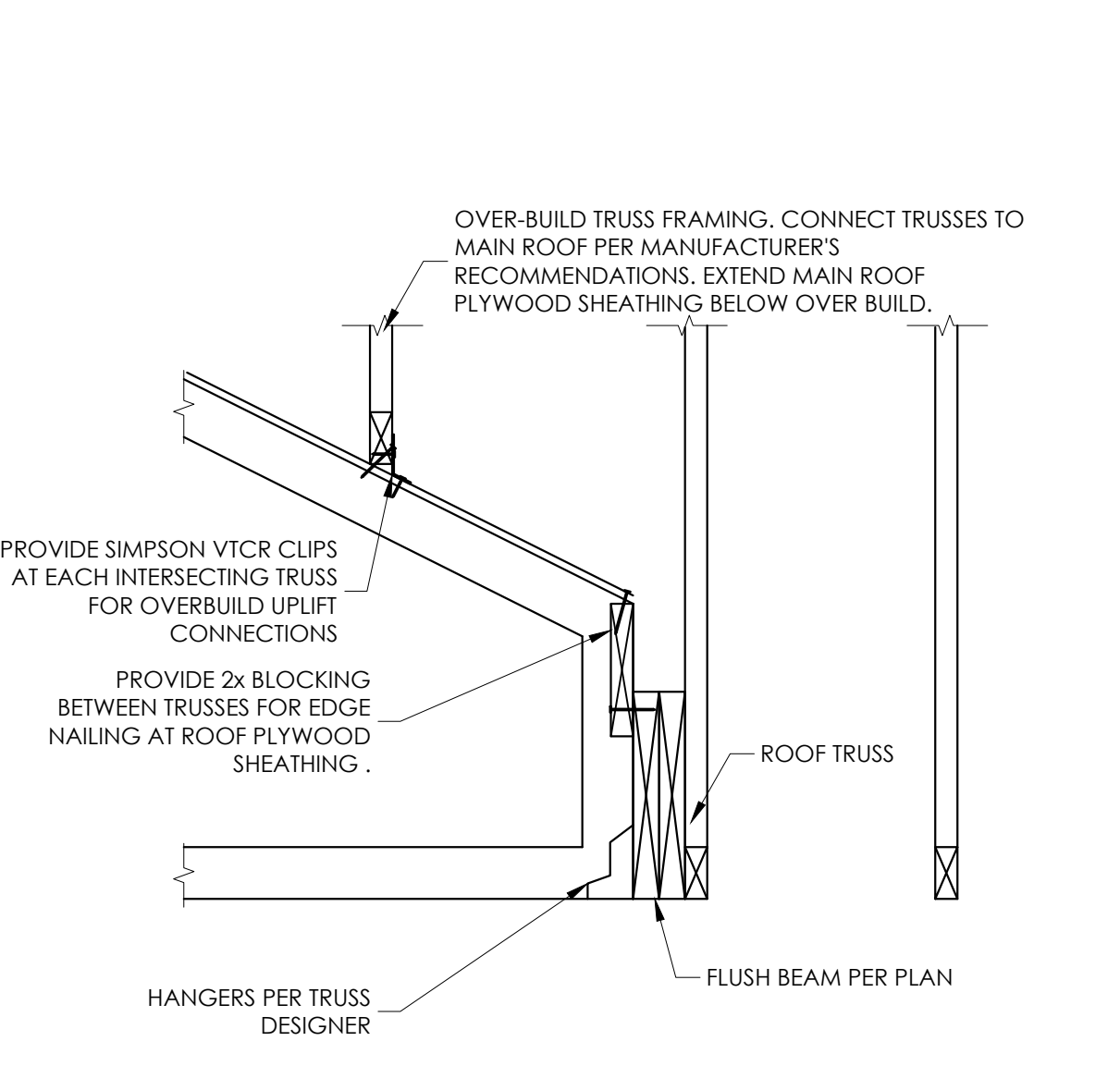
4 STEEL BEAM BEARING POCKET @ WALL  
S-6 SCALE: NONE



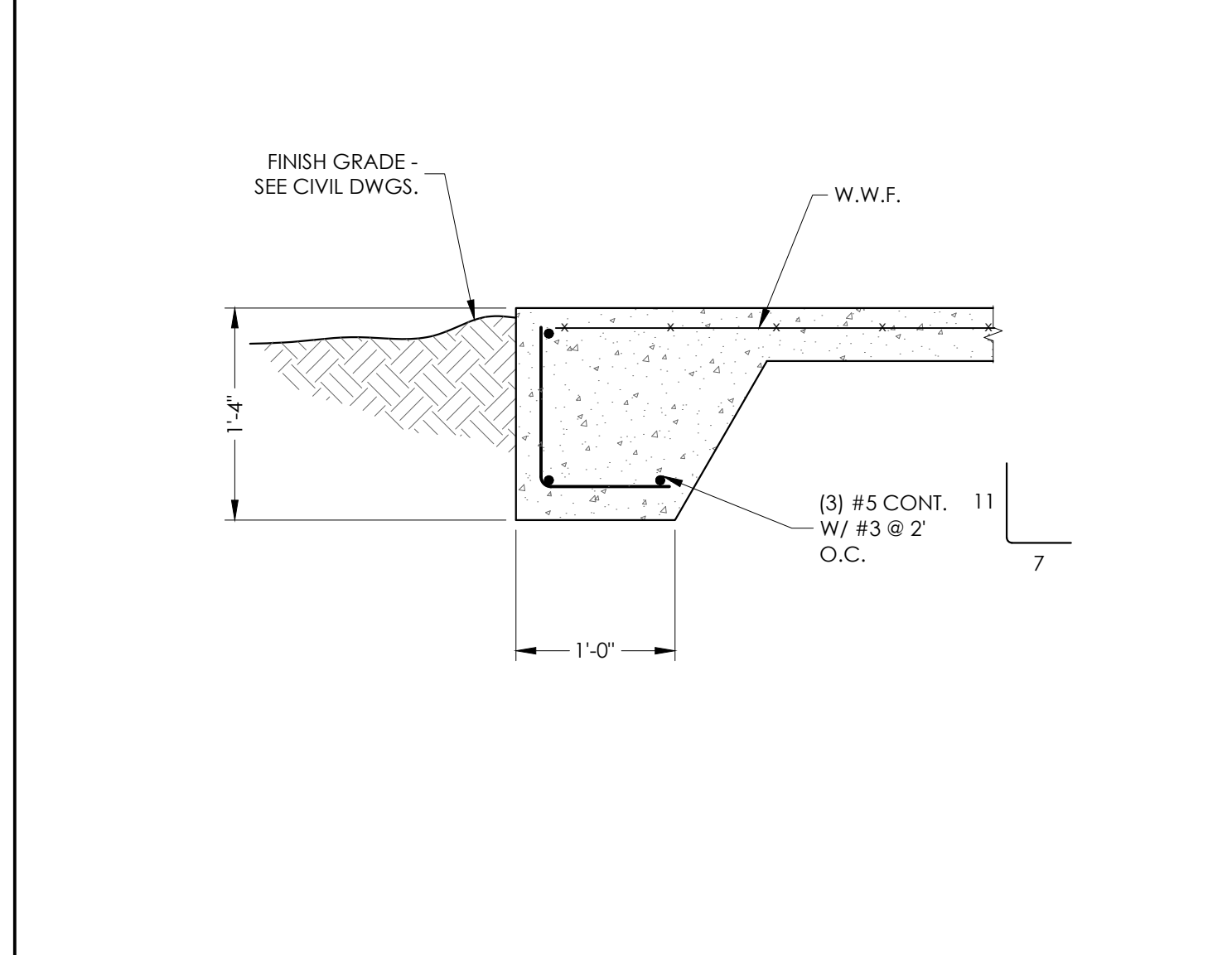
5 SECTION @ FIREWALL FOUNDATION  
S-6 SCALE: NONE



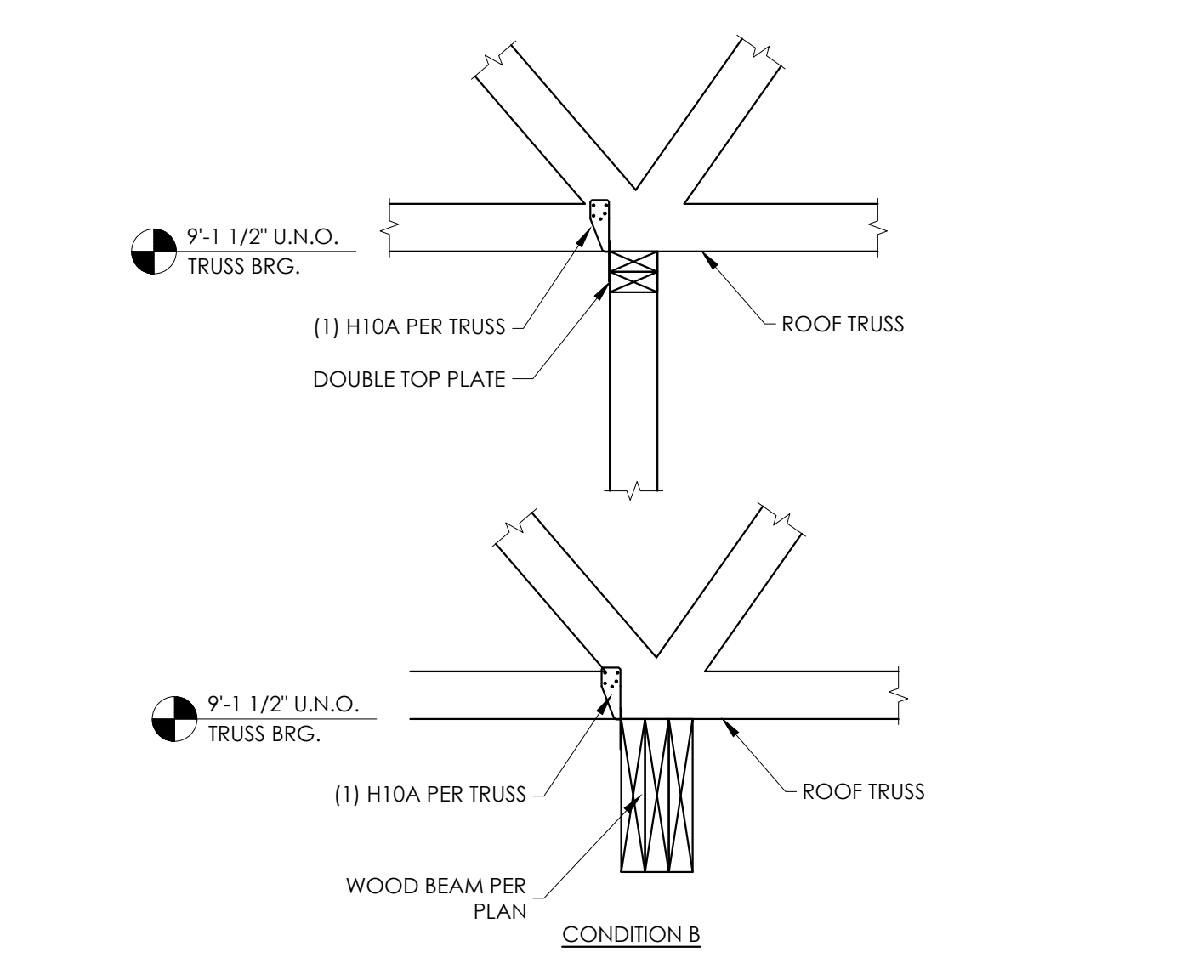
6 SECTION @ FIREWALL OPENING  
S-6 SCALE: NONE



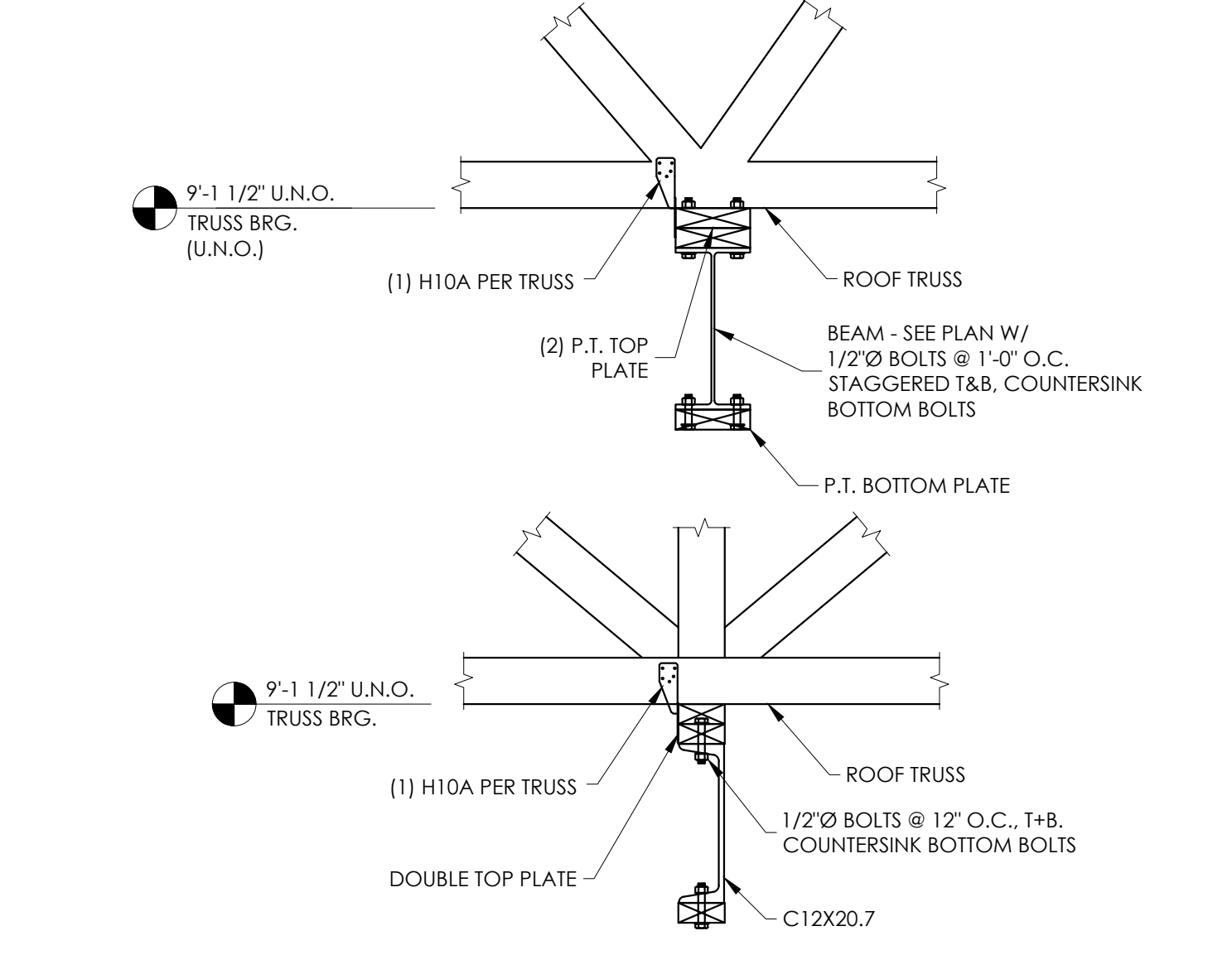
7 TRUSS AT FLUSH BEAM  
S-6 SCALE: NONE



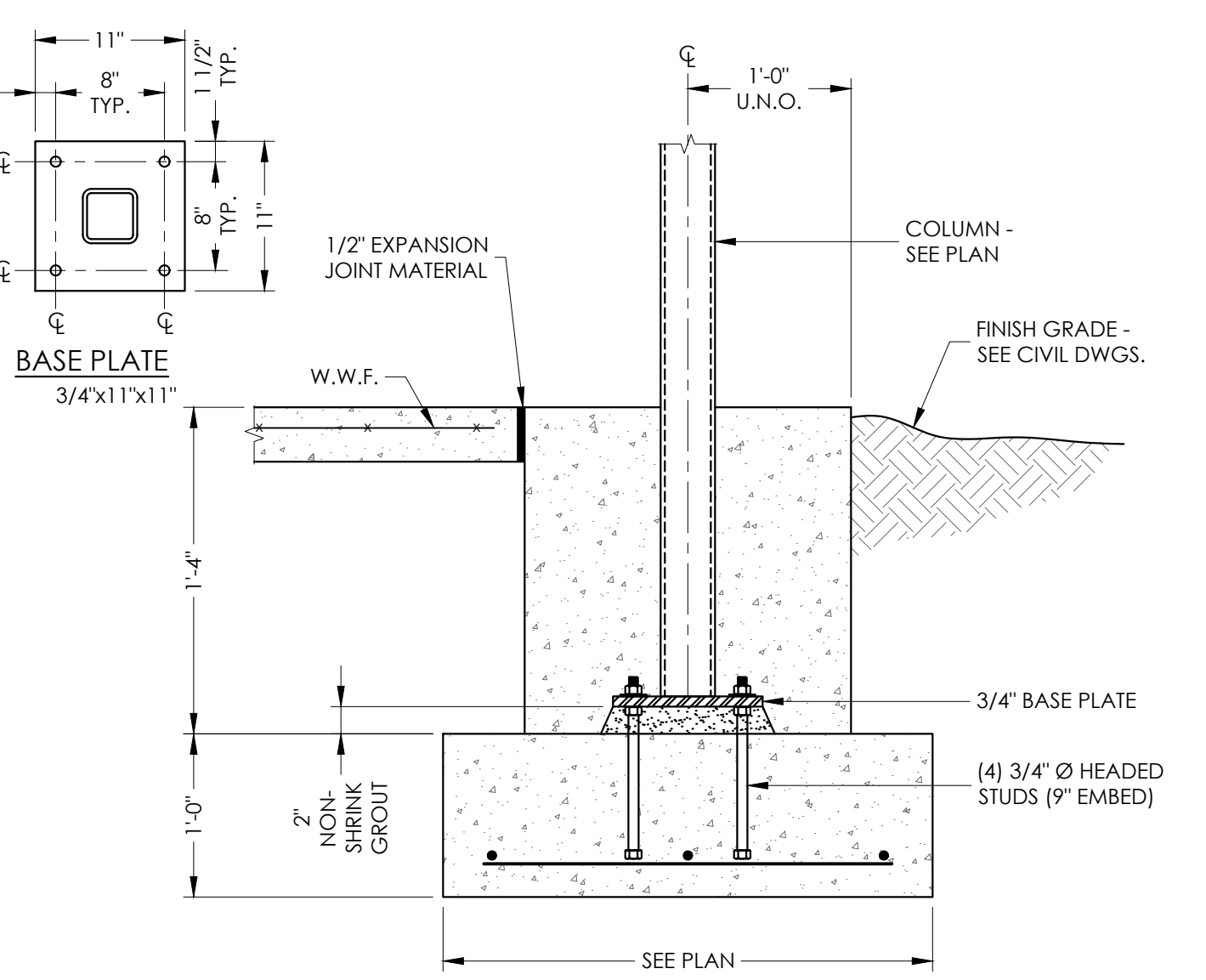
8 SECTION @ PORCH SLAB  
S-6 SCALE: NONE



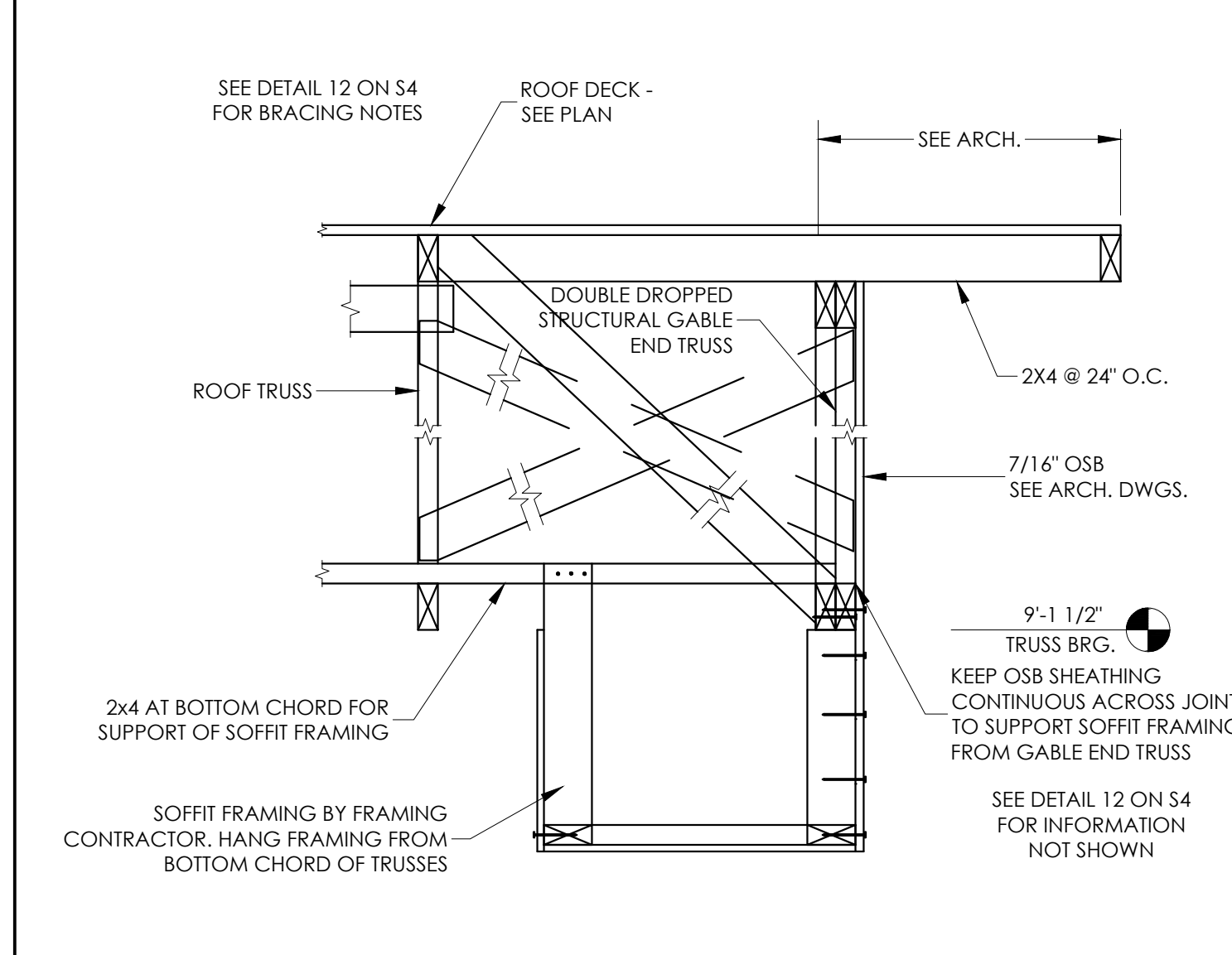
9 TRUSS AT INTERIOR BEARING  
S-6 SCALE: NONE



10 STEEL BEAM BEARING  
S-6 SCALE: NONE



11 SECTION @ EXTERIOR COLUMN  
S-6 SCALE: NONE



12 SECTION @ GABLE END AT WALL  
S-6 SCALE: NONE

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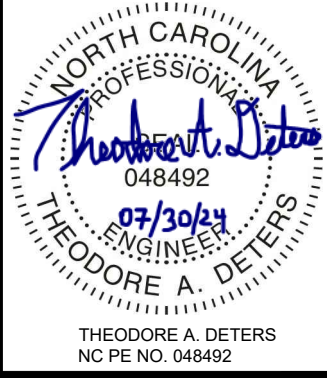
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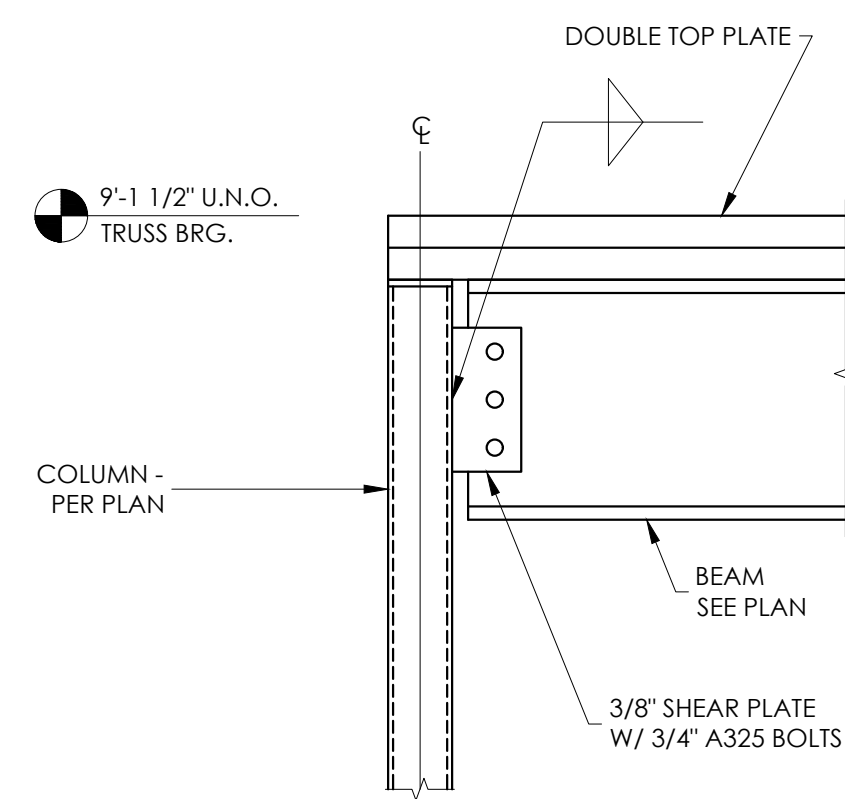
REV	DATE

S7

2 S-7 SCALE: NONE

3 S-7 SCALE: NONE

4 S-7 SCALE: NONE

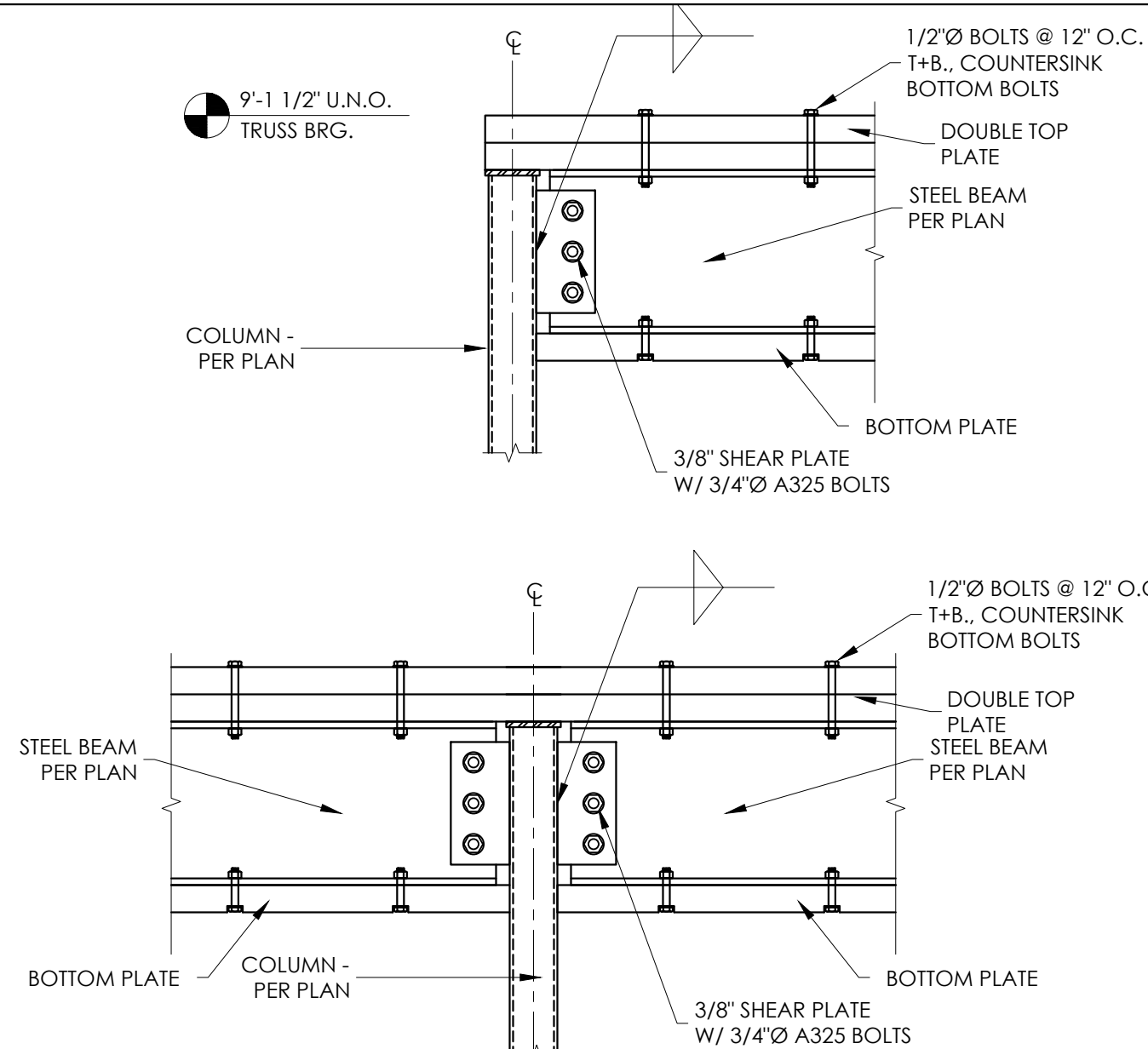


5 S-7 SCALE: NONE

6 S-7 SCALE: NONE

7 S-7 SCALE: NONE

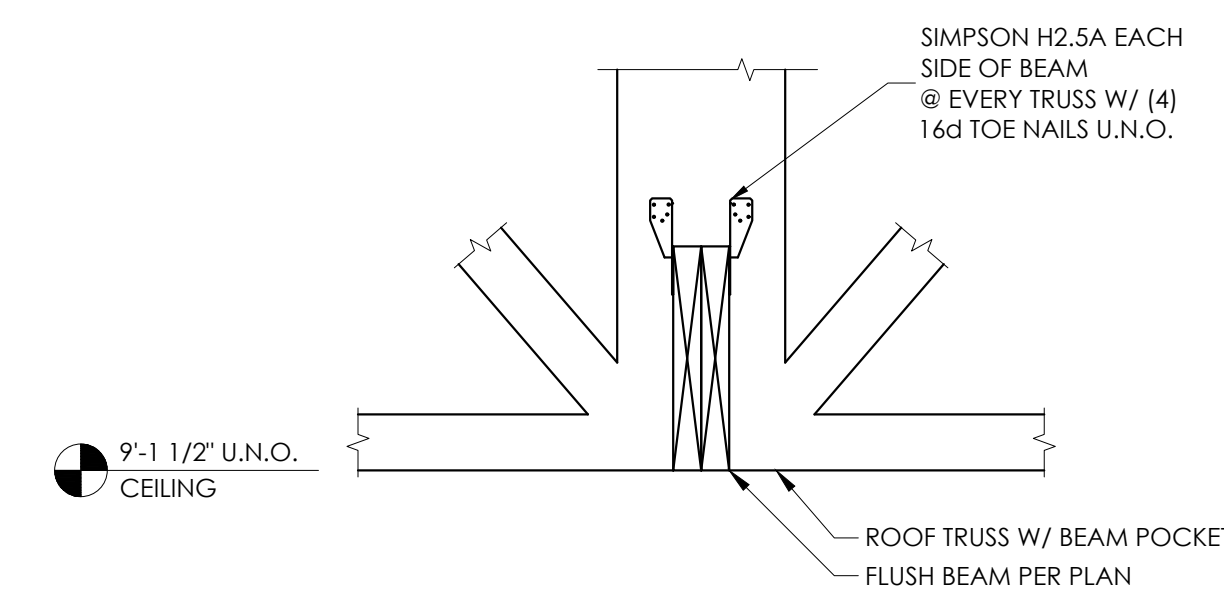
1 S-7 SCALE: NONE



8 S-7 SCALE: NONE

9 S-7 SCALE: NONE

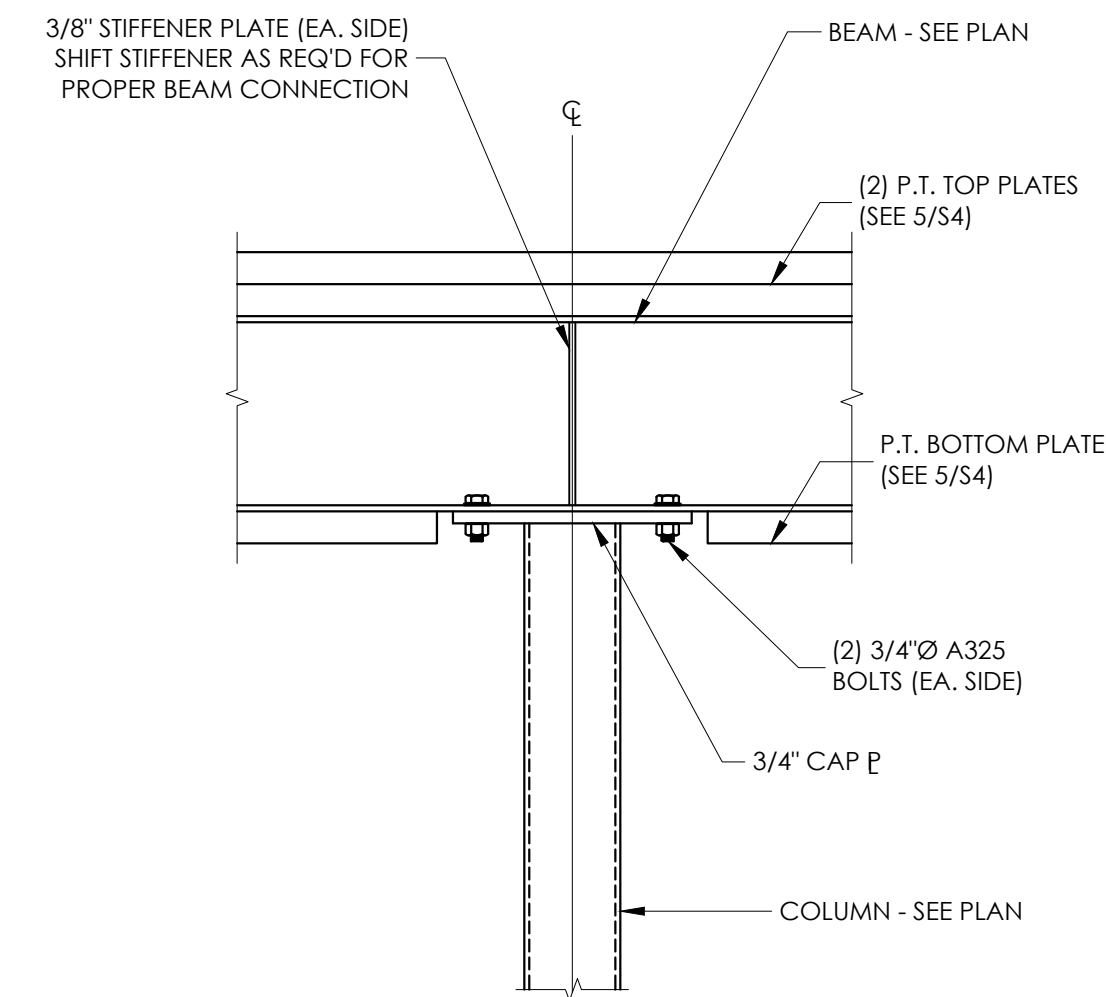
10 S-7



11 S-7 SECTION @ FLUSH BEAM POCKET



1  
S-8 SCALE: NONE

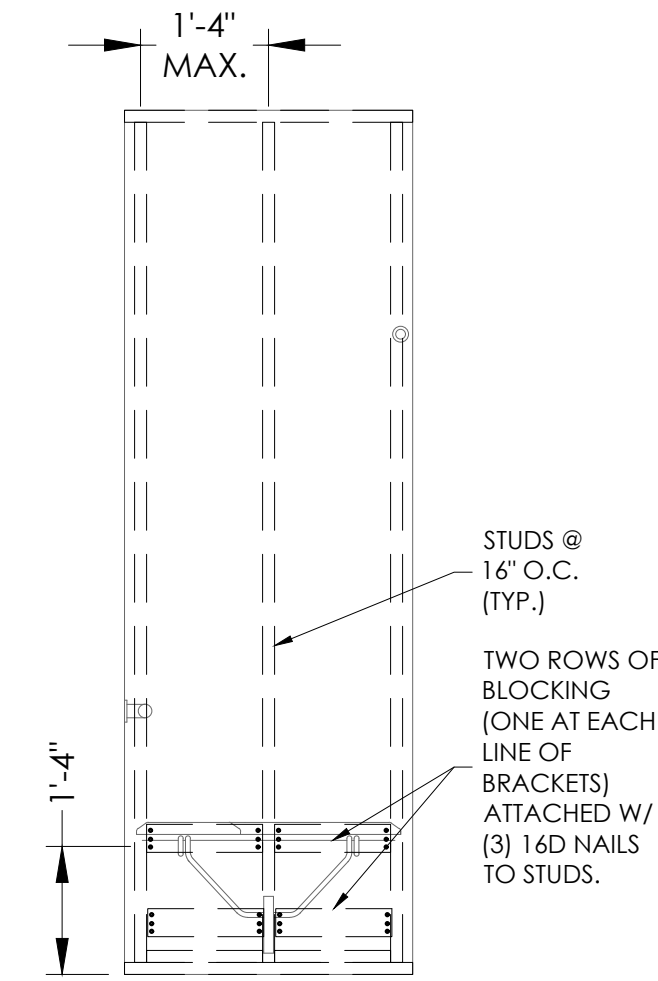


2  
S-8 BEAM-TO-COLUMN CONNECTION  
SCALE: NONE

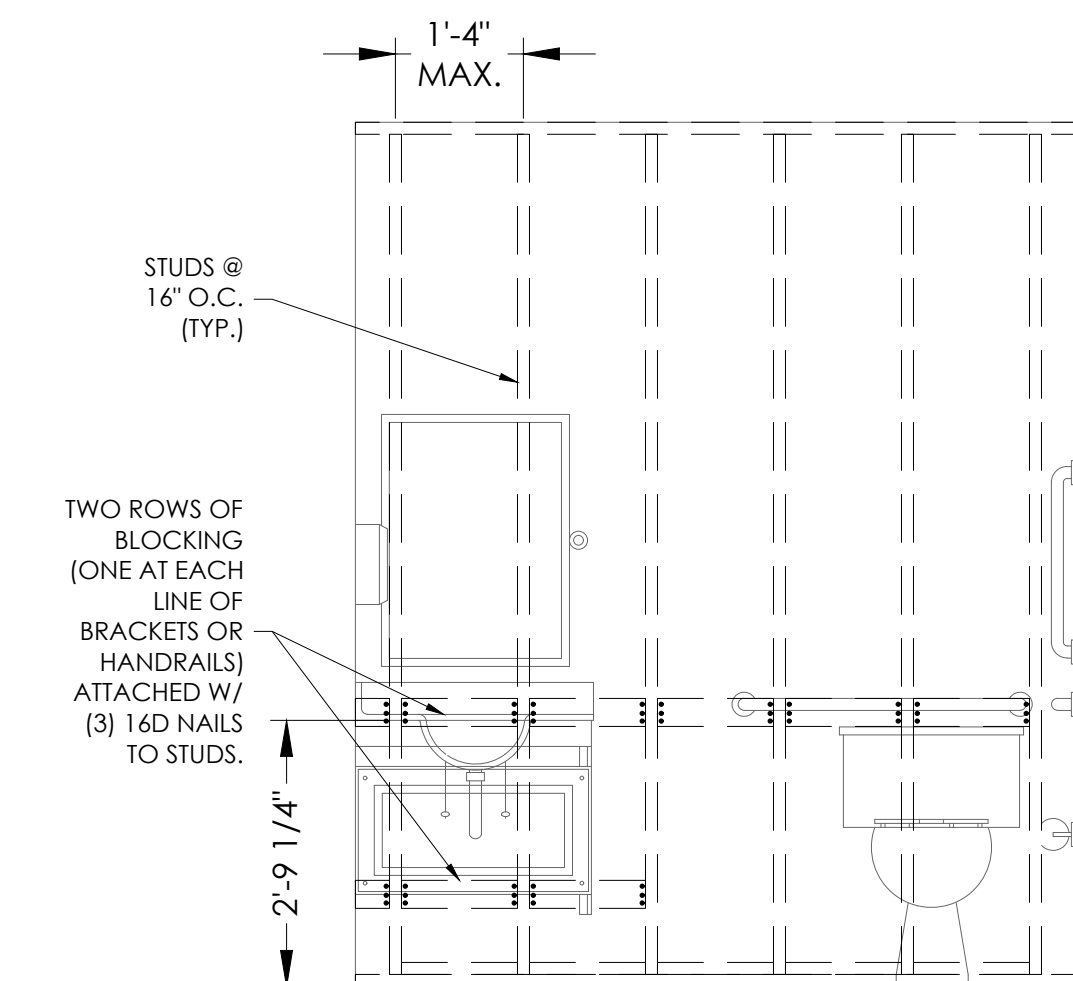
3  
S-8 SCALE: NONE

4  
S-8 SCALE: NONE

5  
S-8 SCALE: NONE



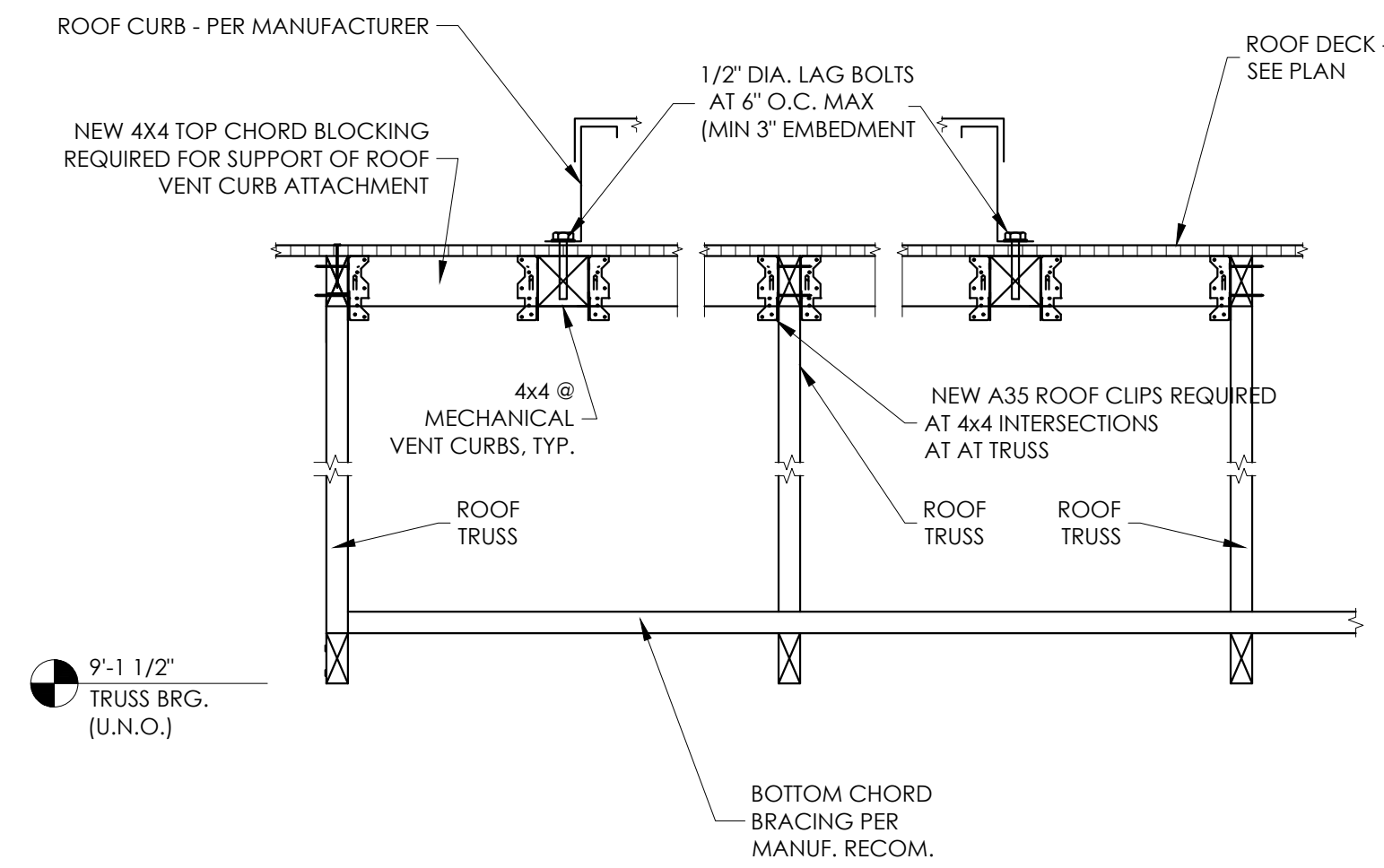
6  
S-8 ELEVATION - SHOWER SEAT  
SCALE: NONE



7  
S-8 ELEVATION - TOILET AND SINK  
SCALE: NONE

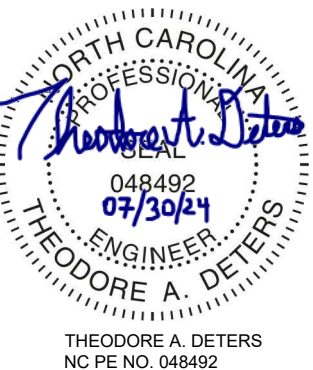
8  
S-8 SCALE: NONE

9  
S-8 SCALE: NONE



10  
S-8 ROOF TOP CURB SUPPORT  
SCALE: NONE

11  
S-8 SCALE: NONE



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**UNI HEALTH  
POST-ACUTE CARE**  
RALEIGH, NC

**David R. Polston - Architect**  
3806 Park Ave. Suite 2-L, Wilmington, NC 28403  
Architecture Planning Design

**24 BED  
ADDITION**

ISSUE DATE: 7.30.2024

REV	DATE





**STRUCTURAL DESIGN DATA SHEET (ASCE 7-10):**

RISK CATEGORY III (ASCE 7-10)  
 OCCUPANCY CLASSIFICATION INSTITUTIONAL GROUP I-2 (2015 IBC)

IMPORTANCE FACTORS:  
 I seismic 1.25  
 I snow 1.10

LIVE LOADS:  
 ROOF 20 psf  
 CATWALK 40 psf  
 FLOOR 100 psf

SNOW LOAD:  
 Pg 15 psf

WIND LOAD:  
 Basic Wind Speed 120 MPH  
 Exposure Category B  
 Wind Base Shear (MWFRS)  
 Vx 48.0 K  
 Vy 48.0 K

SEISMIC LOAD:  
 Spectral Response  
 Ss 0.157  
 S1 0.077  
 Sds 0.167  
 Sd1 0.124  
 Seismic Design Category B  
 Seismic Site Class D (Default)  
 Structural System Light framed walls sheathed w/ structural panels  
 R-Factor 6.5  
 Analysis Procedure Equivalent Lateral Force  
 Seismic Base Shear  
 Vx 11.9 K  
 Vy 11.9 K

SEISMIC ANCHORAGE OF NON-STRUCTURAL COMPONENTS:  
 Per ASCE 7 Chapter 13, non-structural components are not required to be braced against seismic sway.

LATERAL DESIGN CONTROL:  
 X-Direction Wind  
 Y-Direction Wind

SOIL BEARING PROPERTIES:  
 Allowable Bearing Capacity 2000 psf (Presumptive)

**STATEMENT OF SPECIAL INSPECTIONS:**

Project Name: UNI Health Post-Acute Care  
 Building Permit Number:  
 Project Address: Raleigh, North Carolina

The following information is being submitted in accordance with the Special Inspection provisions of the International Building Code. Attached is the Schedule of Special Inspections (SSI) required for this project.

The Special Inspection program outlined herein does not relieve the Contractor or any other entity of contractual duties, including quality control, quality assurance or safety. The contractor is solely responsible for construction means, methods and job site safety.

Respectfully submitted,  
 The Structural Engineer of Record

Signature: *Theodore A. Detters* Date: 07/30/24

**REINFORCED CONCRETE:**

- ALL CONCRETE WORK SHALL CONFORM TO THE 'BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE,' (ACI 318, 14)
- REINFORCING STEEL SHALL BE DEFORMED BARS ASTM A-615 (GRADE 60)
- THE COMPRESSIVE STRENGTH AT 28 DAYS OF CAST-IN-PLACE CONCRETE SHALL BE 3000 P.S.I. U.N.O. (SEE CIVIL DRAWINGS FOR SITE CONCRETE) KEEP COPY OF CONC. TEST REPORTS ON SITE AT ALL TIMES.
- LAP SPLICES FOR #5 REINFORCING BARS SHALL BE 24" MIN., U.N.O.
- CLEAR CONCRETE COVER FOR REINFORCING STEEL:  
 MASONRY WALLS: LOCATE IN CENTER OF WALL (U.N.O.)  
 FOOTINGS: 2" FORMED EDGES  
 3" CAST AGAINST GROUND  
 SLAB ON GRADE: MID-HEIGHT OF SLAB
- THE LONGITUDINAL REINFORCING STEEL IN WALLS AND FOOTINGS SHALL BE CONTINUOUS AROUND CORNERS. SEE TYPICAL DETAILS.
- ALL CONCRETE SHALL BE VIBRATED BY MECHANICAL VIBRATORS.

**STRUCTURAL STEEL:**

- ALL STRUCTURAL STEEL WORK SHALL CONFORM TO THE A.I.S.C. "STEEL CONSTRUCTION MANUAL" 360-10.
- STRUCTURAL STEEL SHALL BE ASTM A-992.
- STRUCTURAL TUBES SHALL BE ASTM A500, GRADE B.
- STEEL FRAMING CONNECTIONS SHALL BE BOLTED OR WELDED. BOLTS SHALL BE 3/4" DIAMETER MINIMUM AND SHALL BE ASTM A-325-N U.N.O., SNUG TIGHT ALL CONNECTIONS.
- ANCHOR BOLTS SHALL BE ASTM F1554 HEADED BOLTS, MINIMUM ANCHOR BOLT EMBEDMENT LENGTH SHALL BE 12 BOLT DIAMETERS U.N.O. CLEAN ANCHOR BOLTS OF ALL GREASE, DIRT, ETC., BEFORE INSTALLATION.
- PAINT ALL STRUCTURAL STEEL WITH ONE COAT OF RED OXIDE RUST-INHIBITIVE PRIMER 2.5 MILS IN THICKNESS. THE COMPATIBILITY OF PRIMER AND ANY TOP COAT SHALL BE VERIFIED BEFORE ANY PAINTING IS PERFORMED. TOUCH-UP ALL EXPOSED METAL AFTER FIELD INSTALLATION. ALL STRUCTURAL STEEL WHICH IS EXPOSED TO THE ELEMENTS SHALL RECEIVE TWO COATS OF EXTERIOR ENAMEL WHICH IS COMPATIBLE TO THE PRIMED SURFACE.
- THE SHOP DRAWINGS SHALL INCLUDE COMPLETE DETAILS AND SCHEDULES FOR FABRICATION AND ASSEMBLY OF STRUCTURAL STEEL MEMBERS. SUBMIT FOUR PRINTS OF EACH DRAWING. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED. CONTRACTOR TO REVIEW AND STAMP DRAWINGS PRIOR TO SUBMISSION TO THE EOR.

**WOOD TRUSSES:**

- ROOF TRUSSES SHALL BE DESIGNED TO SUPPORT THE DESIGN LOADS INDICATED IN THE DESIGN INFORMATION SECTION.
- IN ADDITION TO THE UNIFORM LOADING SPECIFIED FOR TRUSS DESIGN, THE TRUSS SUPPLIER SHALL INCLUDE ANY CONCENTRATED LOADS CAUSED BY ARCHITECTURAL FEATURES OR M. P&E EQUIPMENT OR MATERIALS AND BY SPRINKLER LOADS IN THE TRUSS DESIGN.
- TRUSSES SHALL BE DESIGNED BY A REGISTERED ENGINEER IN THE STATE OF NORTH CAROLINA AND SHOP DRAWINGS BEARING THE ENGINEER'S SEAL SHALL BE SUBMITTED FOR APPROVAL.
- TRUSSES SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH APPLICABLE STANDARDS OF THE TRUSS PLATE INSTITUTE TPI-1-2002.
- LIMIT LL DEFLECTION TO L/360. LIMIT TL DEFLECTION TO L/240 OR 1.25" MAX.

**DESIGN INFORMATION:**

- ALL CONSTRUCTION SHALL CONFORM TO THE 2018 NORTH CAROLINA BUILDING CODE, 2015 INTERNATIONAL BUILDING CODE AND ASCE 7-10.
- DESIGN LOADS:  
 DEAD AND LIVE LOADS  
 ROOF LOADS  
 TOP CHORD DEAD 15 psf  
 BOTTOM CHORD DEAD 5 psf  
 TOP CHORD LIVE 20 psf  
 BOTTOM CHORD LIVE 10 psf (WITHOUT ATTIC STORAGE)  
 CATWALK 40 psf  
 FLOOR LOADS  
 TOP CHORD DEAD N/A  
 BOTTOM CHORD DEAD N/A  
 TOP CHORD LIVE N/A  
 BOTTOM CHORD LIVE N/A  
 RISK CATEGORY III  
 IMPORTANCE FACTORS  
 I seismic 1.25  
 I snow 1.10  
 GROUND SNOW LOAD (pg) 15 psf  
 DESIGN WIND SPEED 120 mph  
 SEISMIC DESIGN PARAMETERS  
 S1 7.7 %g  
 Ss 15.7 %g  
 SITE CLASS D (DEFAULT)  
 Sds 0.167  
 Sd1 0.124  
 SEISMIC DESIGN CATEGORY C  
 R 6.5  
 Cv 0.0321

- ADDITIONAL LIVE LOADS PRESCRIBED IN ASCE7-10 RELATED TO ROOF ATTICS AND ROOF TRUSSES, INCLUDING LIMITED ACCESS STORAGE IN ATTICS SHALL APPLY TO PRE-FABRICATED TRUSSES, AND SHALL BE CLEARLY IDENTIFIED ON THE TRUSS SHOP DRAWINGS..
- THE DESIGN ADEQUACY AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC. IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- FOR LOCATION OF MISCELLANEOUS ITEMS (SUCH AS INSERTS, ETC.) AFFECTING STRUCTURAL WORK, SEE ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS.
- THIS PROJECT CONTAINS A SERIES OF DETAILS CONSIDERED "TYPICAL DETAILS". THESE SHALL APPLY AT ALL SITUATIONS THAT ARE THE SAME OR SIMILAR AS THESE DETAILS. THESE "TYPICAL DETAILS" SHALL APPLY WHETHER OR NOT THEY ARE INDICATED OR CUT AT EACH LOCATION.
- VERIFY EXISTING CONDITIONS AND NOTIFY ARCHITECT AND ENGINEER OF ANY CONDITIONS WHICH DO NOT COMPLY WITH PLANS AND SPECIFICATIONS. STRUCTURAL DRAWINGS MUST BE WORKED WITH ARCHITECTURAL DRAWINGS.
- USE OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED. THE CONTRACTOR SHALL REVIEW AND STAMP DRAWINGS ACCORDINGLY PRIOR TO SUBMITTING TO THE ENGINEER. THE OMISSION OF ITEMS FROM SHOP DRAWINGS SHALL NOT RELIEVE CONTRACTOR OF RESPONSIBILITY OF FURNISHING AND INSTALLING ITEMS REGARDLESS OF WHETHER SHOP DRAWINGS HAVE BEEN REVIEWED AND APPROVED.

**WOOD FRAMING (NOT INCLUDING PRE-FABRICATED TRUSSES):**

- ALL WOOD CONSTRUCTION SHALL CONFORM TO THE 2018 NORTH CAROLINA BUILDING CODE, 2015 INTERNATIONAL BUILDING CODE AND TO THE NDS.
- ALL NAILING (UNLESS NOTED OTHERWISE) SHALL CONFORM TO THE NORTH CAROLINA BUILDING CODE.
- ALL STUDS, TOP PLATES AND SILL PLATES IN BEARING WALLS AND SHEARWALLS SHALL BE SPF NO. 2 OR BETTER.
- ALL STUDS, TOP PLATES AND SILL PLATES IN NON-BEARING WALLS SHALL BE SPF NO. 3 OR BETTER.
- ALL 2x NOMINAL HEADERS SHALL BE SPF NO. 2 OR BETTER OR SYP NO. 2 OR BETTER.
- ALL EXPOSED LUMBER SHALL BE PRESERVATIVE TREATED.
- FINGER JOINTED STUDS MAY BE USED IN INTERIOR APPLICATIONS PROVIDED THE STRUCTURAL PROPERTIES EQUAL OR EXCEED THAT OF THE SOLID SAWN LUMBER. FINGER JOINTED LUMBER SHALL NOT BE USED IN EXPOSED CONDITIONS.
- ALL CONNECTIONS IN EXPOSED LUMBER SHALL BE HOT DIPPED GALVANIZED OR STAINLESS STEEL.
- ALL LUMBER IN CONTACT WITH CONCRETE SHALL BE PRESERVATIVE TREATED.
- ALL MANUFACTURED LAMINATED VENEER LUMBER (LVL) SHALL HAVE A MODULUS OF ELASTICITY OF 266 psi AND A MINIMUM BENDING STRENGTH OF 2800 psi.
- UNDER NO CIRCUMSTANCE SHALL LAMINATED VENEER LUMBER BE USED IN AN EXPOSED CONDITION. WHERE MANUFACTURER LUMBER IS REQUIRED IN AN EXPOSED CONDITION THE CONTRACTOR MUST USE PRESERVATIVE TREATED GLU-LAMINATED LUMBER (GLB).
- ALL GLU-LAMINATED LUMBER SHALL BE GRADED ACCORDING TO THE PLANS. IF NO GRADE IS SPECIFIED A MINIMUM GRADE OF 4VF2400 SHALL BE USED.

**FOUNDATION NOTES:**

- FOUNDATION DESIGN IS BASED UPON ASSUMED SOIL VALUES. CONTRACTOR/OWNER SHALL VERIFY PRIOR TO CONSTRUCTION. FOOTINGS ARE DESIGNED TO BEAR ON UNIFORM SUITABLE SOIL CAPABLE OF SUPPORTING 2000 PSF.
- IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO FOLLOW RECOMMENDATIONS BY A LICENSED GEOTECHNICAL ENGINEER TO ACHIEVE 2000 PSF AND LESS THAN 1" ANTICIPATED SETTLEMENT.
- THE SOIL BEARING CAPACITY AND CONSISTENCY SHALL BE VERIFIED FOR THE BUILDING LIMITS BY A REGISTERED GEO-TECHNICAL ENGINEER WHEN FOUNDATION EXCAVATIONS HAVE BEEN CARRIED DOWN TO THE PROPOSED ELEVATIONS. THE BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A 2'-0" MINIMUM BELOW ADJACENT GRADE. (U.N.O.)
- WHERE FOOTING EXCAVATIONS ARE TO REMAIN OPEN AND MAY BE EXPOSED TO RAINFALL, THE EXCAVATIONS SHALL BE UNDERCUT AND A 3" THICK MID MAT OF 2000 PSI CONCRETE SHALL BE PLACED OR CLEAN GRAVEL SHALL BE PLACED IN THE BOTTOM TO PROTECT THE BEARING SOILS.
- WHERE FOOTING STEPS ARE NECESSARY, THEY SHALL BE NO STEEPER THAN 1 VERTICAL TO 2 HORIZONTAL UNLESS SHOWN OTHERWISE ON PLANS.
- IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY FOR PREPARING THE BUILDING PAD PER THE GEOTECHNICAL ENGINEER OF RECORD'S RECOMMENDATIONS.
- SITE IS ASSUMED TO BE MASS GRADED. GRADING PLANS WERE NOT PROVIDED AT TIME OF DESIGN AND MUST BE PROVIDED FOR COORDINATION PRIOR TO CONSTRUCTION.

**CONCRETE MASONRY:**

- CONCRETE MASONRY SHALL CONFORM TO THE NATIONAL CONCRETE MASONRY ASSOCIATION SPECIFICATIONS, AND HAVE A DENSITY OF 125 P.C.F. AND SHALL HAVE A MINIMUM PRISM STRENGTH (Fm) OF 1500 P.S.I.
- GROUT FOR FILLING CONCRETE MASONRY CELLS SHALL CONFORM TO STANDARD SPECIFICATIONS FOR "GROUT FOR MASONRY". ASTM C-476-02. AND SHALL HAVE A COMPRESSIVE PRISM STRENGTH (Fm) OF 3000 P.S.I. AT 28 DAYS. THE SLUMP SHALL BE BETWEEN 9" AND 11". WHERE THE MINIMUM DIMENSION OF ANY CONTINUOUS VERTICAL CELL IS 3" OR LESS, USE FINE GROUT. OTHERWISE USE COARSE (PEA GRAVEL) GROUT.
- MORTAR FOR CONCRETE MASONRY SHALL BE TYPE "S" AND SHALL CONFORM TO ASTM C-270-04. 4. GROUT PROCEDURES AND REBAR INSTALLATION SHALL PER ASTM ACI 530 1-99. PROVIDE 36" LAP SPLICES IN REBAR IN 12" CMU FIRE WALL.

**SCHEDULE OF SPECIAL INSPECTIONS:**

Project Name: UNI Health Post-Acute Care  
 Construction divisions which require inspections for this project are as follows:

INSPECTION TASK	CONTINUOUS (C) OR PERIODIC (P) INSPECTIONS		SPECIAL INSPECTIONS FIRM	NOTES & SCOPE
	C	P		
<b>1. VERIFICATION OF SOILS (Table 1704.7)</b>				
Verify materials below shallow Foundations are adequate to achieve the design bearing capacity.		P	Testing Agency (TA)	Testing Agency shall provide soils report
Verify excavations are extended to proper depth.		P	Testing Agency (TA)	
Perform Classification and testing of compacted fill materials.		P	Testing Agency (TA)	
Verify use of proper materials, densities and lift thickness during placement and compaction of compacted fill.	C		Testing Agency (TA)	
Prior to placement of compacted fill, observe sub-grade and verify that site has been prepared properly.		P	Testing Agency (TA)	
<b>2. REINFORCED CONCRETE (Table 1704.4)</b>				
Inspection of reinforcing steel, including prestressing tendons, and placement. ACI 318:3.5, 7.1-7.7		P	Testing Agency (TA)	ACI 318: 3.5.7, 1-7.7 IBC: 1913.4
Verifying use of required design mix: ACI 318: Ch. 4, 5.2-5.4		P	Testing Agency (TA)	ACI 318: Ch. 4, 5.2-5.4 IBC: 1904.2.2, 1913.2, 1913.3
At the time fresh concrete is sampled to fabricate specimens for strength tests, slump, air content, and temperature of concrete.	C		Testing Agency (TA)	ASTM C 172, C 31 ACI: 318: 5.6, 5.8 IBC: 1913.10
<b>2. REINFORCED CONCRETE (Table 1704.4)</b>				
Inspect OSB nailing patterns per structural plans. Inspect roof truss and top plate ties, holdowns, and anchorage per structural plans		P	Special Inspector (SI)	

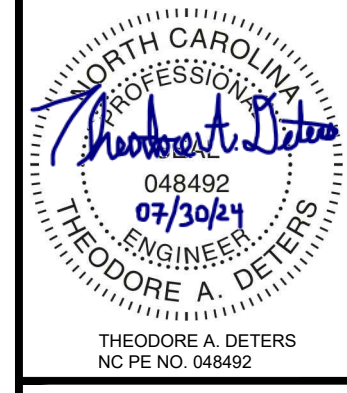
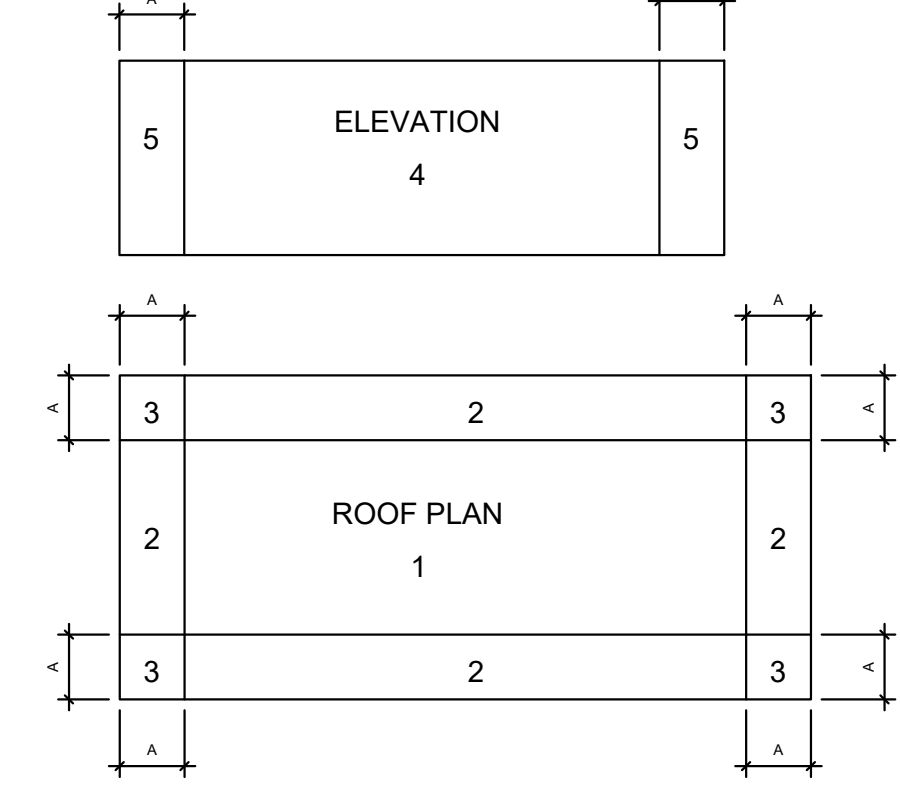
**SCHEDULE OF SPECIAL INSPECTIONS (Continued):**

Project Name: UNI Health Post-Acute Care  
 Construction divisions which require inspections for this project are as follows:

INSPECTION TASK	CONTINUOUS (C) OR PERIODIC (P) INSPECTIONS		SPECIAL INSPECTIONS FIRM	NOTES & SCOPE
	C	P		
<b>3. STRUCTURAL STEEL (Table 1704.3)</b>				
Material verification of high strength bolts, nuts and washers.		P	Special Inspector (SI)	AISC 360, A3.3
Inspection of high strength bolting, snug tight joints		P	Special Inspector (SI)	AISC 360, M2.5 IBC 1704.3.3
Material verification of structural steel.		P	Special Inspector (SI)	Fabricator's bill of materials verification is acceptable.
All field welding.		P	Special Inspector (SI)	AWS D1.1 IBC 1704.3.1
<b>4. RETAINING WALLS (Table 1704.12)</b>				
Inspect all retaining walls over 5 feet in height.		P	Testing Agency (TA)	
<b>5. MASONRY</b>				
As masonry construction begins, the following shall be verified to ensure compliance: (A) Proportions of site mixed mortar. (B) Construction of mortar joints. (C) Location of reinforcement and connectors.		P	Testing Agency (TA)	ACI 318: 3.5.7, 1-7.7 IBC: 1913.4
The inspection program shall verify: (A) Size and location of structural elements. (B) Size, grade, type of reinforcement. (C) Protection of masonry during cold weather (temperature below 40 degrees F) or hot weather (temperature above 90 degrees F)		P	Testing Agency (TA)	Sec. 2108.9.2.1.1, Item 2, Sec. 2104.3, 2104.4, ACI Sec. 1.15.4, 2.1.2, Sec. 1.12, Sec 2.1.8.6, 2.1.8.6.2, ACI 3.3G, Art 2.4.3.4, Art 1.8
Prior to grouting, the following shall be verified to ensure compliance: (A) Grout space is clean. (B) Placement of reinforcement and connectors. (C) Proportions of site-prepared grout. (D) Construction of mortar joints		P	Testing Agency (TA)	Sec. 1.12, Art. 3.2D, Art 3.4, Art. 2.6B, Art. 3.3B
Grout Placement shall be verified to ensure compliance with code and construction provisions.		P	Testing Agency (TA)	Art. 3.5

WIND LOAD SCHEDULE					
COMPONENTS & CLADDING	ROOF WIND LOAD			WALL WIND LOADS	
	ROOF AREA			WALL AREA	
	1	2	3	4	5
PRESSURE (PSF)	+10.6	+10.6	+10.6	+25.5	+25.5
SUCTION (PSF)	-21.5	-52.3	-58.8	-27.7	-33.8

- CORNER DISTANCE, A=5 FEET, ROOF = 100 SF, WALL = 13 S.F. C&C
- VALUES ARE NOT FACTORED. ASD LOAD FACTOR IS 0.6 FOR WIND.
- DP FOR WINDOW AND DOOR CAN CONSERVATIVELY USE NEGATIVE PRESSURES AT WALL AREA 5.



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