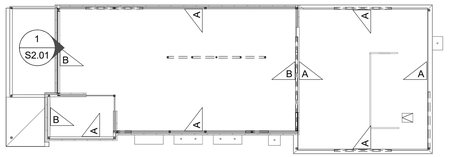
GENERAL

A. USE THE STRUCTURAL DRAWINGS WITH THE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND SHOP DRAWINGS.
B. CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL CONTRACT DOCUMENTS AND LATEST ADDENDA, AS WELL AS SUBMITTING TO ALL SUBCONTRACTORS AND SUPPLIERS PRIOR TO SUBMITTING SHOP DRAWINGS.
C. DO NOT SCALE DRAWINGS OR AUTO-DIMENSION ELECTRONIC FILES. NOTIFY ARCHITECT AND ENGINEER OF ANY DISCREPANCIES IN WRITING PRIOR TO FABRICATION OR CONSTRUCTION.
D. COMPARE ALL CONTRACT DRAWINGS AND REPORT ANY DISCREPANCIES BETWEEN DISCIPLINES, AND WITHIN A GIVEN DISCIPLINE, TO THE ARCHITECT AND ENGINEER PRIOR TO FABRICATION AND ERECTION.
E. IF A CONFLICT EXISTS AMONG THE STRUCTURAL DRAWINGS OR GENERAL NOTES, THE STRICTEST REQUIREMENTS, AS INDICATED BY THE ENGINEER, GOVERNS.
F. COORDINATE ALL ELEVATIONS AND DIMENSIONS, INCLUDING BUT NOT LIMITED TO, OPENINGS IN WALLS AND IN ROOF AND FLOOR SYSTEMS, WITH THE ARCHITECTURAL, PLUMBING, ELECTRICAL, AND MECHANICAL PLANS.
G. VERIFY ALL DIMENSIONS, ELEVATIONS, AND ANY OTHER EXISTING CONDITIONS. NOTIFY THE ARCHITECT AND ENGINEER OF DISCREPANCIES BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK. DURING THE CONSTRUCTION PROCESS, IT IS THE CONTRACTOR'S RESPONSIBILITY TO MAINTAIN THE INTEGRITY OF THE EXISTING STRUCTURE AND TO PROTECT FROM DAMAGE ANY PORTIONS THAT REMAIN. THE SHORING AND BRACING SHOWN (IF ANY) IS A PARTIAL AND SCHEMATIC REPRESENTATION. DETERMINE THE ERECTION PROCEDURE TO ENSURE THE STABILITY AND SAFETY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION.
H. THE COMPLETED LATERAL-FORCE RESISTING SYSTEMS (LFRS), INCLUDING THE DIAPHRAGMS, ARE REQUIRED TO RESIST LATERAL LOADS AND PROVIDE STABILITY UNDER GRAVITY LOADS. DURING CONSTRUCTION, THE CONTRACTOR IS RESPONSIBLE FOR ALL BRACING DURING CONSTRUCTION TO MAINTAIN THE STABILITY AND SAFETY OF ALL STRUCTURAL ELEMENTS UNTIL THE LATERAL-LOAD RESISTING OR STABILITY-PROVIDING SYSTEM IS COMPLETELY INSTALLED AND THE STRUCTURE IS COMPLETELY TIED TOGETHER.
I. UNLESS NOTED OTHERWISE, DETAILS SHOWN ARE TYPICAL FOR ALL SIMILAR CONDITIONS.
J. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS AND METHODS, AS WELL AS SAFETY PRECAUTIONS AND PROGRAMS.
K. BRITT, PETERS & ASSOCIATES, INC. IS NOT RESPONSIBLE FOR ACTS OR OMISSIONS OF THE CONTRACTOR, NOR FAILURE TO PERFORM WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
L. THE BUILDING OWNER IS RESPONSIBLE FOR PERIODIC MAINTENANCE TO ENSURE STRUCTURAL INTEGRITY. MAINTENANCE INCLUDES, BUT IS NOT LIMITED TO, STEEL/CONCRETE COATINGS, SEALANTS, CAULKED JOINTS, EXPANSION JOINTS, CONTROL JOINTS, SPALLS, AND CRACKS IN CONCRETE, AND CLEANING OF EXPOSED STRUCTURAL ELEMENTS.



SNOW DRIFT PLAN

DESIGN CRITERIA

A. STRUCTURAL DRAWINGS ARE BASED ON THE REQUIREMENTS OF THE 2021 INTERNATIONAL BUILDING CODE, W/SC AMMENDMENTS AND THE REFERENCED SECTIONS WITHIN.
B. DEAD LOADS:
1. ROOF SYSTEMS:
a. STEEL (20 PSF TOTAL)
1. STRUCTURE 6 PSF
2. MEP 4 PSF
3. INSULATION AND ROOFING 10 PSF
C. LIVE LOADS:
1. LIVE LOADS ARE BASED ON THE MORE RESTRICTIVE OF THE UNIFORM LOAD OR THE CONCENTRATED LOAD LISTED ACTING OVER A 6.25 SQUARE FOOT AREA. LIVE LOADS HAVE BEEN REDUCED AS PRESCRIBED IN THE AFOREMENTIONED BUILDING CODE.

LIVE LOADS

CATEGORY	UNIFORM LOAD (PSF)	CONCENTRATED LOAD (LBS)
DINING ROOMS/RESTAURANTS	100	
ROOFS: ALL ROOF SURFACES SUBJECT TO WORKERS		300
ROOFS: ORDINARY ROOF	20	

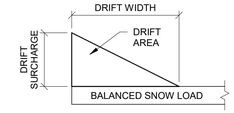
D. DESIGN SNOW LOADS:

1.	GROUND SNOW LOAD:	P_G	10 PSF
2.	FLAT ROOF SNOW LOAD:	P_F	12 PSF
3.	SNOW EXPOSURE FACTOR:	C_E	1.0
4.	SNOW THERMAL FACTOR:	C_T	1.0
5.	SLOPE FACTOR:	C_S	1.0
6.	SNOW IMPORTANCE FACTOR:	I_S	1.0
7.	DRIFT SURCHARGE:	P_d	SEE DIAGRAM
8.	SNOW DRIFT WIDTH:	w	SEE DIAGRAM
9.	RAIN-ON-SNOW SURCHARGE:		5PSF

SNOW DRIFT DIAGRAM1

BALANCED SNOW LOAD : 7 PSF

DRIFT AREA	DRIFT SURCHARGE	DRIFT WIDTH
A	19 PSF TO 0 PSF	5'-0"
B	26 PSF TO 0 PSF	6'-7"



E. DESIGN WIND LOADS:

1.	BASIC WIND SPEED:	V_{ULT}	113 MPH (3-SEC GUST)
2.	BASIC WIND SPEED:	V_{ASD}	88 MPH (3-SEC GUST)
3.	RISK CATEGORY:		II
4.	WIND EXPOSURE:		C
5.	INTERNAL PRESSURE COEFF:	GC_{PI}	±0.18
6.	COMPONENTS & CLADDING WIND PRESSURES (ULTIMATE):		

Ultimate Design Wind Pressure (psf):

			Effective Wind Area (sq ft)					
Walls:			**10**	**20**	**50**	**100**	**200**	**500**
Interior	Zone 4	+	26.5	25.4	23.8	22.6	21.5	23.6
		-	-28.8	-27.6	-26.0	-24.9	-23.7	-25.8
Edge	Zone 5	+	26.5	25.4	23.8	22.6	21.5	23.6
		-	-35.4	-33.0	-29.9	-27.6	-25.2	-29.6
Roof:			**10**	**20**	**50**	**100**	**200**	**500**
Interior	Zone 1	+	16.0	16.0	16.0	16.0	16.0	16.0
		-	-46.2	-43.2	-39.1	-36.1	-33.0	-38.6
Interior	Zone 1'	+	16.0	16.0	16.0	16.0	16.0	16.0
		-	-26.5	-26.5	-26.5	-26.5	-22.8	-26.5
Edge	Zone 2	+	16.0	16.0	16.0	16.0	16.0	16.0
		-	-61.0	-57.0	-51.9	-47.9	-44.0	-51.2
Corner	Zone 3	+	16.0	16.0	16.0	16.0	16.0	16.0
		-	-83.1	-75.2	-64.9	-57.0	-49.2	-63.6
Overhang:			**10**	**20**	**50**	**100**	**200**	**500**
Edge	Zone 2	+	16.0	16.0	16.0	16.0	16.0	16.0
		-	-56.5	-51.3	-44.4	-39.2	-33.9	-43.5
Corner	Zone 3	+	16.0	16.0	16.0	16.0	16.0	16.0
		-	-78.7	-69.5	-57.4	-48.3	-39.1	-55.9
Parapet:			**10**	**20**	**50**	**100**	**200**	**500**
Edge	Zone 2	+	79.5	74.4	67.6	62.4	57.3	66.7
		-	-47.0	-44.6	-41.5	-39.1	-36.7	-41.1
Corner	Zone 3	+	101.9	92.8	80.7	71.6	62.5	79.3
		-	-53.7	-50.1	-45.4	-41.8	-38.3	-44.8

WIDTH OF ZONE, a = 4.1 FT

F. SEISMIC LOADS:

1.	RISK CATEGORY:		II
2.	SEISMIC IMPORTANCE FACTOR:	I_E	1.0
3.	SHORT PERIOD SPECTRAL RESPONSE ACCELERATION:	S_S	0.259 g
4.	1-SEC PERIOD SPECTRAL RESPONSE ACCELERATION:	S_1	0.094 g
5.	SITE CLASS:		**D**
6.	SHORT PERIOD DESIGN SPECTRAL RESPONSE ACCELERATION:	S_{DS}	0.275 g
7.	1-SEC PERIOD DESIGN SPECTRAL RESPONSE ACCELERATION:	S_{D1}	0.151 g
8.	SEISMIC DESIGN CATEGORY:		**C**
9.	BASIC SEISMIC-FORCE RESISTING SYSTEM:	STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE	
10.	DESIGN BASE SHEAR:	V	38 K
11.	SEISMIC RESPONSE COEFFICIENT:	C_S	0.09
12.	RESPONSE MODIFICATION FACTOR:	R	3

G. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
H. RAIN LOADS:
1. RAIN INTENSITY (100-YEAR STORM): I ____ IN/HR
I. VERIFY ALL MECHANICAL EQUIPMENT WEIGHTS, LOCATIONS,, AND ASSOCIATED OPENINGS WITH THE MECHANICAL CONTRACTOR, AND SUBMIT INFORMATION PRIOR TO FABRICATION OF THE SUPPORTING STRUCTURE. NOTIFY THE ENGINEER IF THE ACTUAL WEIGHT EXCEEDS THE WEIGHT INDICATED ON THE STRUCTURAL DRAWINGS.
J. DESIGN, DETAIL, AND CONSTRUCT WALLS, PARTITIONS, ROOFING, CLADDING, AND OTHER COMPONENTS TO ACCOMMODATE VERTICAL DEFLECTIONS AND LATERAL DRIFTS.
1. ALLOWABLE INTERSTORY DRIFT = 0.0025*H (10 YEAR SERVICE LEVEL WIND)
2. ALLOWABLE INTERSTORY DRIFT = **[0.007] [0.010] [0.015] [0.025]** *H (SEISMIC)
K. DETAIL WOOD CONSTRUCTION TO ACCOMMODATE ANTICIPATED SHRINKAGE. PLUMBING, HOLDOWN ROD SYSTEMS, MASONRY SHAFTS, MASONRY VENEER SUPPORTS, WINDOW SILLS WITH MASONRY VENEER, ETC MUST BE CONSTRUCTED TO ALLOW FOR WOOD SHRINKAGE.

FOUNDATIONS

A. FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT BY ECS SOUTHEAST, LLC., DATED NOVEMBER 7, 2023 TITLED "GEOTECHNICAL ENGINEERING REPORT -MONTERREY RESTAURANT MEXICANO".
B. REVIEW THE GEOTECHNICAL REPORT AND ADHERE TO ALL RECOMMENDATIONS WITHIN, INCLUDING CUT, SUBGRADE PREPARATION, FILL, ETC.
C. FOUNDATIONS HAVE BEEN DESIGNED USING A NET SOIL BEARING PRESSURE OF 2,500 PSF.
D. ALL SOILS WORK, INCLUDING BACKFILL OF UTILITY TRENCHES AND THE VERIFICATION OF BEARING CAPACITY MUST BE UNDER THE DIRECTION OF A QUALIFIED GEOTECHNICAL ENGINEER. PROXIMITY OF UTILITY TRENCHES TO BUILDING FOUNDATION SYSTEM MUST BE AS APPROVED BY THE GEOTECHNICAL ENGINEER TO ENSURE INTEGRITY OF THE BEARING SOILS.
E. ALL FOUNDATIONS BEAR ON UNDISTURBED EARTH OR ENGINEERED FILL AT ELEVATIONS SHOWN ON PLANS AND DETAILS. COORDINATE FINAL TOP OF FOOTING ELEVATIONS WITH THE ARCHITECTURAL ELEVATIONS, MEP DRAWINGS, AND CIVIL GRADING PLANS PRIOR TO PLACEMENT. FOUNDATION STEPS INDICATED ARE APPROXIMATE, UNLESS NOTED OTHERWISE, AND MUST BE FIELD COORDINATED. THE BOTTOM OF EXTERIOR FOUNDATION ELEVATIONS MUST BE BELOW THE FROST DEPTH ELEVATION 18 INCHES MEASURED FROM EXTERIOR FINISHED GRADE.
F. BEAR FLOOR SLABS ON 4 INCH MINIMUM DRAINAGE COURSE (COMPACTED STONE) UNLESS NOTED OTHERWISE IN THE GEOTECHNICAL REPORT OR DRAWINGS. PLACE THE VAPOR RETARDER BETWEEN THE DRAINAGE COURSE AND THE SLAB. VAPOR RETARDER IS ASTM E1745, CLASS B, 10 MIL UNLESS NOTED OTHERWISE. PLACE, PROTECT, AND REPAIR PER ASTM E1643 AND MANUFACTURER'S INSTRUCTIONS.
G. DO NOT INSTALL FOUNDATION CONCRETE UNTIL ALL FOUNDATION WORK HAS BEEN COORDINATED WITH UNDERGROUND UTILITIES. NOTIFY THE ENGINEER OF ALL CONFLICTS BETWEEN FOUNDATIONS AND UTILITIES.
H. ALL FOUNDATIONS, OR PORTIONS THEREOF BELOW GRADE, MAY BE EARTH FORMED BY NEAT EXCAVATIONS. DO NOT PLACE FOUNDATIONS, SLABS, OR OTHER CONCRETE ON FROZEN SUBGRADE OR IN STANDING WATER.
I. CENTER ALL FOUNDATIONS ON WALLS AND/OR COLUMNS, UNLESS NOTED OTHERWISE.

CONCRETE

A. CONCRETE MUST CONFORM TO THE CONCRETE PROPERTIES SPECIFIED IN THE CONCRETE PROPERTIES TABLE.
B. CONCRETE MUST HAVE ALLOWABLE UNIT SHRINKAGE OF 0.045% AT 28 DAYS (SEE ASTM C157).
C. SLABS TO RECEIVE MOISTURE SENSITIVE FLOOR COVERINGS MUST HAVE MAXIMUM WATER/CEMENTITIOUS MATERIAL RATIO OF 0.45.
D. CONCRETE CONSTRUCTION MUST CONFORM TO THE CURRENT "ACI MANUAL OF CONCRETE PRACTICE".
E. ALL CONCRETE PLACEMENT SHALL ADHERE TO APPLICABLE SECTIONS OF ACI 305 AND ACI 306 FOR HOT WEATHER/COLD WEATHER CONCRETE PLACEMENT.
F. CONCRETE MATERIALS MUST CONFORM TO THE FOLLOWING SPECIFICATIONS:
1. PORTLAND CEMENT: ASTM C150, TYPE I OR II
2. AGGREGATE (NORMAL WEIGHT): ASTM C33
G. ALL REINFORCEMENT MUST CONFORM TO THE FOLLOWING SPECIFICATIONS:
1. ALL REINFORCING, UNO: ASTM A615 GRADE 60
2. DEFORMED BAR ANCHORS (DBA): ASTM A496 (75 KSI)
3. EPOXY-COATED REINFORCING: ASTM A775
4. GALVANIZED REINFORCING: ASTM A767 CLASS II (2.0 OZ. PER SF ZINC)
5. WELDABLE REINFORCING: ASTM A706 GRADE 60
6. WELDED WIRE REINFORCEMENT (WWR):
a. SMOOTH WIRE: ASTM A1064 (65 KSI)
b. DEFORMED WIRE: ASTM A1064 (70 KSI)
c. POLYPROPYLENE FIBRILLATED FIBER MAY BE USED TO SUBSTITUTE WWR IN SLABS ON GRADE WHEN ADDED TO CONCRETE MIX ACCORDING TO MANUFACTURER'S INSTRUCTIONS AND RECOMMENDED DOSAGES.
H. REINFORCEMENT DETAILING:
1. DETAIL AND PLACE REINFORCEMENT IN ACCORDANCE WITH ACI 315.
2. DEVELOPMENT AND SPLICE LENGTHS ARE IN TENSION UNLESS NOTED OTHERWISE. REFER TO THE REINFORCING BAR LAP LENGTH SCHEDULE ON THE TYPICAL DETAIL SHEETS.
3. PLACE WWR 2" CLEAR FROM TOP OF SLAB UNESS NOTED OTHERWISE. LAP WWR ONE CROSSWIRE SPACING PLUS 2".
4. INSTALL CORNER BARS AT ALL FOOTINGS AND WALL INTERSECTIONS TO MATCH HORIZONTAL REINFORCING SIZE AND SPACING. AT INTERSECTIONS OF CONTINUOUS SPREAD FOOTINGS, EXTEND ALL BARS TO FAR SIDE OF INTERSECTING FOOTING.
5. INSTALL AND SECURE REINFORCEMENT TO PREVENT DISPLACEMENT DURING CONCRETE PLACEMENT. PROVIDE THE FOLLOWING CONCRETE COVER FOR REINFORCING ACI 318 SECTION 7.7 AND IBC TABLE 720.1, UNLESS SPECIFICALLY NOTED OTHERWISE:
a. CAST AGAINST EARTH: 3"
b. EXPOSED TO EARTH/WEATHER: #6 THRU #18 2"
c. EXPOSED TO EARTH/WEATHER: #5 & SMALLER 1 1/2"
d. SLABS, WALLS, JOISTS: #14 & #18 1 1/2"
e. SLABS, WALLS, JOISTS: #11 & SMALLER 3/4"
f. BEAMS, COLUMNS: 1 1/2"
g. SHELLS FOLDED PLATE MEMBERS: #6 & LARGER 1 1/2"
h. SHELLS FOLDED PLATE MEMBERS: #5 & SMALLER 3/4"
6. INSTALL DOWELS TO MATCH REINFORCEMENT SIZE AND SPACING INDICATED, UNLESS NOTED OTHERWISE.
I. CAST FOUNDATION WALLS, GRADE BEAMS, AND FOOTINGS IN ALTERNATE PANELS NOT TO EXCEED 60'-0" IN LENGTH. INSTALL SHEAR KEYS AT EACH CONSTRUCTION JOINT AND LOCATED AT 1/3 POINTS OF SPANS.
J. TEMPORARILY BRACE CONCRETE WALLS AGAINST EARTH PRESSURE AND OTHER FORCES UNTIL FLOOR SLABS AND PERMANENT SUPPORTS ARE IN PLACE AND HAVE ATTAINED REQUIRED STRENGTHS.
K. DO NOT USE HORIZONTAL CONSTRUCTION JOINTS IN CONCRETE POURS UNLESS SHOWN ON THE DRAWINGS. THE ENGINEER MUST APPROVE ALL DEVIATIONS OR ADDITIONAL JOINTS IN WRITING.
L. CAST SLABS AND BEAMS/JOISTS MONOLITHICALLY UNLESS NOTED OTHERWISE.
M. CHAMFER ALL PERMANENTLY EXPOSED CONCRETE EDGES 3/4 INCH, UNLESS NOTED OTHERWISE.
N. REFERENCE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS OF OPENINGS AND SLEEVES IN CONCRETE WALLS AND SUPPORTED FLOORS. SPREAD REINFORCEMENT AT OPENINGS AND SLEEVES UNLESS OTHERWISE INDICATED. DO NOT CUT REINFORCEMENT.
O. SLOPE CONCRETE SLABS TO FLOOR DRAINS SHOWN ON MECHANICAL, PLUMBING, CIVIL, AND ARCHITECTURAL DRAWINGS.
P. BOND NEW CONCRETE TO HARDENED CONCRETE WITH A STRUCTURAL ADHESIVE BONDING AGENT PER ASTM C1059 THE SPECIFICATIONS. INSTALL PER THE MANUFACTURER'S INSTRUCTIONS.
Q. NO HOLES OR OPENINGS THROUGH FOUNDATION WALLS AND/OR FOOTINGS WITHOUT ENGINEER'S APPROVAL.
R. DO NOT EMBED ALUMINUM IN CONCRETE.

CONCRETE PROPERTIES

USAGE	STRENGTH (PSI)	TYPE	COMMENTS	DURABILITY CLASSIFICATION
ALL CONCRETE NOT OTHERWISE SPECIFIED	4000	NWT		F0, S0, W0, C1
FOOTINGS	3000	NWT		F0, S0, W0, C1
FOUNDATION WALLS	3000	NWT		F1, S0, W0, C1
SLAB-ON-GRADE INTERIOR	3500	NWT		F0, S0, W0, C0

CONCRETE PROPERTIES TABLE NOTES:
1. MINIMUM STRENGTH AND MAXIMUM DENSITY MEASURED AT 28 DAYS.
2. NWT = NORMAL WEIGHT CONCRETE
3. LWT = SAND-LIGHTWEIGHT CONCRETE 120 PCF MAX
a. 4% TO 7% AIR ENTRAINMENT FOR LIGHTWEIGHT CONCRETE ON COMPOSITE METAL DECKS
4. DURABILITY CLASSIFICATION INDICATES CONCRETE REQUIREMENTS BY EXPOSURE CLASS, REFER TO TABLE 19.3.2.1 OF ACI 318.

BRITT, PETERS AND ASSOCIATES INC
consulting engineers
101 Falls Park Drive
Suite 601
Greenville, SC 29601
(864) 271-8869
www.brittpeters.com
BPA Project #: 240281

Seal

DP3 ARCHITECTS
DP3 Architects, Ltd.
15 South Main Street, Suite 400
Greenville, SC 29601
864.232.8200
www.DP3architects.com

Project

MARIACHIS
RESTAURANT
LANCASTER, SC

Project Number	23213
Drawn By	BC
Checked By	ROG
Date	05/AUG/24

Revisions

Drawing

GENERAL NOTES

S0.00

CONCRETE UNIT MASONRY

A. MASONRY CONSTRUCTION MUST CONFORM WITH ACI 530.1.
B. CONCRETE MASONRY UNITS (CMU) ARE LIGHTWEIGHT COMPLYING WITH ASTM C90. UNITS HAVE A MINIMUM AVERAGE NET-AREA COMPRESSIVE STRENGTH OF 2,000 PSI. MINIMUM NET AREA COMPRESSIVE STRENGTH OF MASONRY (F'M) IS 2,000 PSI.
C. MORTAR MUST CONFORM TO ASTM C270, TYPE M OR S.
D. GROUT MUST CONFORM TO ASTM C476, WITH A 28 DAY COMPRESSIVE STRENGTH EQUAL TO OR GREATER THAN THE SPECIFIED NET AREA COMPRESSIVE STRENGTH OF MASONRY (F'M).
E. REINFORCING BARS ARE ASTM A615, GRADE 60.
F. VERTICAL AND HORIZONTAL REINFORCING ARE CONTINUOUS AND LAPPED A MINIMUM OF 72 BAR DIAMETERS.
G. POSITION AND HOLD REINFORCING STRAIGHT AS INDICATED. INSTALL REBAR POSITIONERS AT SPACING NOT TO EXCEED 200 BAR DIAMETERS, AT GROUT LIFT HEIGHTS, OR BAR SPLICE LOCATIONS, WHICHEVER IS LESS, TO HOLD REBAR IN PROPER LOCATION UNTIL GROUT CURES.
H. INSTALL 9 GAGE LADDER TYPE HORIZONTAL JOINT REINFORCING AT 16" OC MAXIMUM SPACING UNLESS NOTED OTHERWISE. JOINT REINFORCING COMPLIES WITH ASTM A951 AND GALVANIZED PER ASTM A153, CLASS B. LAP JOINT REINFORCEMENT AT LEAST 6 INCHES (MUST CONTAIN AT LEAST ONE CROSS WIRE OF EACH PIECE OF REINFORCEMENT WITHIN THE LAP). LAP WITH STANDARD T- AND L-SHAPED PIECES AT INTERSECTIONS AND CORNERS.
I. INSTALL DOWELS FROM FOUNDATIONS OR SUPPORTING CONCRETE MEMBER BELOW, SAME SIZE AND SPACING AS VERTICAL REINFORCING, UNLESS NOTED OTHERWISE. DOWELS HAVE STANDARD ACI HOOKS.
J. FULLY GROUT ALL CELLS AND WALLS BELOW GRADE. SLUSH JOINT BETWEEN WYTHES.
K. LOW-LIFT GROUTING PROCEDURES IN ACCORDANCE WITH ACI 530.1.
L. IF HIGH-LIFT GROUTING, COMPLY WITH ACI 530.1, INCLUDING CLEANOUTS AT EACH GROUTED CELL.
   1. DO NOT EXCEED 5 FEET GROUT POUR LIFT, UNLESS CLEANOUTS ARE PROVIDED IN THE BOTTOM COURSE OF EACH 5 FOOT LIFT.
   2. MECHANICALLY VIBRATE ALL LIFTS IN EXCESS OF 1 FOOT.
   3. DO NOT STOP GROUT POUR WITHIN 1-1/2 INCHES OF BED JOINT.
   4. TOTAL GROUT POUR MUST NOT EXCEED 24 FEET WHEN GROUTING THE CELLS OF HOLLOW MASONRY.
M. INSTALL MASONRY IN A RUNNING BOND PATTERN.
N. SHORE ALL MASONRY LINTELS UNTIL MASONRY AND GROUT HAVE SET FOR A MINIMUM OF 7 DAYS.
O. MASONRY WALLS HAVE BEEN DESIGNED IN THE FINAL CONSTRUCTED CONFIGURATION ASSUMING FULL BRACING TOP, BOTTOM, AND/OR SIDE OF WALL. DURING CONSTRUCTION, BRACE ALL CMU TO RESIST ERECTION AND LATERAL LOADS THAT MAY BE APPLIED PRIOR TO COMPLETION OF CONSTRUCTION.

STRUCTURAL STEEL

A. HOT ROLLED STEEL BARS, PLATES, SHAPES, AND SHEET PILING MUST BE NEW STEEL CONFORMING TO ASTM A6. FABRICATE AND INSTALL STEEL IN ACCORDANCE WITH AISC 303 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" AND AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS". AND AISC 341 "SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS".
B. STRUCTURAL STEEL IS AS FOLLOWS, UNLESS NOTED OTHERWISE:
   1. WIDE FLANGE AND WT-SHAPES: ASTM A992 — F_Y = 50 KSI
   2. STEEL PIPE: ASTM A53, GRADE B — F_Y = 35 KSI
   3. RECTANGULAR AND SQUARE HSS: ASTM A500, GRADE C **[B]** — F_Y = 50 KSI **[46 KSI]**
   4. ALL OTHER STRUCTURAL STEEL: ASTM A36 — F_Y = 36 KSI
   5. ANCHOR RODS: ASTM F1554, GRADE 36
   6. THREADED RODS: ASTM A36
   7. STIFFENER PLATES AND DOUBLER PLATES: ASTM A572, GRADE 50
   8. ASTM A572 GRADE 50 IS ACCEPTABLE AS A SUBSTITUTE FOR A992.
C. CENTER COLUMNS AND BEAMS ON GRID LINES UNLESS NOTED OTHERWISE.
D. CONNECTIONS:
   DESIGN ALL STEEL CONNECTIONS [NOT COMPLETELY DETAILED ON THESE DRAWINGS FOR THE FACTORED LOAD AND RESISTANCE FACTOR DESIGN (LRFD) FORCES INDICATED. SUBMIT CONNECTION CALCULATIONS AND DETAILS SEALED BY A REGISTERED PROFESSIONAL ENGINEER. CONNECTION ENGINEER MUST REVIEW STEEL SHOP DRAWINGS FOR CONNECTION SCOPE ITEMS AND SUBMIT A SEALED LETTER SUMMARIZING THE REVIEW PER AISC 303. SHOP DRAWINGS CONTAINING CONNECTIONS FOR WHICH CALCULATIONS HAVE NOT BEEN RECEIVED WILL BE RETURNED AS AN INCOMPLETE SUBMITTAL. CONNECTION ECCENTRICITY MUST BE TAKEN INTO ACCOUNT WHEN DESIGNING AND DETAILING THE CONNECTIONS. UNLESS NOTED OTHERWISE.
E. BOLT CONNECTIONS (UNLESS OTHERWISE NOTED OR REQUIRED):
   1. BOLTS: ASTM F3125, GRADE A325
   2. WASHERS: ASTM F436, TYPE 1
   3. NUTS: ASTM A563, GRADE DH
   4. CONNECT A MINIMUM OF ONE-HALF (1/2) THE DEPTH OF THE MEMBER.
   5. UNLESS NOTED OTHERWISE, BOLTS MAY BE TIGHTENED TO THE "SNUG TIGHT" CONDITION IN LIEU OF PRETENSIONING, USE SLIP-CRITICAL CONNECTIONS FOR ALL BOLTED MOMENT CONNECTIONS AND BRACE CONNECTIONS. USE BEARING CONNECTIONS WITH THREADS INCLUDED FOR ALL OTHER CONNECTIONS.
   6. PRETENSION ANCHOR RODS AT LATERAL-FORCE-RESISTING-SYSTEM COLUMNS (BRACED FRAMES, MOMENT FRAMES, ETC.)
   7. CENTER BOLT IN SLOTTED HOLES.
F. WELD CONNECTIONS (UNLESS NOTED OTHERWISE):
   1. WELDING IN ACCORDANCE WITH AWS D1.1, "STRUCTURAL WELDING CODE - STEEL".
   2. USE E70XX (SMAW), F7XX-EXXX (SAW), ER70S-X (GMAW), OR E7XT-X (FCAW) ELECTRODES FOR WELDING, UNLESS NOTED OTHERWISE.
   3. SHOW ALL FIELD WELDS REQUIRED ON ERECTION DRAWINGS.
   4. USE CONTINUOUS 1/4" FILLET WELDS UNLESS NOTED OTHERWISE.
G. BEAR STEEL BEAMS ON MASONRY AND CONCRETE A MINIMUM OF 8 INCHES, UNLESS NOTED OTHERWISE.
H. CUTS INDICATED ON THE DRAWINGS, OR AS REQUIRED FOR OTHER TRADES, MUST BE MADE IN THE SHOP AND SHOWN ON THE SHOP DRAWINGS. FIELD PERFORMED HOLES OR CUTS ARE NOT PERMITTED WITHOUT ENGINEER APPROVAL.
I. INSTALL NONMETALLIC SHRINKAGE-RESISTANT GROUT BELOW BASE PLATES, IN ACCORDANCE WITH ASTM C1107 AND A MINIMUM STRENGTH OF 6,000 PSI.
J. FABRICATE STRUCTURAL STEEL WITH ONE COAT OF SHOP PRIMER EXCEPT THE FOLLOWING MEMBERS: GALVANIZED SURFACES, SLIP-CRITICAL SURFACES, SURFACES TO BE FIELD WELDED, SURFACES TO RECEIVE FIRE PROOFING, OR UNLESS NOTED OTHERWISE. COORDINATE AREAS TO BE FIREPROOFED WITH ARCHITECTURAL DRAWINGS PRIOR TO FABRICATION.
K. GALVANIZED STRUCTURAL STEEL: ASTM A123 OR ASTM A153. GALVANIZE AFTER FABRICATION. GALVANIZE ALL EXTERIOR EXPOSED STEEL, UNLESS NOTED OTHERWISE. REPAIR DAMAGED GALVANIZED COATINGS IN ACCORDANCE WITH ASTM A780.
L. UNLESS NOTED OTHERWISE, THE TOP OF ALL STEEL COLUMNS ARE FABRICATED WITH A STEEL CAP PLATE - MINIMUM CAP PLATE DIMENSIONS MATCH COLUMN WIDTH AND DEPTH, AND MINIMUM THICKNESS OF CAP PLATE EQUALS COLUMN WEB THICKNESS (1/2" MINIMUM).
M. COORDINATE THE EXACT LOCATION AND SIZE OF ALL OPENINGS FOR MECHANICAL EQUIPMENT WITH THE MECHANICAL CONTRACTOR PRIOR TO FABRICATION.
N. REFERENCE THE ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS FOR ADDITIONAL STEEL (IF ANY) NOT INDICATED ON THE STRUCTURAL DRAWINGS.

OPEN WEB STEEL JOISTS AND JOIST GIRDERS

A. DESIGN, FABRICATE, AND ERECT STEEL JOISTS, JOIST GIRDERS, AND BRIDGING PER THE STEEL JOIST INSTITUTE (SJI). DESIGN FOR WIND UPLIFT AS INDICATED IN THE 'DESIGN WIND PRESSURE TABLE PER ASCE 7 = XX PSF. UPLIFT FORCES ON JOISTS SHALL BE RESISTED BY A SERVICE DL OF 12 PSF, UNO.
B. THE SJI LOAD TABLES ARE THE MINIMUM DESIGN LOADINGS FOR JOISTS AND JOIST GIRDERS. DESIGN JOISTS AND JOIST GIRDERS TO SUPPORT ALL OTHER LOADINGS INDICATED ON THE DRAWINGS.
C. BRIDGING INDICATED (IF ANY) IS FOR SCHEMATIC PURPOSES ONLY. GREATER OR FEWER LINES OF BRIDGING MAY BE REQUIRED BY SJI, AND WILL SUPERSEDE THE CONTRACT DOCUMENTS. DETAIL AND FABRICATE BRIDGING ACCORDING TO SJI SPECIFICATIONS. INSTALL ADDITIONAL ERECTION BRACING FOR JOISTS AND JOIST GIRDERS AS REQUIRED FOR STABILITY. SEE PLANS AND DETAILS FOR ANY SPECIAL BRIDGING AND BRACING REQUIREMENTS.
D. CAMBER JOISTS PER SJI SPECIFICATIONS.
E. DO NOT EXCEED MAXIMUM JOIST SPACING INDICATED. COORDINATE JOIST PLACEMENT WITH PARTITIONS AND WORK OF OTHER TRADES TO AVOID INTERFERENCES.
F. REFERENCE THE ARCHITECTURAL DRAWINGS FOR JOIST BOTTOM CHORD EXTENSIONS FOR CEILING SUPPORT (AS REQUIRED).
G. COMPLY WITH AWS STANDARDS AND SJI SPECIFICATIONS FOR JOIST WELDS. BOLTS ARE ASTM A325.
H. DESIGN STEEL JOISTS, JOIST GIRDERS, BRIDGING, AND CONNECTIONS FOR ALL LOADS AND CONDITIONS INDICATED.
I. DO NOT WELD JOIST OR JOIST GIRDER BOTTOM CHORD EXTENSIONS TO STABILIZER PLATES AT COLUMNS, UNLESS NOTED OTHERWISE.
J. WHERE DOUBLE JOISTS BEAR ON MASONRY OR CONCRETE WALLS, USE BEARING PLATES OF TWICE THE TYPICAL WIDTH. DO NOT RELOCATE JOISTS INDICATED AT THE CENTER LINE OF THE MASONRY/CONCRETE WALLS UNLESS APPROVED BY THE ENGINEER.
K. REINFORCE JOISTS PER DETAIL "STEEL JOIST REINFORCEMENT FOR CONCENTRATED LOADS".
L. COORDINATE LOCATIONS OF MECHANICAL EQUIPMENT, ROOF OPENINGS, AND OTHER APPURTENANCES PRIOR TO FABRICATION.
M. UNLESS NOTED OTHERWISE, JOIST SEAT DEPTHS ARE AS FOLLOWS; SUBMIT ANY DEVIATIONS FOR APPROVAL, PRIOR TO SHOP DRAWING SUBMITTAL:
   1. K AND KCS JOISTS:
      a. LESS THAN OR EQUAL TO 1/4:12 SLOPE: 2-1/2 INCHES
      b. GREATER THAN 1/4:12 SLOPE: PER SJI

STEEL DECKING

A. DESIGN, FABRICATE, AND ERECT STEEL DECKING PER THE STEEL DECK INSTITUTE (SDI) "DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS, AND ROOF DECKS" AND AISI "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".
B. SPAN DECK PERPENDICULAR TO THE SUPPORTS WITH A MINIMUM OF THREE SPAN CONTINUOUS, UNLESS NOTED OTHERWISE. ATTACH STEEL DECK TO ALL SUPPORTS INDICATED.
C. INSTALL STEEL DECK WITH SUFFICIENT BEARING AT END AND INTERMEDIATE SUPPORTS TO PREVENT WEB CRIPPLING (1 1/2" AND 3", RESPECTIVELY).
D. SHORE DECKING AS REQUIRED WITH MINIMUM SHORING BEAM WIDTHS PER THE DECK MANUFACTURER'S RECOMMENDATIONS.
E. COORDINATE LOCATIONS AND DETAILS OF MECHANICAL EQUIPMENT, DECK OPENING SLEEVES, INSERTS, ETC. PRIOR TO FABRICATION. ADD STEEL SUPPORTS ON ALL SIDES OF DECK OPENINGS MEASURING GREATER THAN 12" ON ANY SIDE. SPAN SUPPORTS BETWEEN ADJACENT BEAMS OR JOISTS ON TWO SIDES. UNLESS OTHERWISE NOTED OR DETAILED ON THE DRAWINGS, USE L5x5x5/16 ANGLES FOR SPANS EXCEEDING 6'-0" AND L3x3x1/4 ANGLES FOR SPANS LESS THAN 6'-0". COORDINATE OPENING SIZES, LOCATIONS, AND DETAILS WITH THE ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS.
F. INSTALL CELL CLOSURES, COLUMN CLOSURES, FINISH STRIPS, GIRDER FILLERS, POUR STOPS, AND ATTACHMENTS AS REQUIRED TO ACHIEVE A COMPLETE SYSTEM. UNLESS NOTED OTHERWISE, INSTALL POUR STOPS OF LENGTH, DEPTH, AND GAGE APPROPRIATE FOR OVERHANG AND SLAB DEPTH INDICATED.
G. WELD L3x3x3/16 STEEL ANGLES TO FACE OF COLUMNS TO SUPPORT FLOOR DECK WHERE DECKING IS CUT AROUND COLUMNS.
H. COMPLY WITH AWS D1.3 "STRUCTURAL WELDING CODE - SHEET STEEL" FOR DECK WELDS. USE WELDING WASHERS FOR DECK THINNER THAN 0.028 INCHES.
I. SUBMIT SEALED CALCULATIONS FOR ALTERNATE DECK FASTENERS PRIOR TO FABRICATION.
J. DO NOT CAST CONDUIT IN ELEVATED SLABS WITHOUT ENGINEER APPROVAL. CONDUIT OUTER DIAMETER CANNOT EXCEED 1/3 OF THE SLAB THICKNESS ABOVE THE DECK.
K. ROOF DECK:
   1. MINIMUM SECTION PROPERTIES:

ROOF DECK SCHEDULE

DECK TYPE	DESIGN THICKNESS (IN)	FINISH	SECTION PROPERTIES				
			Ip^4	Sp (IN³/FT)	In^4	Sn (IN³/FT)	Fy (KSI)
1.5B22	0.0358	GALV G60	0.155	0.186	0.183	0.192	33

   2. DECK ATTACHMENT:
      1. SIMPSON STRONG-TIE STRONG-DRIVE #12-24 XL SCREW IN 36/**[XX]** PATTERN AT SUPPORTS*
      2. SIMPSON STRONG-TIE STRONG-DRIVE #12-24 XL SCREW AT **[XX]"** OC AT DIAPHRAGM BOUNDARIES*
   3. **[XX]**-#10 SIDELAP SCREWS PER DECK SPAN
      * 5/8" DIA PUDDLE WELD WHERE SUPPORT MEMBER THICKNESS EXCEEDS 3/8".

COLD-FORMED STEEL FRAMING

A. COLD-FORMED STEEL FRAMING FOR THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AMERICAN IRON AND STEEL INSTITUTE "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" (AISI S100).
   DETAIL MEMBERS AND CONNECTIONS FOR ALL FRAMING CONDITIONS, INCLUDING WALLS, CORNERS, HEADERS, AND JAMBS. SOME CONDITIONS MAY REQUIRE MODIFICATION OF COLD-FORMED FRAMING MEMBERS (SUCH AS NOTCHING OR REVISING SIZES) OR MULTIPLE STUDS TO SUPPORT INCREASED LOADS. CONTRACTOR COORDINATE ALL CONDITIONS, CONNECTIONS, AND DETAILS.
B. FABRICATION AND INSTALLATION MUST BE IN ACCORDANCE WITH AISI "SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" AND MANUFACTURER INSTRUCTIONS. INSTALL MANUFACTURER'S RECOMMENDED STANDARD TRACK, CLIP ANGLES, BRACING, REINFORCEMENTS, FASTENERS, AND ACCESSORIES FOR THE APPLICATIONS INDICATED AND AS NEEDED FOR A COMPLETE FRAMING SYSTEM. TEMPORARY (CONSTRUCTION) BRACING OF FRAMING MEMBERS (PRIOR TO SHEATHING INSTALLATION) IS BY THE CONTRACTOR PER AISI AND MANUFACTURER RECOMMENDATIONS.
C. COLD-FORMED STEEL MATERIAL: ASTM A1003 STEEL SHEET WITH G60 GALV COATING CONFORMING TO ASTM A653, WITH A MINIMUM YIELD STRENGTH OF 33 KSI (USE 50 KSI FOR 54 MILS AND THICKER) UNLESS NOTED OTHERWISE.
D. MEMBER SIZES INDICATED ARE PER THE "STEEL STUD MANUFACTURERS ASSOCIATION" (SSMA). COMPONENTS SHOWN ARE STRUCTURAL MEMBERS (33 MIL OR THICKER), UNLESS NOTED OTHERWISE. NON-STRUCTURAL MEMBERS AND DRYWALL GAGES ARE NOT PERMITTED.
E. SCREWS ARE NON-CORROSIVE NO 8-18 (DIA=0.125") OR LARGER, UNLESS NOTED OTHERWISE. DO NOT USE STAINLESS STEEL OR COPPER-COATED FASTENERS.
F. WELDING: AWS D1.3 "STRUCTURAL WELDING CODE-SHEET STEEL". CONSULT MANUFACTURER FOR EQUIPMENT RECOMMENDATIONS AND PROPER ELECTRODE SELECTION.
G. INSTALL MINIMUM OF THREE (3) WALL STUDS AT CORNERS AND INTERSECTING STUD WALLS (UNLESS OTHERWISE INDICATED).
H. PREPUNCHED HOLES CANNOT BE LOCATED WITHIN 10 INCHES FROM WALL STUD ENDS.
I. TRACKS ARE THE SAME DEPTH AS STUDS OR JOISTS, UNLESS NOTED OTHERWISE. CONNECT TRACKS TO STUD AND/OR JOIST SUPPORTS AT 16" OC MAXIMUM, ON EACH SIDE. ALIGN WALL STUD FRAMING WITH SUPPORTED STUD/JOIST MEMBERS ABOVE.
J. DO NOT SPLICE MEMBERS UNLESS OTHERWISE INDICATED. FASTEN MULTI-PLY MEMBERS TOGETHER USING TACK WELDS OR #10 SCREWS AT 12" OC MAXIMUM SPACING, UNLESS NOTED OTHERWISE.
K. CROSS BRIDGING OR FULL-DEPTH BLOCKING IS REQUIRED AT AND ROOF JOISTS/RAFTERS NOT RECEIVING CEILING SHEATHING AND AT WALL STUDS NOT RECEIVING SHEATHING ON BOTH FACES. UNLESS NOTED OTHERWISE, MAXIMUM BRIDGING/BLOCKING SPACING IS 6'-0" OC OR AT 1/3 POINTS OF MEMBER SPAN, WHICHEVER IS LESS. COORDINATE EXTENTS OF WALL AND CEILING SHEATHING WITH THE ARCHITECTURAL DRAWINGS.
L. CLADDING, PARTITION FRAMING, AND CONNECTIONS MUST ACCOMMODATE VERTICAL AND LATERAL DISPLACEMENT OF THE PRIMARY STRUCTURE. COMPLY WITH SSMA TECHNICAL NOTE NO 1 DATED JANUARY 2000 FOR SLIP TRACK DESIGN.
M. REPAIR DAMAGED GALVANIZED COATINGS AND WELDED AREAS IN ACCORDANCE WITH ASTM A780.

SPECIAL INSPECTIONS AND TESTING

A. SPECIAL INSPECTIONS AND TESTING ARE PERFORMED IN ACCORDANCE WITH IBC CHAPTER 17 AND LOCAL JURISDICTION PROVISIONS, BY AN INDEPENDENT INSPECTION AND TESTING AGENCY. THE SPECIAL INSPECTOR MUST OBSERVE AND TEST THE WORK FOR CONFORMANCE TO THE CONTRACT DOCUMENTS.
B. THE SPECIAL INSPECTOR MUST FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, THE ENGINEER OR ARCHITECT OF RECORD, AND ALL OTHER DESIGNATED INDIVIDUALS. ALL DISCREPANCIES MUST BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF NOT CORRECTED, TO THE PROPER DESIGN AUTHORITY AND THE BUILDING OFFICIAL.
C. THE SPECIAL INSPECTOR MUST SUBMIT A FINAL SIGNED REPORT STATING WHETHER THE WORK IS, TO THE BEST OF THE INSPECTOR'S KNOWLEDGE, IN CONFORMANCE WITH THE CONTRACT DOCUMENTS, SOILS REPORT, AND APPLICABLE WORKMANSHIP OF THE BUILDING CODE.

SUBMITTALS

A. CONTRACTOR MUST REVIEW AND STAMP ALL SHOP DRAWINGS BEFORE SUBMITTING FOR REVIEW. SUBMIT SHOP DRAWINGS TO THE ARCHITECT AND/OR ENGINEER FOR REVIEW. FABRICATE AND CONSTRUCT FROM THE REVIEWED SUBMITTALS. ALLOW 10 BUSINESS DAYS FOR EACH SUBMITTAL REVIEW UNLESS AN ALTERNATE REVIEW TIME IS AGREED UPON BY ALL PARTIES. IN THE EVENT MULTIPLE SUBMITTALS ARE SUBMITTED AT THE SAME TIME, THE CONTRACTOR MUST INDICATE WHICH SUBMITTALS HAVE PRIORITY.
B. MAINTAIN A RECORD SET OF APPROVED SHOP DRAWINGS IN THE FIELD.
C. SUBMIT IN WRITING ANY DEVIATION FROM, ADDITION TO, SUBSTITUTION FOR, OR MODIFICATION TO, THE STRUCTURE OR ANY PART OF THE STRUCTURE DETAILED, TO THE ENGINEER FOR REVIEW. SHOP DRAWINGS SUBMITTED FOR REVIEW DO NOT CONSTITUTE "IN-WRITING" UNLESS IT IS CLEARLY NOTED SPECIFIC CHANGES ARE BEING REQUESTED.
D. PREPARE A LIST AND SCHEDULE OF ALL STRUCTURAL SUBMITTALS PRIOR TO CONSTRUCTION.
E. SUBMIT THE FOLLOWING SHOP DRAWINGS FOR THE ENGINEER'S REVIEW:
   1. CONCRETE MIX DESIGNS
   2. REINFORCING STEEL
   3. STEEL JOISTS AND JOIST GIRDERS (1, 3)
   4. MISCELLANEOUS STEEL
   5. METAL AND FABRIC CANOPIES - CONNECTION TO BUILDING IS BY SUPPLIER (1, 3)
   6. STRUCTURAL STEEL, SHOP, AND ERECTION DRAWINGS (1, 3)
   7. STEEL DECKING
   8. EMBEDDED ITEMS (PLATES, ANGLES, BOLTS, ETC.) OR ITEMS ATTACHED TO THE STRUCTURAL FRAME FOR BUILDING CLADDING ATTACHMENT OR FOR ATTACHMENT OF OTHER ITEMS (2)
F. SUBMIT ITEMS MARKED (1) SEALED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED. SUBMIT ITEMS MARKED (2) FOR OWNER'S RECORD ONLY, AND WILL NOT HAVE THE ENGINEER'S SHOP DRAWING STAMP AFFIXED. SUBMIT ITEMS MARKED (3) WITH DESIGN CALCULATIONS SEALED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED.
   1. THE OMISSION FROM THE SHOP DRAWINGS OF ANY MATERIALS REQUIRED BY THE CONTRACT DOCUMENTS DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY OF FURNISHING AND INSTALLING SUCH MATERIALS, REGARDLESS OF WHETHER THE SHOP DRAWINGS HAVE BEEN REVIEWED AND APPROVED.
G. THE USE OF ELECTRONIC FILES OR REPRODUCTIONS OF CONTRACT DOCUMENTS BY ANY CONTRACTOR, SUBCONTRACTOR, ERECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFIES ACCEPTANCE OF ALL INFORMATION SHOWN HEREON AS CORRECT, AND OBLIGATES THEM TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS THAT MAY OCCUR HEREON.

BRITT, PETERS AND ASSOCIATES INC
consulting engineers
101 Falls Park Drive
Suite 601
Greenville, SC 29601
(864) 271-8869
www.brittpeters.com
BPA Project #: 240281

Seal

SOUTH CAROLINA LICENSED PROFESSIONAL ENGINEER No. 35067 ROBERT OWEN GERVIN
08/06/2024

SOUTH CAROLINA Britt, Peters & Associates, Inc. No. C00781 CERTIFICATE OF AUTHORIZATION

DP3 ARCHITECTS
DP3 Architects, Ltd.
15 South Main Street, Suite 400
Greenville, SC 29601
864.232.8200
www.DP3architects.com

Project

MARIACHIS
RESTAURANT
LANCASTER, SC

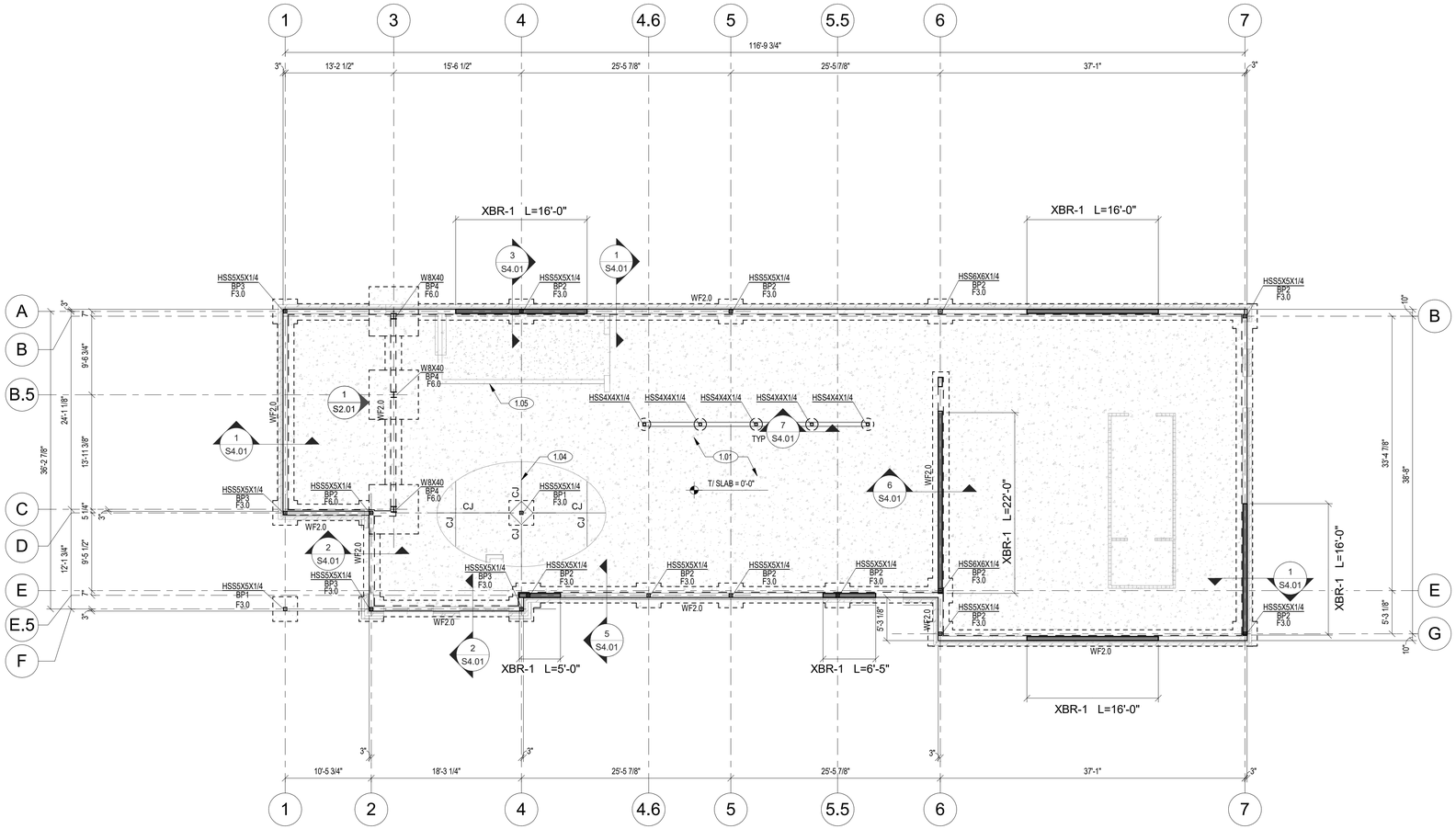
Project Number	23213
Drawn By	BC
Checked By	ROG
Date	05/AUG/24

Revisions

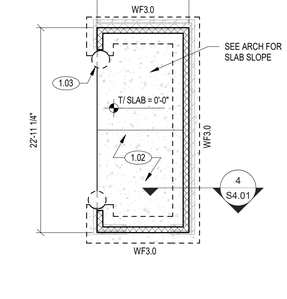
Drawing

GENERAL NOTES

S0.01



1 FOUNDATION
1/8" = 1'-0"



2 TRASH ENCLOSURE FOUNDATION
1/8" = 1'-0"

FOUNDATION PLAN NOTES
1. REF PLAN FOR TOP OF SLAB ELEVATION (T/ SLAB). COORD W/ ARCH AND CIVIL.
2. TOP OF EXTERIOR FOOTING (T/ FTG) = -1'-4" BELOW FINISHED FLOOR, TYPICAL UNO.
3. WALL CONSTRUCTION: 600S162-43 @ 16" OC TYP, UNO

FOUNDATION PLAN LEGEND
- (#.##) DENOTES SHEET NOTE, REF SCHEDULE THIS SHEET
- F#x# DENOTES FOOTING (F), REF SCHEDULE ON SHEET ____
- WF#.# & TS#.# DENOTES WALL FOOTING (WF) OR THICKENED SLAB (TS), REF SCHEDULE THIS SHEET
- BP# DENOTES COLUMN BASE PLATE, REF SCHEDULE ON SHEET ____
- T/ FTG = X'-X" DENOTES TOP OF FOOTING (T/ FTG)
- DENOTES 8" CMU WALL, REINF W/ #5 @ 48" OC, TYP, UNO
- FD DENOTES FLOOR DRAIN, SLOPE SLAB TOWARDS ALL FD (COORD W/ ARCH)
- DENOTES STEP FOOTING, REF TYPICAL DETAILS. GC TO COORDINATE LOCATIONS NOT DIMENSIONED ON PLAN IN FIELD WITH FINAL GRADING PLAN, ARCHITECTURAL ELEVATIONS, AND MEP DRAWINGS
- CJ DENOTES SLAB CONTROL OR CONSTRUCTION JOINT, REF TYPICAL DETAILS
- XBR-1 DENOTES X-STRAP WALL, SEE 13/S3.01

SHEET NOTE SCHEDULE - FOUNDATION PLAN (#.##)

REF PLANS AND DETAILS FOR SHEET NOTES REQUIRED, NOT ALL NOTES APPLICABLE TO THIS SHEET

MARK	DESCRIPTION
1.01	4" CONCRETE SLAB REINF W/ 6x6-W1.4xW1.4 WWR ON 10 MIL VAPOR RETARDER ON 4" GRANULAR BASE ON PREPARED SUBGRADE
1.02	6" CONCRETE SLAB REINF W/ 6x6 - W1.4xW1.4 WWR ON 10 MIL VAPOR RETARDER ON 4" GRANULAR BASE ON PREPARED SUBGRADE
1.03	24" DIA x 4'-0" DEEP SONOTUBE W/ (4) #5 VERT AND #3 TIES AT 2" OC TYP AT GATE POST
1.04	PROVIDE CONTROL JOINTS SPACED NO MORE THAN 12'-0" OC WITH AN ASPECT RATIO OF NO MORE THAN 1.25:1
1.05	CONCRETE CURB. REF. TO S3.02

FOUNDATION (F) SCHEDULE

MARK	WIDTH "W"	LENGTH "L"	THICKNESS "T"	REINFORCING BOTTOM BARS LONG	BOTTOM BARS SHORT	TOP BARS LONG	TOP BARS SHORT	REMARKS
F3.0	3'-0"	3'-0"	1'-0"	(3) #4	(3) #4	--	--	
F6.0	6'-0"	6'-0"	2'-0"	(6) #5	(6) #5	(6) #5	(6) #5	

WALL FOOTING (WF) SCHEDULE

MARK	DIMENSIONS WIDTH "W"	DIMENSIONS THICKNESS "T"	REINFORCING BOTTOM BARS LONG	BOTTOM BARS SHORT	TOP BARS LONG	TOP BARS SHORT	REMARKS
WF2.0	2'-0"	1'-0"	(2) #5	#4 @ 18" OC	--	--	
WF3.0	3'-0"	1'-4"	(3) #5	#4 @ 18" OC	--	--	

BRITT, PETERS AND ASSOCIATES INC consulting engineers
101 Falls Park Drive
Suite 601
Greenville, SC 29601
(864) 271-8869
www.brittpeters.com
BPA Project #: 240281

Seal

SOUTH CAROLINA LICENSED PROFESSIONAL ENGINEER ROBERT OWEN GERVIN No. 35067
08/06/2024

SOUTH CAROLINA Britt, Peters & Associates, Inc. No. C00781 CERTIFICATE OF AUTHORIZATION

DP3 ARCHITECTS
DP3 Architects, Ltd.
15 South Main Street, Suite 400
Greenville, SC 29601
864.232.8200
www.DP3architects.com

Project

MARIACHIS RESTAURANT
LANCASTER, SC

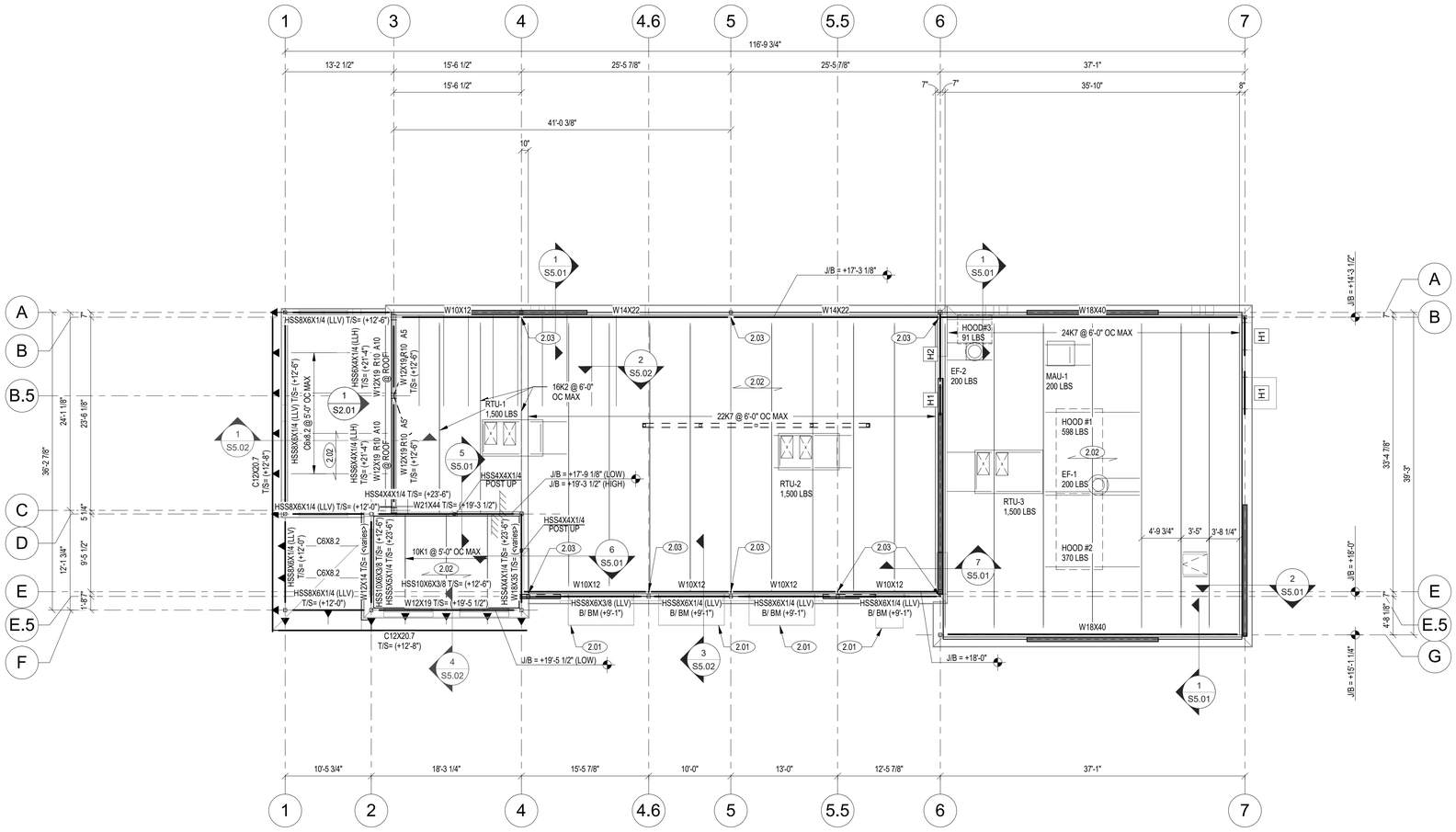
Project Number	23213
Drawn By	BC
Checked By	ROG
Date	05/AUG/24

Revisions

Drawing

FOUNDATION PLAN

S1.01



1 ROOF FRAMING

1/8" = 1'-0"

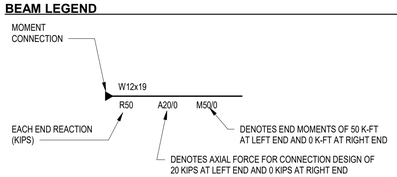
ROOF FRAMING PLAN NOTES

1. REF PLAN FOR JOIST BEARING (J/B). COORD W/ ARCH.
2. REF TO S3.04 FOR LOOSE LINTEL

ROOF FRAMING PLAN LEGEND

#.##	DENOTES SHEET NOTE, REF SCHEDULE THIS SHEET
	DENOTES DECK SPAN DIRECTION
H#	DENOTES HEADER REF TO S3.01
T/S	DENOTES TOP OF STEEL
B/BM	DENOTES BOTTOM OF BEAM
J/B	DENOTES JOIST BEARING

BEAM LEGEND



SHEET NOTE SCHEDULE - FRAMING PLANS (#.##)

REF PLANS AND DETAILS FOR SHEET NOTES REQUIRED, NOT ALL NOTES APPLICABLE TO THIS SHEET

MARK	DESCRIPTION
2.01	DENOTES ALUMINUM AWNING BY MANUFACTURER
2.02	1.5" TYPE B, 22 GA GALV. METAL ROOF DECK
2.03	BRACKET, REF 4/S5.01

BRITT, PETERS AND ASSOCIATES INC consulting engineers
101 Falls Park Drive
Suite 601
Greenville, SC 29601
(864) 271-8869
www.brittpeters.com
BPA Project #: 240281

Seal

SOUTH CAROLINA LICENSED PROFESSIONAL ENGINEER No. 35067 ROBERT OWEN GERVIN

08/06/2024

SOUTH CAROLINA Britt, Peters & Associates, Inc. No. C00781 CERTIFICATE OF AUTHORIZATION

DP3 ARCHITECTS
DP3 Architects, Ltd.
15 South Main Street, Suite 400
Greenville, SC 29601
864.232.8200
www.DP3architects.com

Project

MARIACHIS
RESTAURANT
LANCASTER, SC

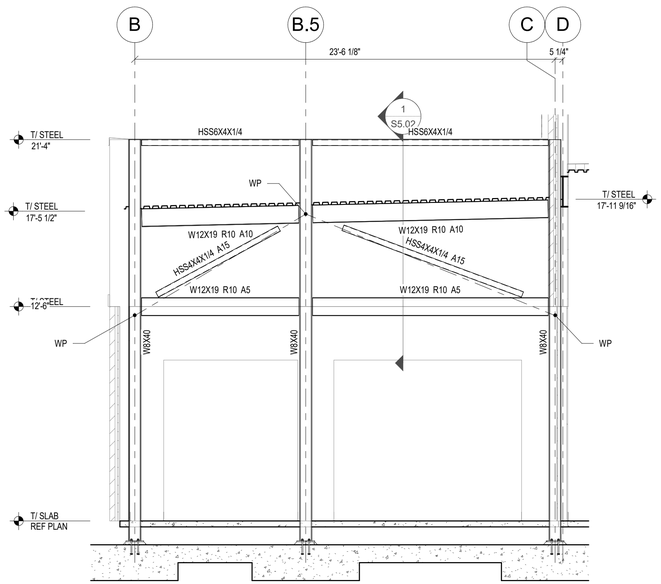
Project Number	23213
Drawn By	BC
Checked By	ROG
Date	05/AUG/24

Revisions

Drawing

ROFF FRAMING PLAN

S1.02



1 ELEVATION

1/4" = 1'-0"

BRITT, PETERS AND ASSOCIATES INC consulting engineers
101 Falls Park Drive
Suite 601
Greenville, SC 29601
(864) 271-8869
www.brittpeters.com
BPA Project #: 240281

Seal

SOUTH CAROLINA LICENSED PROFESSIONAL ENGINEER No. 35067 ROBERT OWEN GERVIN

08/06/2024

SOUTH CAROLINA Britt, Peters & Associates, Inc. No. C00781 CERTIFICATE OF AUTHORIZATION

DP3 ARCHITECTS
DP3 Architects, Ltd.
15 South Main Street, Suite 400
Greenville, SC 29601
864.232.8200
www.DP3architects.com

Project

MARIACHIS
RESTAURANT
LANCASTER, SC

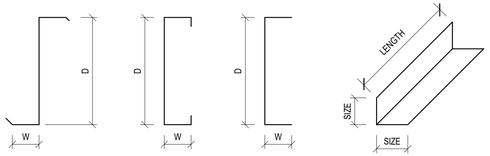
Project Number	23213
Drawn By	BC
Checked By	ROG
Date	05/AUG/24

Revisions

Drawing

ELEVATIONS

S2.01



ZEES CEES TRACK ANGLE

MEMBERS ARE DESIGNATED ON DETAILS AND PLANS AS FOLLOWS:

- CEES - 600S162-033 — D, S=STUDS, W — THICKNESS IN MILS
- TRACKS - 600T125-033 — D, T=TRACK, W — THICKNESS IN MILS
- ZEES - 8ZS2.25-070 — D, ZS=Z-SEC LIP, W — THICKNESS IN INCHES
- ANGLES - L3x3x16ga — SIZE — THICKNESS IN GAUGE

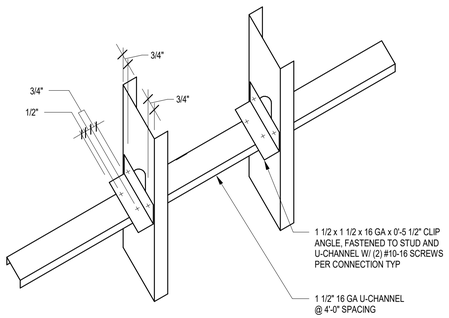
125 = 1.25" = 1 1/4"
137 = 1.37" = 1 3/8"
162 = 1.62" = 1 5/8"
200 = 2.00" = 2"
250 = 2.50" = 2 1/2"
300 = 3.00" = 3"
362 = 3.62" = 3 5/8"
400 = 4.00" = 4"
600 = 6.00" = 6"
800 = 8.00" = 8"
1000 = 10.00" = 10"
1200 = 12.00" = 12"

D = SECTION DEPTH
W = FLANGE WIDTH

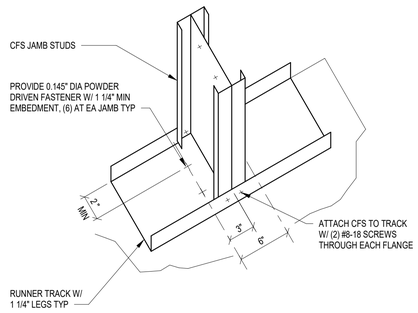
MIL - GAUGE DESIGNATIONS

MIL THICK	GAUGE
33	20 GA
43	18 GA
54	16 GA
68	14 GA
97	12 GA
118	10 GA

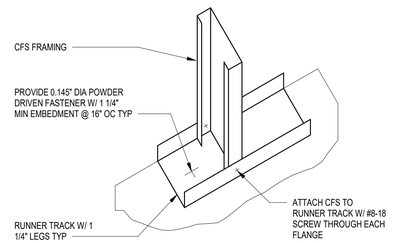
1 CFS MEMBER DESIGNATION LEGEND
3/4" = 1'-0"



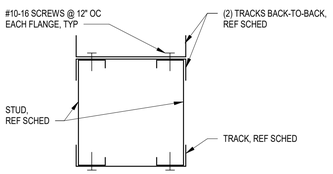
2 LATERAL BRIDGING
3/4" = 1'-0"



3 ANCHORAGE AT JAMBS
3/4" = 1'-0"



4 RUNNER TRACK ANCHORAGE
3/4" = 1'-0"

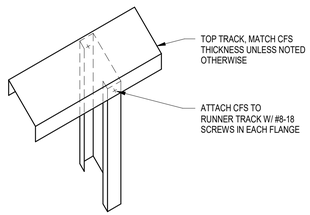


5 BOXED HEADER DETAIL
3" = 1'-0"

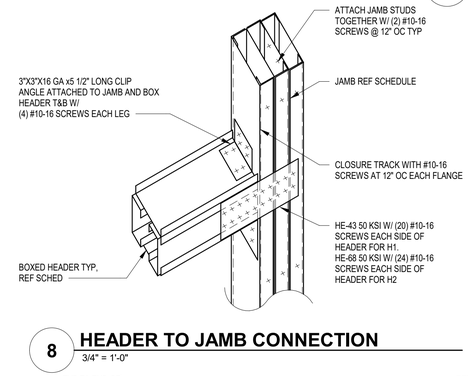
HEADER SCHEDULE

SPAN	HEADER	JAMB	SILL
H1	2 HORIZ. TRACKS 600T125-54 50KSI 2 VERT. STUDS 600S162-54 50KSI W/ STIFFCLIP HE43-50 CONNECTION	2 VERT. STUDS 600S162-54 50KSI 2 HORIZ. STUDS 600S162-54 50KSI W/ VERTICLIP SLB EA SIDE AT TOP AND STIFFCLIP AL EA SIDE AT BOTTOM	1 HORIZ. STUD 600S162-54 1 HORIZ. TRACK 600T125-54
H2	3 VERT. TRACKS 1200T150-97 50KSI 3 VERT. STUDS 1200S162-97 50KSI 2 HORIZ. TRACKS 600T125-54 50KSI W/ STIFFCLIP HE68-50 CONNECTION	3 VERT. TRACKS 600T150-54 50KSI 3 VERT. STUDS 600S162-54 50KSI W/ VERTICLIP SLB EA SIDE AT TOP AND STIFFCLIP AL EA SIDE AT BOTTOM	1 HORIZ. STUD 600S162-54 1 HORIZ. TRACK 600T125-54

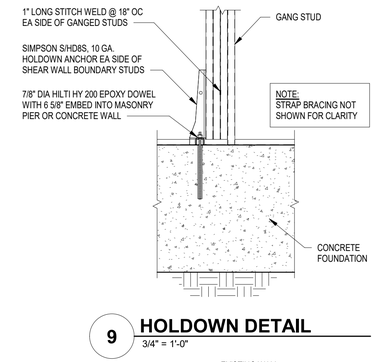
6 HEADER SCHEDULE
3/4" = 1'-0"



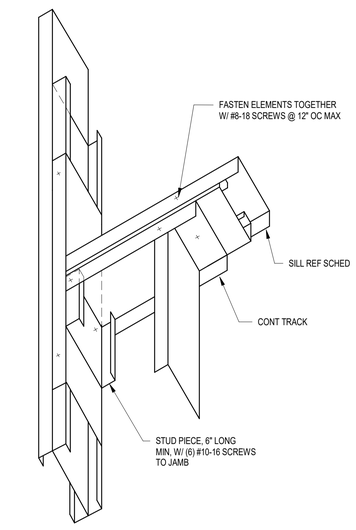
7 STUD TO TOP TRACK CONNECTION
3/4" = 1'-0"



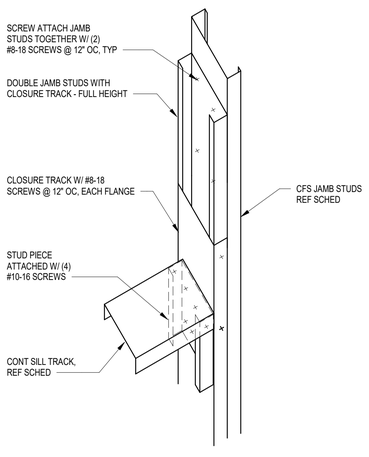
8 HEADER TO JAMB CONNECTION
3/4" = 1'-0"



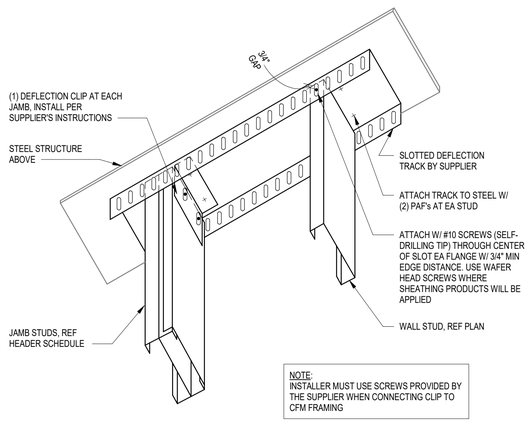
9 HOLDOWN DETAIL
3/4" = 1'-0"



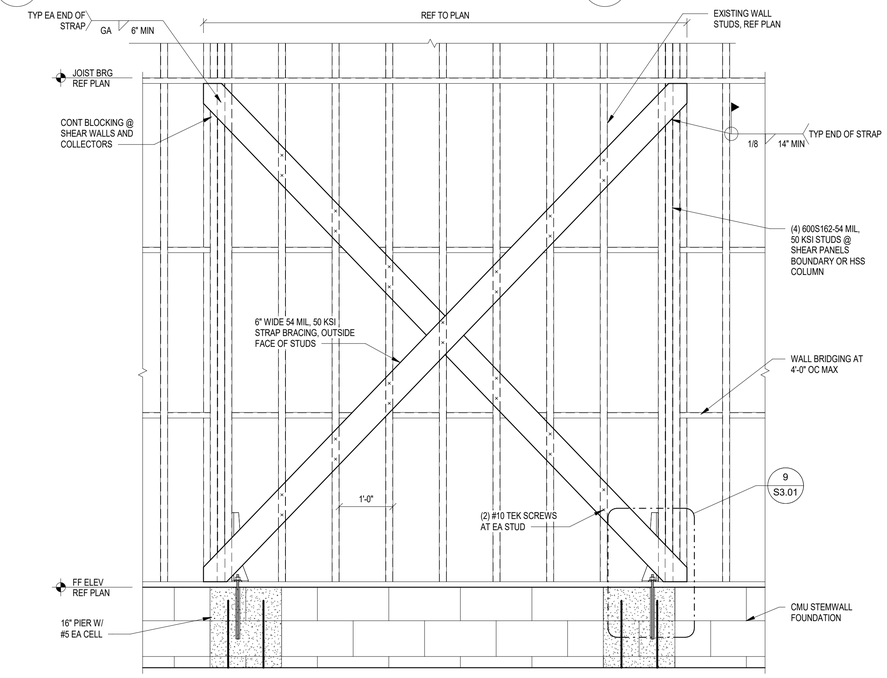
10 BUILT UP SILL
3/4" = 1'-0"



11 TYPICAL SILL TO JAMB CONNECTION
3/4" = 1'-0"



12 TYPICAL DEFLECTION TRACK CONNECTION
3/4" = 1'-0"



13 CFS STRAP BRACING ELEVATION
3/4" = 1'-0"

NOTE REGARDING REINF COVER REQUIREMENTS

ALL REINFORCING SHALL BE PLACED IN ACCORDANCE WITH THE MINIMUM COVER REQUIREMENTS PER ACI AS OUTLINED IN THE GENERAL NOTES. SPECIFIC BAR LOCATIONS SHOWN IN SECTIONS AND DETAILS MAY OVERRIDE BUT NOT VIOLATE THE MINIMUM COVER REQUIREMENTS.

BRITT, PETERS AND ASSOCIATES INC consulting engineers
101 Falls Park Drive
Suite 601
Greenville, SC 29601
(864) 271-8869
www.brittpeters.com
BPA Project #: 240281

Seal

SOUTH CAROLINA LICENSED PROFESSIONAL ENGINEER ROBERT OWEN GERVIS No. 35067
08/06/2024

SOUTH CAROLINA Britt, Peters & Associates, Inc. No. C00781 CERTIFICATE OF AUTHORIZATION

DP3 ARCHITECTS

DP3 Architects, Ltd.
15 South Main Street, Suite 400
Greenville, SC 29601
864.232.8200
www.DP3architects.com

Project

MARIACHIS
RESTAURANT
LANCASTER, SC

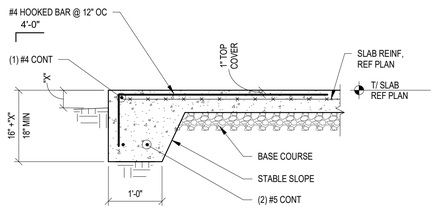
Project Number	23213
Drawn By	BC
Checked By	ROG
Date	05/AUG/24

Revisions

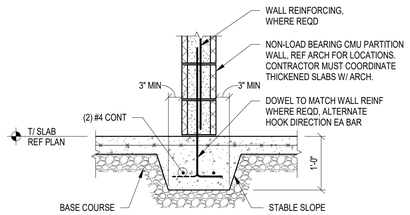
Drawing

TYPICAL COLD FORMED STEEL DETAILS

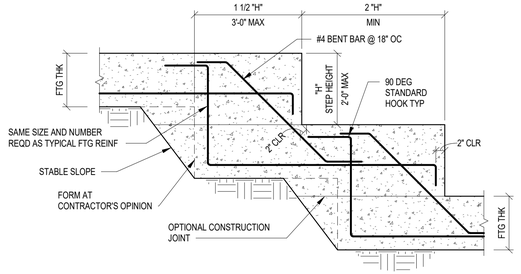
S3.01



1 TYPICAL TURNDOWN
3/4" = 1'-0"

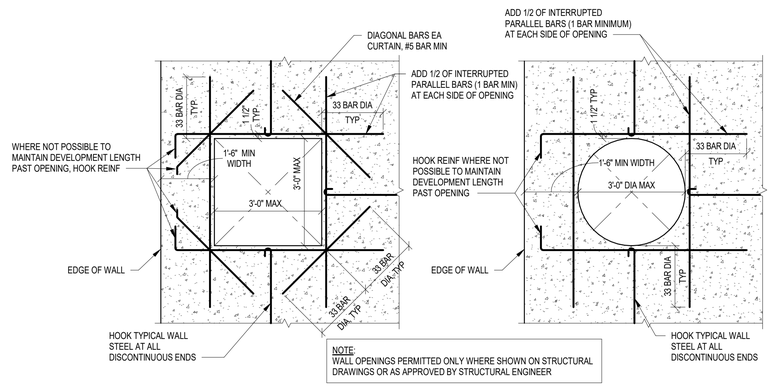


2 THICKENED SLAB AT NON-LOADBEARING INTERIOR WALLS
3/4" = 1'-0"



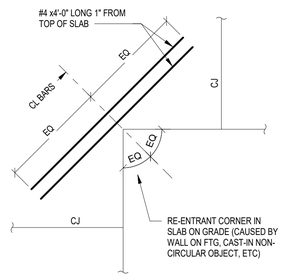
NOTE:
GC TO COORDINATE FINAL TOP OF FOOTING ELEVATIONS WITH THE ARCHITECTURAL ELEVATIONS, MEP DRAWINGS, AND CIVIL GRADING PLANS PRIOR TO PLACEMENT OF CONCRETE. FOOTING STEPS DENOTED ON PLAN ARE APPROXIMATE, UNLESS NOTED OTHERWISE, AND SHALL BE FIELD COORDINATED.

3 STEPPED FOOTING DETAIL
3/4" = 1'-0"

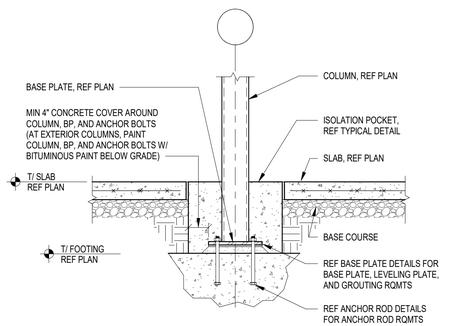


NOTE:
WALL OPENINGS PERMITTED ONLY WHERE SHOWN ON STRUCTURAL DRAWINGS OR AS APPROVED BY STRUCTURAL ENGINEER

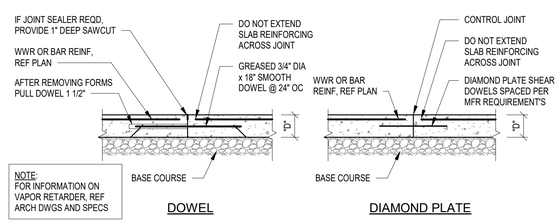
4 REINFORCING AROUND OPENINGS IN CONCRETE WALLS, UNO
1/2" = 1'-0"



5 RE-ENTRANT CORNER
3/4" = 1'-0"

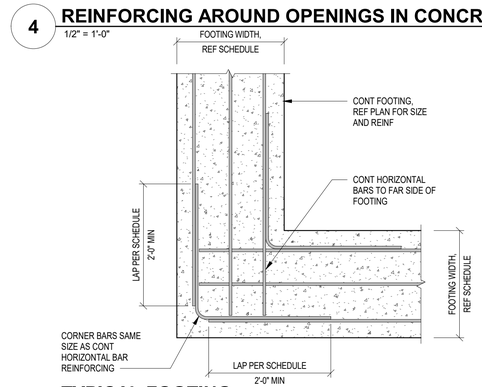


6 ISOLATION POCKET SECTION
3/4" = 1'-0"

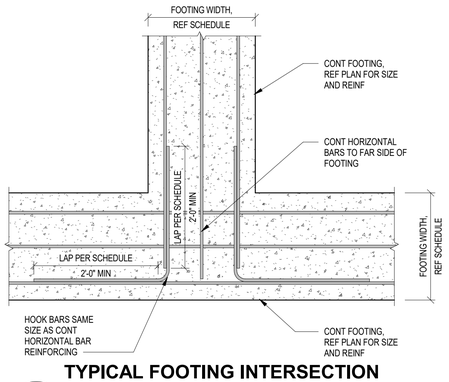


NOTE:
FOR INFORMATION ON VAPOR RETARDER, REF ARCH DWGS AND SPECS

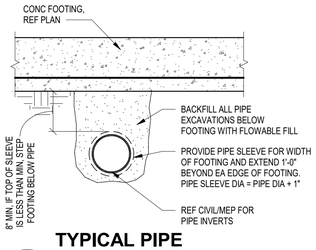
7 TYPICAL CONSTRUCTION JOINT
3/4" = 1'-0"



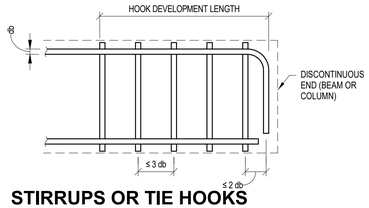
8 CORNER REINFORCING DETAIL
3/4" = 1'-0"



9 TYPICAL FOOTING INTERSECTION REINFORCING INTERSECTION
3/4" = 1'-0"



12 TYPICAL PIPE BACKFILL DETAIL AT FOOTING
3/4" = 1'-0"



13 STIRRUPS OR TIE HOOKS REQUIREMENTS AT DISCONTINUOUS END
3/4" = 1'-0"

BRITT, PETERS AND ASSOCIATES INC consulting engineers
101 Falls Park Drive
Suite 601
Greenville, SC 29601
(864) 271-8869
www.brittpeters.com
BPA Project #: 240281

Seal

SOUTH CAROLINA REGISTERED PROFESSIONAL ENGINEER No. 35067 ROBERT OWEN GERVIN
08/06/2024

SOUTH CAROLINA Britt, Peters & Associates, Inc. No. C00781 CERTIFICATE OF AUTHORIZATION

DP3 ARCHITECTS
DP3 Architects, Ltd.
15 South Main Street, Suite 400
Greenville, SC 29601
864.232.8200
www.DP3architects.com

Project

MARIACHIS
RESTAURANT
LANCASTER, SC

Project Number	23213
Drawn By	BC
Checked By	ROG
Date	05/AUG/24

Revisions

Drawing

TYPICAL CONCRETE DETAILS

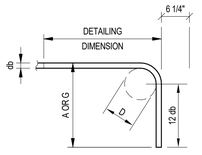
S3.02

RECOMMENDED END HOOKS					HOOK MIN DEVELOPMENT LENGTHS (IN)		
BAR SIZE	FINISHED BEND DIAMETER D (IN)	180 DEG HOOKS		90 DEG HOOKS	NORMAL WT CONCRETE		
		A OR G (IN)	J (IN)	A OR G (IN)	3000	4000	5000
#3	2 1/4	5	3	6	9	8	7
#4	3	6	4	8	11	10	9
#5	3 3/4	7	5	10	14	12	11
#6	4 1/2	8	6	12	17	15	13
#7	5 1/4	10	7	14	20	17	15
#8	6	11	8	16	22	19	17
#9	9 1/2	15	11 3/4	19	25	22	20
#10	10 3/4	17	13 1/4	22	28	25	22
#11	12	19	14 3/4	24	31	27	24

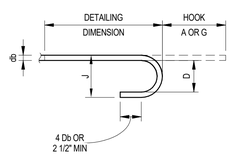
D = INSIDE BEND OF DIAMETER
1. HOOK EMBEDMENT LENGTHS IN TABLE SHALL BE FACTORED FOR THE FOLLOWING CONDITIONS:
   - LIGHTWEIGHT CONCRETE: 1.3 x TABLE LENGTH
   - EPOXY COATED BARS: 1.2 x TABLE LENGTH

STIRRUP AND TIE HOOK SCHEDULE			
BAR SIZE	D (IN)	90 DEG HOOK A OR G (IN)	135 DEG HOOK A OR G (IN)
#3	1 1/2	4	4
#4	2	4 1/2	4 1/2
#5	2 1/2	6	5 1/2

D = INSIDE BEND OF DIAMETER

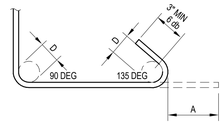


90 DEG HOOK

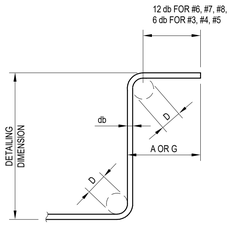


180 DEG HOOK

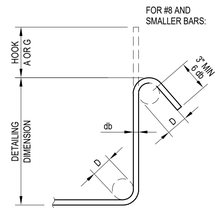
END HOOK TYPES



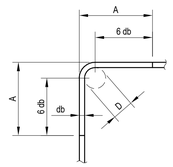
CROSS TIE



90 DEG HOOK



135 DEG HOOK



CORNER TIE HOOK

1 STIRRUP AND TIE HOOK TYPES DETAIL
3/4" = 1'-0"

REINFORCING BAR LAP LENGTH SCHEDULE (CLASS B)											
BAR	SLABS AND HORIZONTAL WALL REINF		BEAMS		FOOTINGS AND RETAINING WALLS		VERTICAL WALL REINF		COLUMNS		SPECIAL BOUNDARY ELEMENTS		BAR
	4000 PSI	5000 PSI	4000 PSI	5000 PSI	3000 PSI	4000 PSI	5000 PSI	7000 PSI	5000 PSI	7000 PSI	5000 PSI	7000 PSI	
#3	1' - 4"	1' - 4"	1' - 4"	1' - 4"	1' - 4"	1' - 4"	1' - 4"	1' - 4"	1' - 4"	1' - 4"	1' - 8"	1' - 8"	#3
#4	1' - 7"	1' - 5"	1' - 4"	1' - 4"	1' - 6"	1' - 4"	1' - 4"	1' - 4"	1' - 8"	1' - 8"	1' - 8"	1' - 8"	#4
#5	2' - 4"	2' - 1"	1' - 8"	1' - 6"	1' - 10"	1' - 7"	1' - 5"	1' - 4"	2' - 1"	2' - 1"	1' - 9"	1' - 8"	#5
#6	3' - 1"	2' - 10"	2' - 3"	2' - 0"	2' - 2"	1' - 11"	1' - 11"	1' - 8"	2' - 6"	2' - 6"	2' - 1"	1' - 9"	#6
#7	5' - 0"	4' - 6"	3' - 8"	3' - 3"	3' - 7"	3' - 1"	3' - 2"	2' - 8"	2' - 11"	2' - 11"	3' - 1"	2' - 7"	#7
#8	6' - 2"	5' - 7"	4' - 7"	4' - 2"	4' - 6"	3' - 11"	4' - 0"	3' - 4"	3' - 3"	2' - 3"	3' - 6"	2' - 11"	#8
#9	7' - 6"	6' - 9"	5' - 8"	5' - 1"	5' - 6"	4' - 10"	4' - 11"	4' - 2"	3' - 11"	3' - 8"	3' - 11"	3' - 4"	#9
#10	9' - 0"	8' - 1"	6' - 7"	5' - 11"	6' - 9"	5' - 10"	5' - 11"	5' - 0"	4' - 10"	4' - 2"	4' - 8"	3' - 12"	#10
#11	10' - 7"	9' - 6"	7' - 3"	6' - 6"	8' - 1"	7' - 0"	7' - 1"	6' - 0"	5' - 9"	4' - 11"	5' - 8"	4' - 9"	#11

NOTES:
1. FOR DEVELOPMENT LENGTHS, DIVIDE THE TABULATED VALUES BY 1.3.
2. PROVIDE CONTINUOUS REINFORCING WHEREVER POSSIBLE; SPLICE ONLY AS SHOWN OR APPROVED.
3. DOWELS SHALL MATCH THE SIZE AND SPACING OF THE SPECIFIED REINFORCING AND SHALL BE LAPPED WITH CLASS 'B' SPLICES.
4. UNLESS NOTED OTHERWISE LAP LENGTHS SHALL BE OF LENGTHS TABULATED AND AS MODIFIED BY THESE NOTES:
5. WHERE HORIZONTAL REINFORCING IS PLACED SUCH THAT MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLICE, MULTIPLY TABULATED VALUES BY 1.3.
6. FOR EPOXY OR ZINC DUAL-COATED REINFORCING, MULTIPLY TABULATED VALUES BY THE GREATER OF THE FOLLOWING THAT APPLY:
   A. 1.5 FOR REINFORCING HAVING A CLEAR COVER LESS THAN 3 BAR DIAMETERS OR FOR REINFORCING HAVING A CLEAR SPACING LESS THAN 6 BAR DIAMETERS
   B. 1.2 FOR REINFORCING HAVING A CLEAR COVER OF 3 BAR DIAMETERS OR MORE OR FOR REINFORCING HAVING A CLEAR SPACING OF 6 BAR DIAMETERS OR MORE
7. THE FACTORS FROM NOTES 5 AND 6, WHEN MULTIPLIED TOGETHER, NEED NOT BE GREATER THAN 1.7.
8. FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE, MULTIPLY THE TABULATED VALUES BY 1.3.
9. FOR 75 KSI REINFORCING, MULTIPLY THE TABULATED VALUES BY 1.25 (VALUES BASED ON 60 KSI).
10. COLUMNS THAT HAVE #11 BARS SPLICED TO #14 OR #18 BARS SHALL USE LAP LENGTHS LISTED FOR #11 BARS.
11. SPLICES BETWEEN #14 AND #18 BARS REQUIRE MECHANICAL COUPLERS, REF RELEVANT DETAILS OR SPECIFICATIONS.
12. IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZES, PROVIDE LAP BASED ON SMALLER BAR SIZE OR CLASS 'A' OF LARGER BAR, WHICHEVER IS GREATER.
13. UNLESS NOTED OTHERWISE ALL REINFORCING BARS SHALL LAP AROUND CORNERS.
14. FOR VERTICAL WALL REINFORCING AND HORIZONTAL WALL REINFORCING FOR SPECIAL CONCRETE SHEAR WALLS, TABULATED VALUES SHALL BE MULTIPLIED BY 1.25 TO INSURE DUCTILITY. TABULATED VALUES FOR SPECIAL BOUNDARY ELEMENTS HAVE ALREADY BEEN MULTIPLIED BY 1.25.
15. ALL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE.
16. FOR CONCRETE STRENGTHS NOT LISTED, USE THE NEXT SMALLEST CONCRETE STRENGTH LISTED IN THE TABLE TO CALCULATE THE REQUIRED BAR LAP.

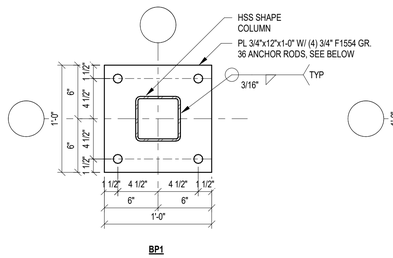
BASES OF DESIGN TABLE
1. SITUATIONS NOT CONFORMING TO THE FOLLOWING BELOW SHALL BE REVIEWED BY A CASE BY CASE BASIS
2. SLABS AND HORIZONTAL WALL REINFORCING: 3/4" (MIN) COVER AND 1 1/2" (MIN) CLEAR SPACING BETWEEN BARS.
3. BEAMS: 1 1/2" (MIN) COVER WITH STIRRUP AND EITHER THE GREATER OF 2 3/8" OR 2 DB (MIN) CLEAR SPACING BETWEEN BARS
4. FOOTINGS AND RETAINING WALLS: 3" (MIN) CLEAR FOR GRADE BEAMS, PILE CAPS OR FOOTINGS AND 2" (MIN) CLEAR FOR RETAINING WALLS AND 3" (MIN) CLEAR SPACING BETWEEN BARS
5. VERTICAL WALL REINFORCING: 3/4" (MIN) COVER WITH HORIZONTAL #4 BAR (MIN) AND 4" (MIN) SPACING BETWEEN BARS
6. COLUMNS: 1 1/2" (MIN) COVER WITH #3 TIES AT 12" OC (MIN) AND 3" (MIN) CLEAR SPACING BETWEEN BARS.
7. SPECIAL BOUNDARY ELEMENTS: 3/4" (MIN) COVER WITH HORIZONTAL #4 BAR (MIN) AND CONFINEMENT PER TYPICAL DETAILS AND 4" (MIN) SPACING BETWEEN BARS

3 REINF BAR LAP LENGTH SCHEDULE
3/4" = 1'-0"

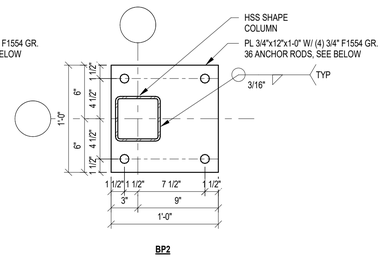
REINFORCING BAR LAP LENGTH SCHEDULE (CLASS B)				
GRADE 60 STEEL				
NORMAL WEIGHT CONCRETE STRENGTH				
BAR	3000 PSI	4000 PSI	5000 PSI	7000 PSI
#3	21"	18"	17"	14"
#4	28"	25"	22"	19"
#5	36"	31"	28"	23"
#6	43"	37"	33"	28"
#7	62"	54"	48"	41"
#8	71"	62"	55"	47"
#9	80"	70"	62"	53"
#10	90"	78"	70"	59"
#11	100"	87"	78"	66"

LAP SCHEDULE NOTES:
1. LENGTH SHOWN CONFORM TO NON-SEISMIC PROVISIONS OF ACI 318 FOR UNCOATED BARS ENCLOSED BY PROPERLY SPACED TIES OR STIRRUPS
2. LENGTH IN TABLE SHALL BE FACTORED FOR THE FOLLOWING CONDITIONS
   - HORIZONTAL BARS MORE THAN 12" ABOVE BOTTOM OF CAST MEMBER: 1.3xTABLE LENGTH
   - LIGHT WEIGHT CONCRETE: 1.3xTABLE LENGTH
   - BAR CLEAR SPACING SHALL BE NO LESS THAN ONE BAR DIAMETER AND/OR BAR CLEAR COVER LESS THAN ONE BAR DIAMETER: 1.5xTABLE LENGTH
   - WHERE MORE THAN ONE CONDITION APPLIES, ALL APPLICABLE FACTORS SHALL BE APPLIED TO LENGTH INDICATED IN TABLE
   - GRADE 80 STEEL: 1.15x TABLE LENGTH (EGN VERIFY)
3. THIS TABLE SHALL APPLY UNLESS SPECIFICALLY NOTED, DETAILED OR SCHEDULED OTHERWISE
4. UNLESS NOTED OTHERWISE ALL REINFORCING BARS SHALL LAP AROUND CORNERS

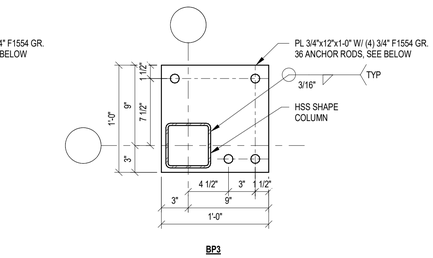
5 REINF BAR LAP LENGTH SCHEDULE
3/4" = 1'-0"



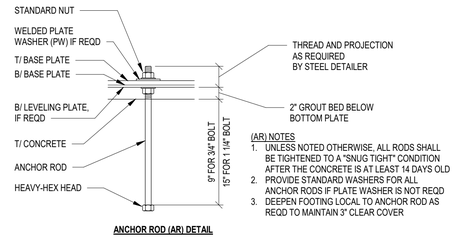
BP1



BP2

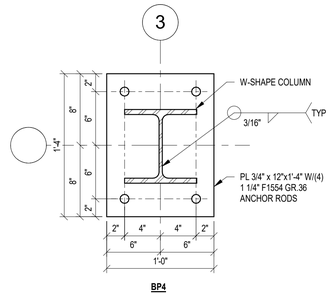


BP3



ANCHOR ROD (AR) DETAIL

(AR) NOTES:
1. UNLESS NOTED OTHERWISE, ALL RODS SHALL BE TIGHTENED TO A "SNUG TIGHT" CONDITION AFTER THE CONCRETE IS AT LEAST 14 DAYS OLD
2. PROVIDE STANDARD WASHERS FOR ALL ANCHOR RODS IF PLATE WASHER IS NOT REQD
3. DEEPEN FOOTING LOCAL TO ANCHOR ROD AS REQD TO MAINTAIN 3" CLEAR COVER



BP4

8 BASE PLATE DETAILS
1 1/2" = 1'-0"

BRITT, PETERS AND ASSOCIATES INC
consulting engineers
101 Falls Park Drive
Suite 601
Greenville, SC 29601
(864) 271-8869
www.brittpeters.com
BPA Project #: 240281

Seal

SOUTH CAROLINA LICENSED PROFESSIONAL ENGINEER No. 35067 ROBERT OWEN GERVIN
08/06/2024

SOUTH CAROLINA Britt, Peters & Associates, Inc. No. C00781 CERTIFICATE OF AUTHORIZATION

DP3 ARCHITECTS

DP3 Architects, Ltd.
15 South Main Street, Suite 400
Greenville, SC 29601
864.232.8200
www.DP3architects.com

Project

MARIACHIS
RESTAURANT
LANCASTER, SC

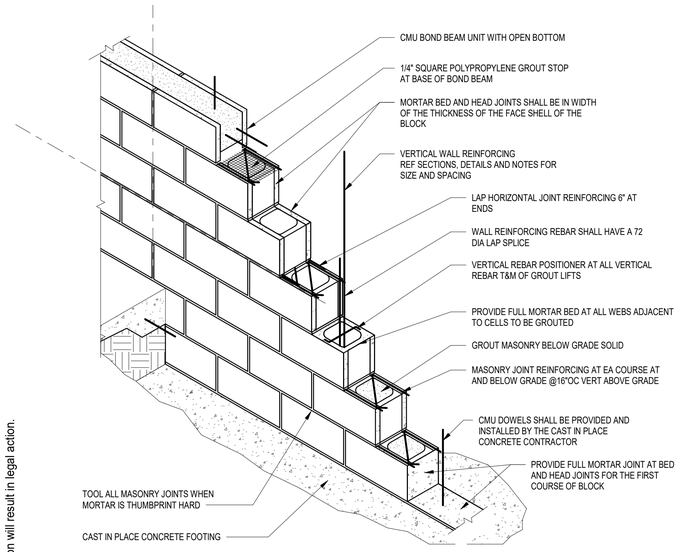
Project Number	23213
Drawn By	BC
Checked By	ROG
Date	05/AUG/24

Revisions

Drawing

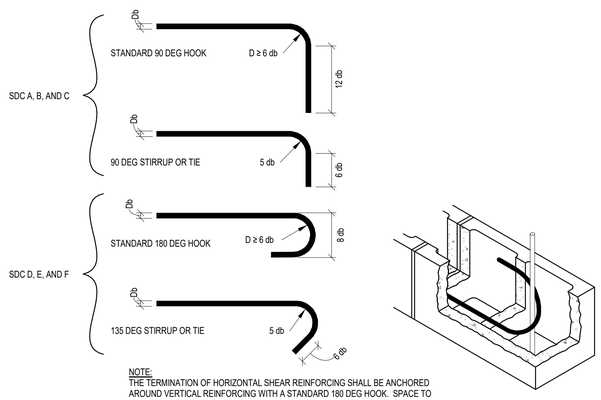
TYPICAL CONCRETE DETAILS

S3.03



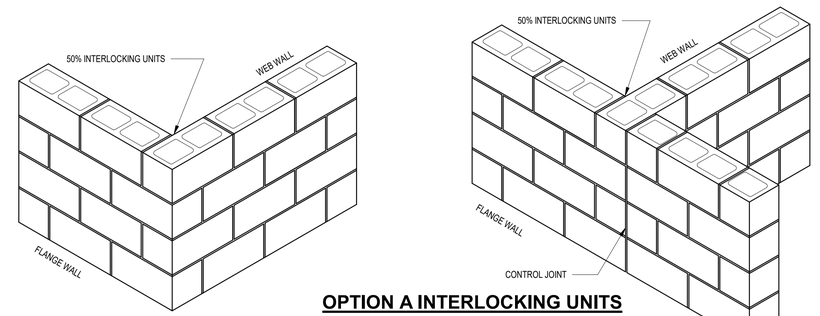
NOTE:
MASONRY SHALL BE CONSTRUCTED IN LIFTS OF 48" OR LESS. AT LOCATIONS MID HEIGHT IN MASONRY THE GROUT SHALL BE HELD DOWN 1" FROM THE TOP OF THE MASONRY

1 WALL CONSTRUCTION
3/4" = 1'-0"

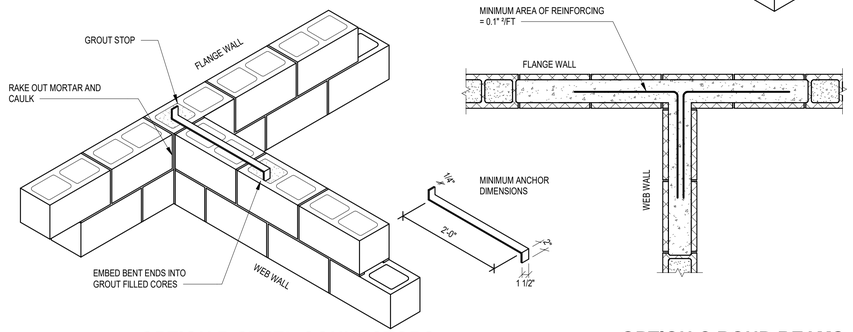


NOTE:
THE TERMINATION OF HORIZONTAL SHEAR REINFORCING SHALL BE ANCHORED AROUND VERTICAL REINFORCING WITH A STANDARD 180 DEG HOOK. SPACE TO ACCOMMODATE THE HOOK IS LIMITED. THE HOOK MAY NEED TO BE PLACED ON AN ANGLE WITHIN THE BOND BEAM AS SHOWN

2 MASONRY WALL HORIZONTAL REINFORCING - STANDARD HOOKS AND BENDS
1 1/2" = 1'-0"



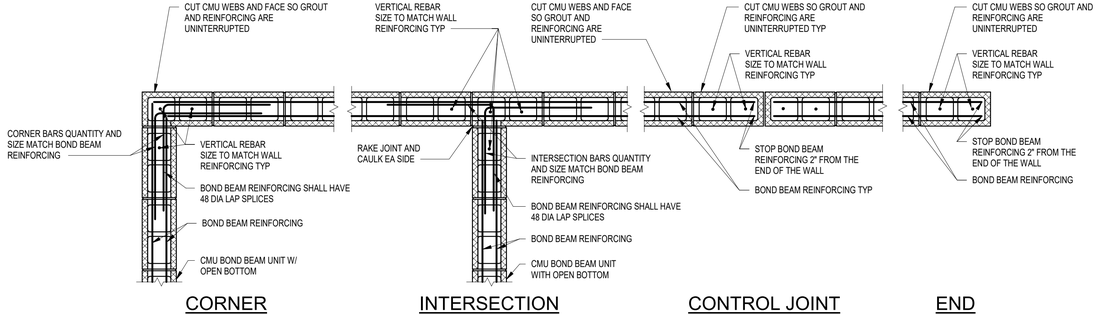
OPTION A INTERLOCKING UNITS



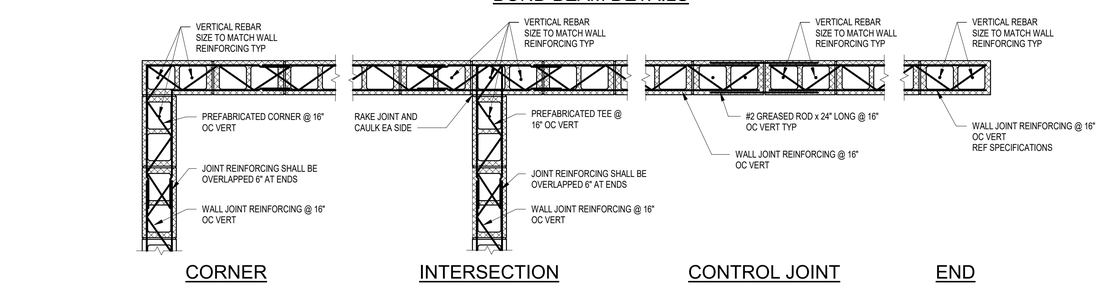
OPTION B STEEL CONNECTORS

OPTION C BOND BEAMS

3 INTERSECTING WALLS
3/4" = 1'-0"



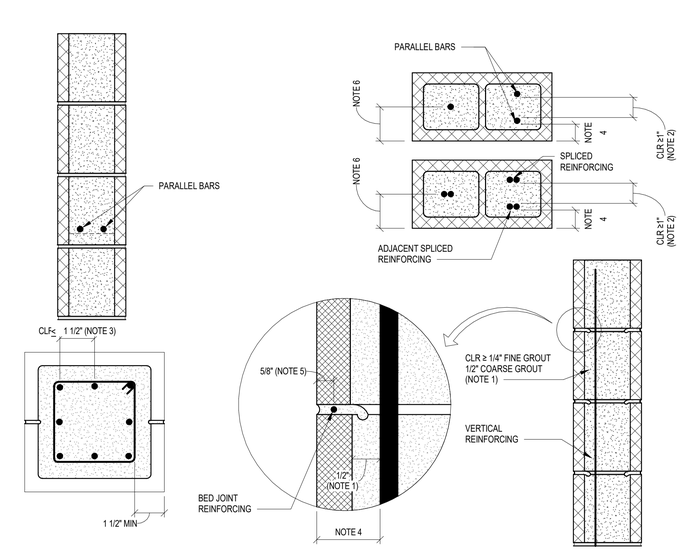
BOND BEAM DETAILS



JOINT REINFORCING DETAILS

NOTE:
PROVIDE 1/2" JOINT B/W ALL MASONRY AND CIP COLUMNS

5 STRUCTURAL MASONRY WALL DETAILS
3/4" = 1'-0"



NOTES:
1. THE THICKNESS OF GROUT BETWEEN THE REINFORCING AND MASONRY UNITS SHALL NOT BE LESS THAN 1/4" FOR FINE GROUT OR 1/2" FOR COARSE GROUT. (NOTE: THIS REQUIREMENT DOES NOT APPLY TO THE PRESENCE OF MORTAR PROTRUSIONS).
2. CLEAR DISTANCE BTWN PARALLEL BARS SHALL NOT BE LESS THAN THE NOMINAL DIAMETER OF THE BARS (d), NOR LESS THAN 1".
3. IN COLUMNS AND PILASTERS, THE CLEAR DISTANCE BTWN VERTICAL BARS SHALL NOT BE LESS THAN 1-1/2 TIMES THE NOMINAL BAR DIA (d), NOR LESS THAN 1 1/2".
4. REINFORCING BARS SHALL HAVE A MASONRY COVER DISTANCE (WHICH INCLUDES THE UNIT, GROUT, AND MORTAR) NOT LESS THAN:
   A. FOR MASONRY EXPOSED TO EARTH OR WEATHER: 2" FOR BARS LARGER THAN #5 AND 1 1/2" FOR #5 BARS OR SMALLER.
   B. FOR MASONRY NOT EXPOSED TO EARTH OR WEATHER: 1 1/2".
5. JOINT REINFORCING SHALL BE FULLY EMBEDDED IN MORTAR OR GROUT WITH A MINIMUM COVER OF 5/8" WHEN EXPOSED TO EARTH OR WEATHER OR WHEN THE AVERAGE AMBIENT RELATIVE HUMIDITY EXCEEDS 75%. FOR ALL OTHER CASES THE MINIMUM COVER DISTANCE IS REQD TO BE 1/2".
6. FOR CELLS WITH SINGLE BAR, CENTER BAR IN CELL.

6 PLACEMENT OF REINFORCEMENT
1 1/2" = 1'-0"

LOOSE ANGLE LINTEL SCHEDULE

CLEAR SPAN	ANGLE
0'-0" - 5'-0"	L3 1/2x3 1/2x3/8
5'-1" - 8'-0"	L5x3 1/2x3/8 (LLV)
8'-1" - 10'-0"	L5x3 1/2x3/8 (LLV)

NOTES:
1. PROVIDE 8" MINIMUM BEARING AT EACH END OF ANGLE.
2. TOE OF ANGLE SHALL BE LOCATED 1" FROM FACE OF BRICK MAX.
3. FOR EXACT SIZE AND LOCATION OF WALL OPENINGS, COORDINATE WITH ARCHITECTURAL DRAWINGS.
4. ANGLE LINTEL SCHEDULE APPLIES ONLY TO ANGLE LINTELS NOT OTHERWISE SHOWN ON THE STRUCTURAL DRAWINGS.
5. ANGLE LINTELS IN EXTERIOR WALLS SHALL BE HOT DIP GALVANIZED.
6. AT BRICK VENEER CONTROL JOINT, FORM SLIP PLANE BY PLACING FLASHING ABOVE AND BELOW ANGLE. PROVIDE 1/4" GAP AT EACH END OF ANGLE FOR THERMAL EXPANSION.

LOOSE ANGLE LINTEL SCHEDULE

BRITT, PETERS AND ASSOCIATES INC consulting engineers
101 Falls Park Drive
Suite 601
Greenville, SC 29601
(864) 271-8869
www.brittpeters.com
BPA Project #: 240281

Seal

08/06/2024

DP3 ARCHITECTS
DP3 Architects, Ltd.
15 South Main Street, Suite 400
Greenville, SC 29601
864.232.8200
www.DP3architects.com

Project

MARIACHIS
RESTAURANT
LANCASTER, SC

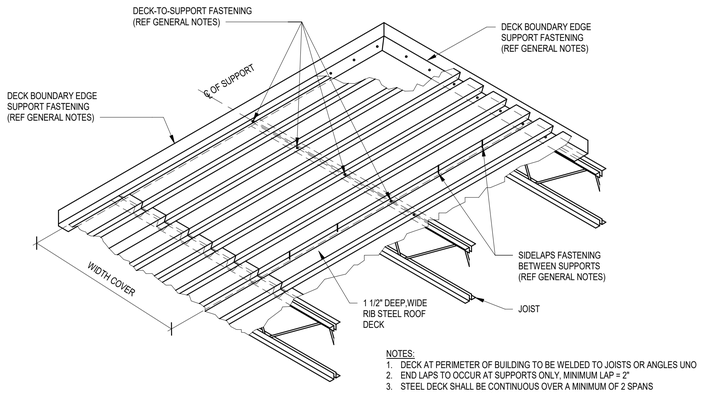
Project Number	23213
Drawn By	BC
Checked By	ROG
Date	05/AUG/24

Revisions

Drawing

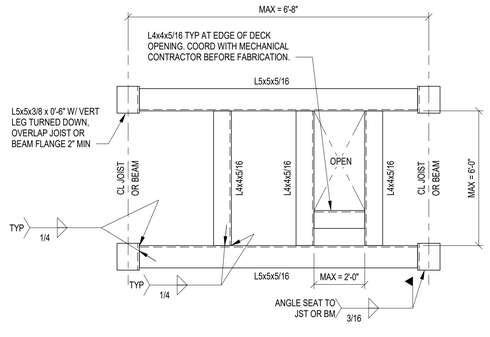
TYPICAL MASONRY DETAILS

S3.04



NOTES:
1. DECK AT PERIMETER OF BUILDING TO BE WELDED TO JOISTS OR ANGLES UNO
2. END LAPS TO OCCUR AT SUPPORTS ONLY. MINIMUM LAP = 2"
3. STEEL DECK SHALL BE CONTINUOUS OVER A MINIMUM OF 2 SPANS
4. BUTT WELD ALL SPLICES AT PERIMETER ANGLE

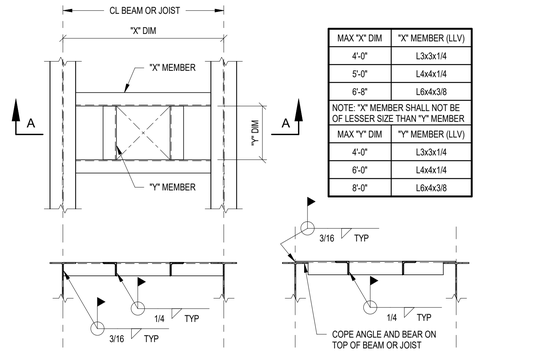
1 1 1/2" ROOF DECK FASTENING ISOMETRIC
3/4" = 1'-0"



REF MECH DWGS FOR SIZE, NUMBER AND LOCATION OF ROOF TOP UNITS AND DECK OPENINGS. ALIGN FRAMING MEMBERS WITH ROOF TOP UNIT CURB.

ENG NOTE:
THIS DETAIL DESIGNED FOR 3,000 LB ROOF TOP UNIT.

TYP ROOF TOP EQUIPMENT DECK OPENING DETAIL (ANGLE)
3/4" = 1'-0"



MAX "X" DIM	"X" MEMBER (LLV)
4'-0"	L3x3x1/4
5'-0"	L4x4x1/4
6'-8"	L6x4x3/8
NOTE: "X" MEMBER SHALL NOT BE OF LESSER SIZE THAN "Y" MEMBER	
MAX "Y" DIM	"Y" MEMBER (LLV)
4'-0"	L3x3x1/4
6'-0"	L4x4x1/4
8'-0"	L6x4x3/8

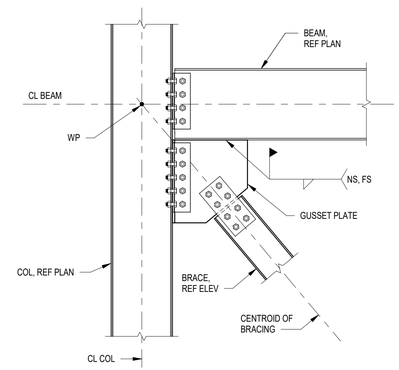
SECTION A-A OPTION 1

SECTION A-A OPTION 2

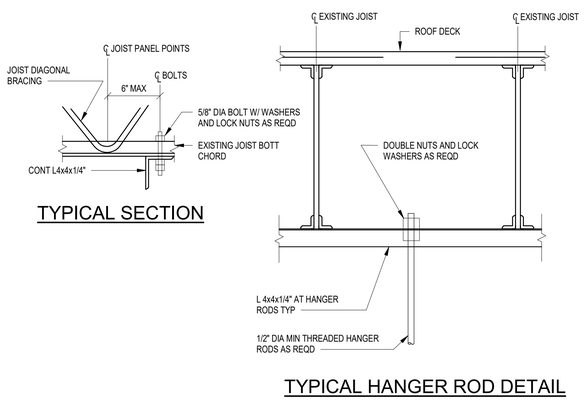
NOTES:
1. USE THIS DETAIL AT ROOF DECK ONLY. NOT COMPOSITE SLABS
2. FOR SIZE AND LOCATION OF REQUIRED ROOF OPENINGS VERIFY W/ MECHANICAL DWGS AND OR HVAC MANUFACTURER.

ENGINEER NOTE:
SIZES HAVE BEEN CHECKED FOR DL=20PSF AND LL=30PSF (EXCLUDES LOADS IN OPENINGS)

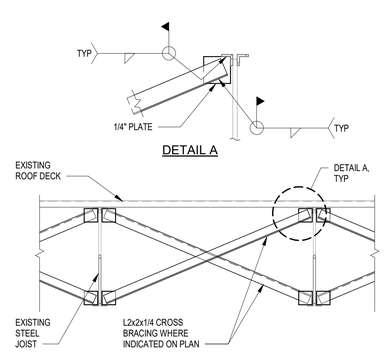
TYPICAL ROOF SUPPORT AT OPENINGS
3/4" = 1'-0"



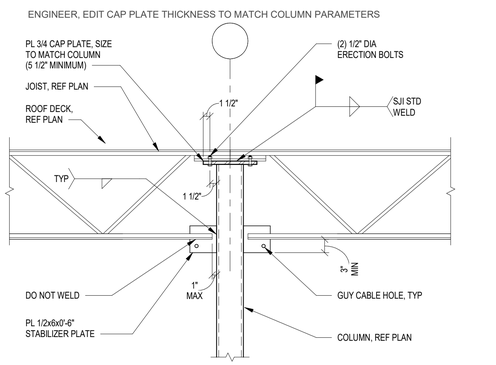
TYP FRAME CONNECTION - W BRACE
3/4" = 1'-0"



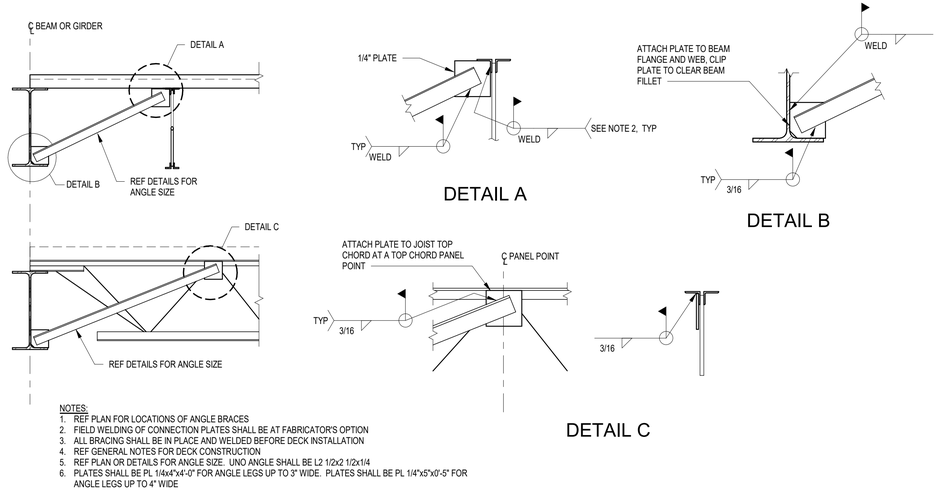
TYPICAL HANGER ROD DETAIL
3/4" = 1'-0"



TYPICAL STEEL JOIST CROSS BRACE
3/4" = 1'-0"

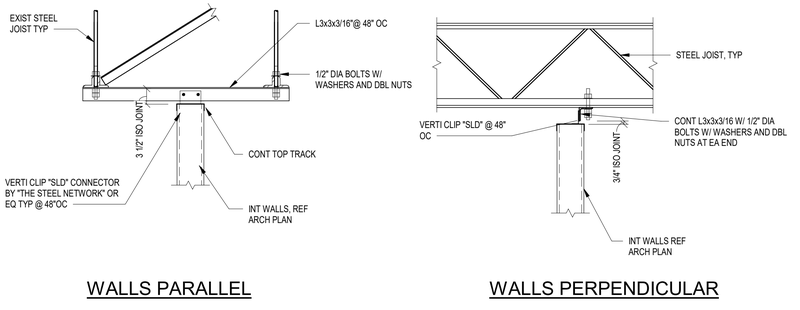


TYPICAL ROOF JOIST TO COLUMN CONNECTION
3/4" = 1'-0"

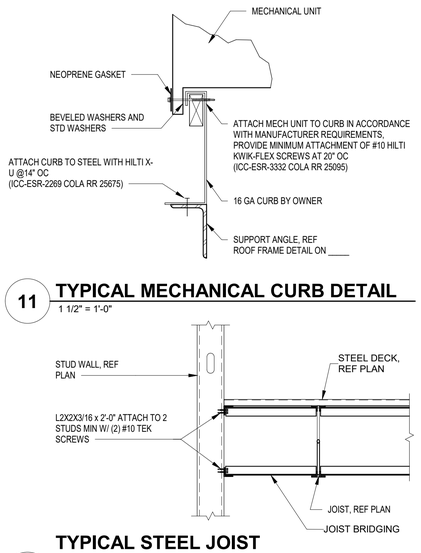


NOTES:
1. REF PLAN FOR LOCATIONS OF ANGLE BRACES
2. FIELD WELDING OF CONNECTION PLATES SHALL BE AT FABRICATOR'S OPTION
3. ALL BRACING SHALL BE IN PLACE AND WELDED BEFORE DECK INSTALLATION
4. REF GENERAL NOTES FOR DECK CONSTRUCTION
5. REF PLAN OR DETAILS FOR ANGLE SIZE. UNO ANGLE SHALL BE L2 1/2x2 1/2x1/4
6. PLATES SHALL BE PL 1/4x4"x4'-0" FOR ANGLE LEGS UP TO 3" WIDE. PLATES SHALL BE PL 1/4"x5"x0'-5" FOR ANGLE LEGS UP TO 4" WIDE

BOTTOM FLANGE BRACE
3/4" = 1'-0"



INTERIOR WALL CONNECTION
3/4" = 1'-0"



TYPICAL MECHANICAL CURB DETAIL
1 1/2" = 1'-0"

TYPICAL STEEL JOIST BRIDGING ATTACHMENT TO WALL
3/4" = 1'-0"

BRITT, PETERS AND ASSOCIATES INC consulting engineers
101 Falls Park Drive
Suite 601
Greenville, SC 29601
(864) 271-8869
www.brittpeters.com
BPA Project #: 240281

Seal

SOUTH CAROLINA REGISTERED PROFESSIONAL ENGINEER No. 35067 ROBERT OWEN GERVIN
08/06/2024

SOUTH CAROLINA Britt, Peters & Associates, Inc. No. C00781 CERTIFICATE OF AUTHORIZATION

DP3 ARCHITECTS
DP3 Architects, Ltd.
15 South Main Street, Suite 400
Greenville, SC 29601
864.232.8200
www.DP3architects.com

Project

MARIACHIS
RESTAURANT
LANCASTER, SC

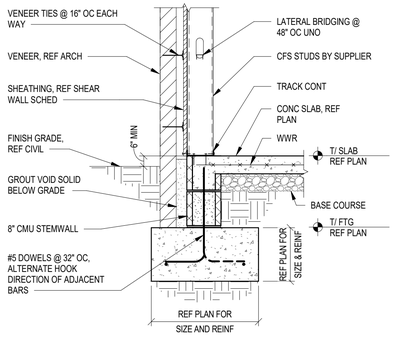
Project Number	23213
Drawn By	BC
Checked By	ROG
Date	05/AUG/24

Revisions

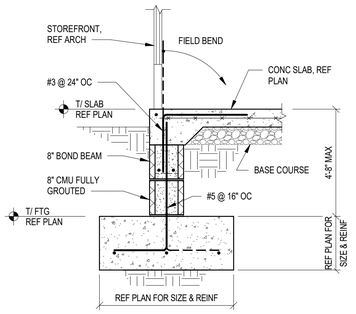
Drawing

TYPICAL STEEL DETAILS

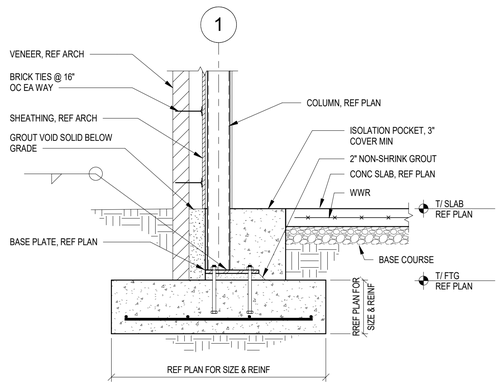
S3.06



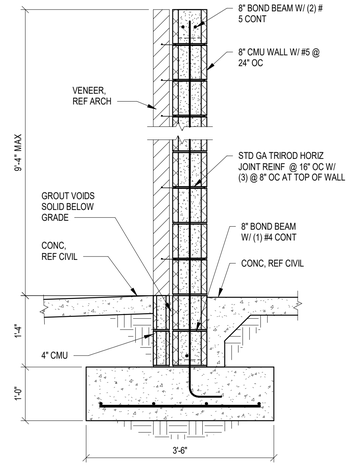
1 SECTION
3/4" = 1'-0"



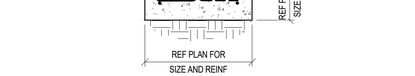
2 SECTION
3/4" = 1'-0"



3 SECTION
3/4" = 1'-0"



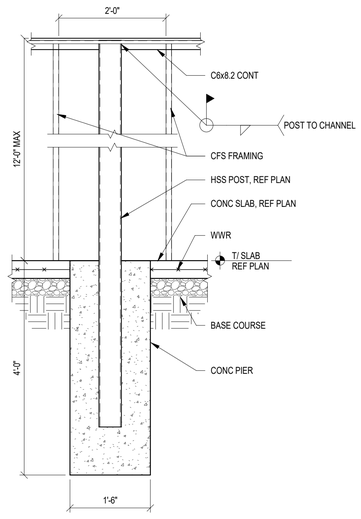
4 DUMPSTER ENCLOSURE SECTION
3/4" = 1'-0"



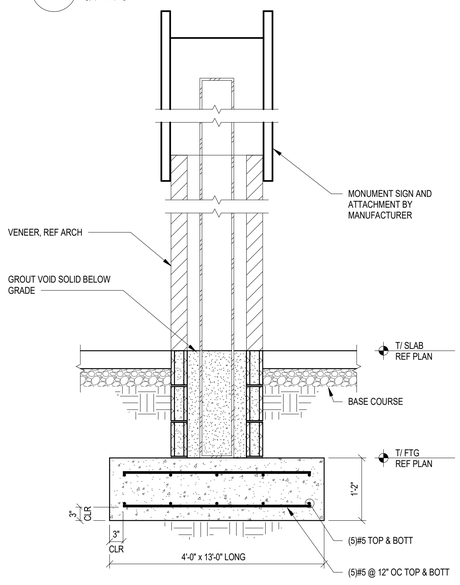
5 SECTION
3/4" = 1'-0"

6 SECTION
3/4" = 1'-0"

9 CONCRETE CURB
1 1/2" = 1'-0"



7 SECTION
3/4" = 1'-0"



8 MONUMENT SIGN FOUNDATION
3/4" = 1'-0"

NOTE REGARDING REINF COVER REQUIREMENTS

ALL REINFORCING SHALL BE PLACED IN ACCORDANCE WITH THE MINIMUM COVER REQUIREMENTS PER ACI AS OUTLINED IN THE GENERAL NOTES. SPECIFIC BAR LOCATIONS SHOWN IN SECTIONS AND DETAILS MAY OVERRIDE BUT NOT VIOLATE THE MINIMUM COVER REQUIREMENTS.

BRITT, PETERS AND ASSOCIATES INC consulting engineers
101 Falls Park Drive
Suite 601
Greenville, SC 29601
(864) 271-8869
www.brittpeters.com
BPA Project #: 240281

Seal

SOUTH CAROLINA LICENSED PROFESSIONAL ENGINEER No. 35067 ROBERT OWEN GERVIN
08/06/2024

SOUTH CAROLINA Britt, Peters & Associates, Inc. No. C00781 CERTIFICATE OF AUTHORIZATION

DP3 ARCHITECTS
DP3 Architects, Ltd.
15 South Main Street, Suite 400
Greenville, SC 29601
864.232.8200
www.DP3architects.com

Project

MARIACHIS
RESTAURANT
LANCASTER, SC

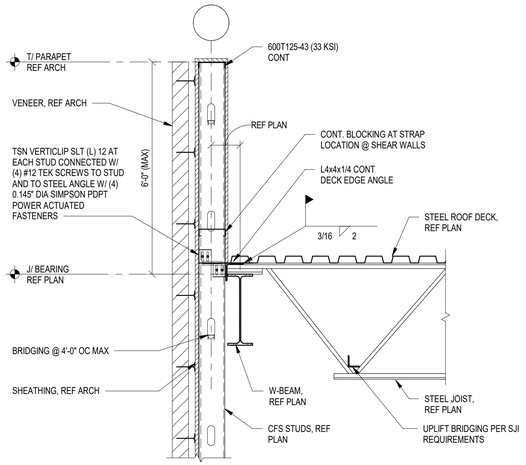
Project Number	23213
Drawn By	BC
Checked By	ROG
Date	05/AUG/24

Revisions

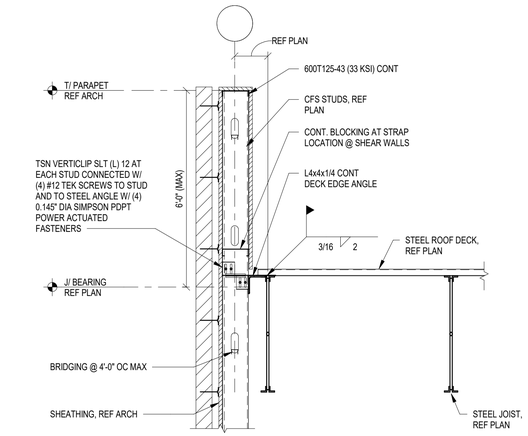
Drawing

FOUNDATION SECTIONS

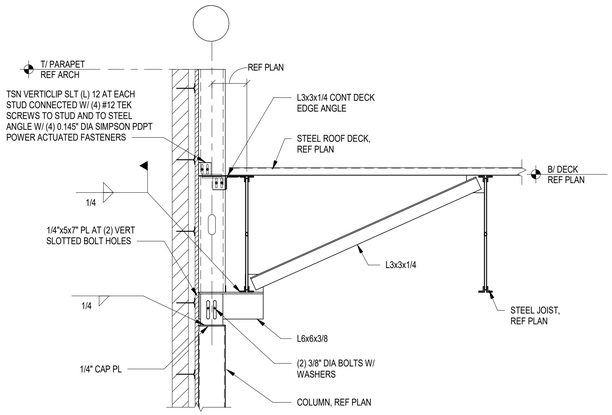
S4.01



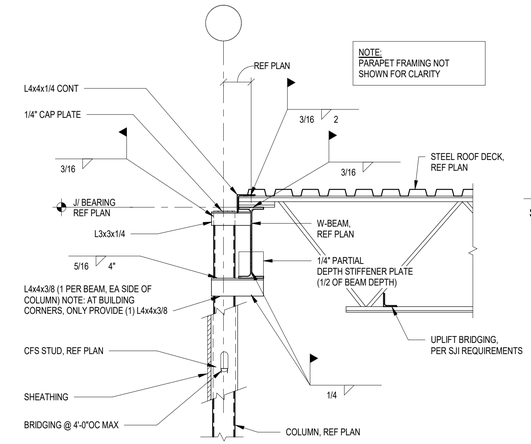
1 SECTION
3/4" = 1'-0"



2 SECTION
3/4" = 1'-0"



3 SECTION
3/4" = 1'-0"



4 SECTION
3/4" = 1'-0"

5 SECTION
3/4" = 1'-0"

6 SECTION
3/4" = 1'-0"

7 SECTION
3/4" = 1'-0"

BRITT, PETERS AND ASSOCIATES INC
consulting engineers
101 Falls Park Drive
Suite 601
Greenville, SC 29601
(864) 271-8869
www.brittpeters.com
BPA Project #: 240281

Seal

SOUTH CAROLINA LICENSED PROFESSIONAL ENGINEER
No. 35067
ROBERT OWEN GERVIN
08/06/2024

SOUTH CAROLINA
Britt, Peters & Associates, Inc.
No. C00781
CERTIFICATE OF AUTHORIZATION

DP3 ARCHITECTS
DP3 Architects, Ltd.
15 South Main Street, Suite 400
Greenville, SC 29601
864.232.8200
www.DP3architects.com

Project

MARIACHIS
RESTAURANT
LANCASTER, SC

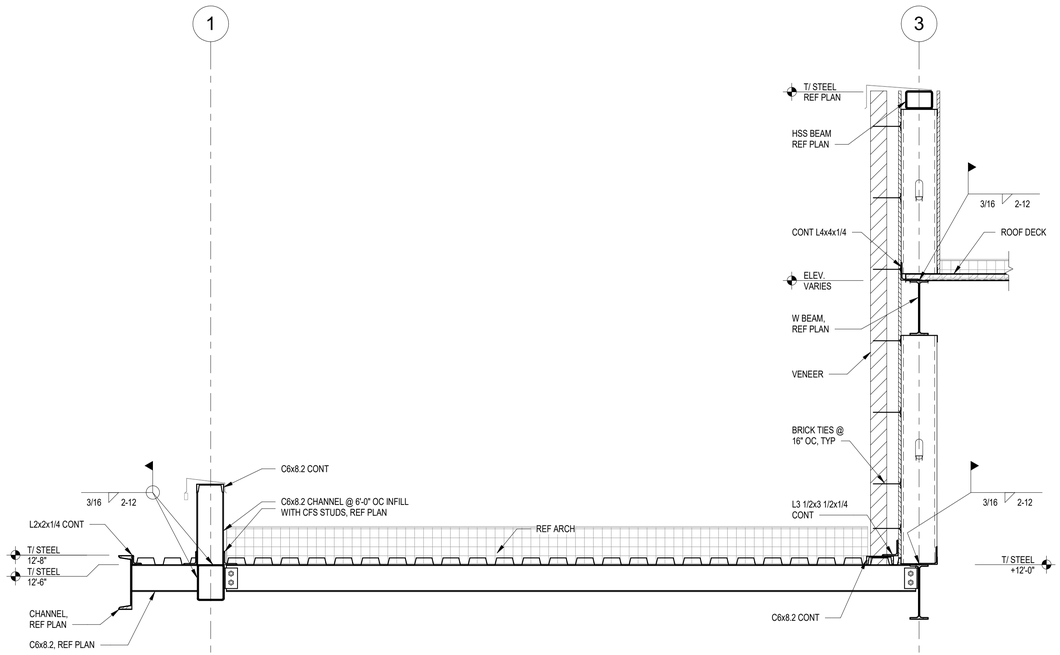
Project Number	23213
Drawn By	BC
Checked By	ROG
Date	05/AUG/24

Revisions

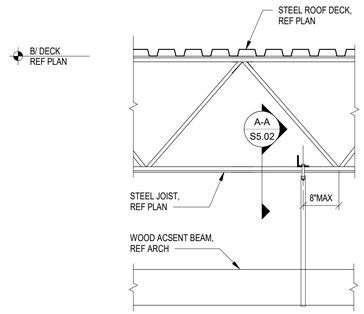
Drawing

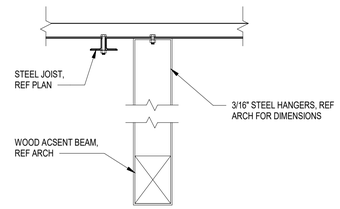
ROOF SECTIONS

S5.01



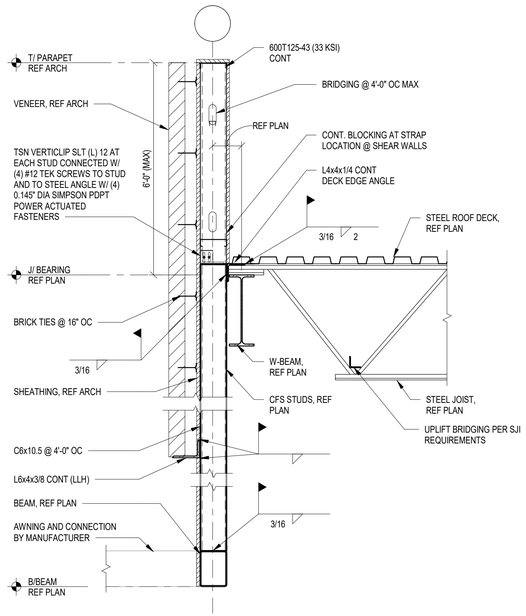
1 SECTION
3/4" = 1'-0"



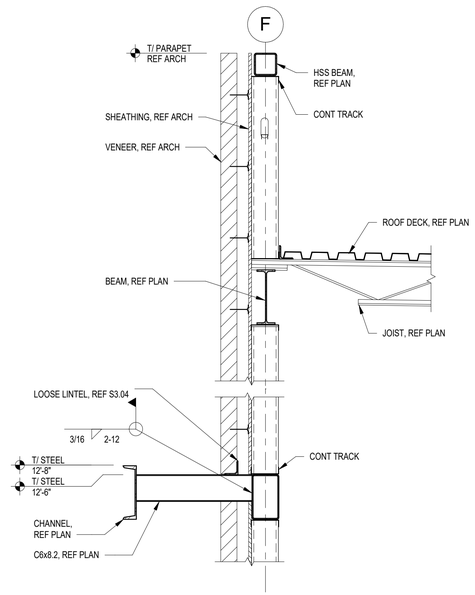


SECTION A-A

2 SECTION
3/4" = 1'-0"



3 SECTION
3/4" = 1'-0"



4 SECTION
3/4" = 1'-0"

BRITT, PETERS AND ASSOCIATES INC
consulting engineers
101 Falls Park Drive
Suite 601
Greenville, SC 29601
(864) 271-8869
www.brittpeters.com
BPA Project #: 240281

Seal

SOUTH CAROLINA REGISTERED PROFESSIONAL ENGINEER
No. 35067
ROBERT OWEN GERVIN
08/06/2024

SOUTH CAROLINA
Britt, Peters & Associates, Inc.
No. C00781
CERTIFICATE OF AUTHORIZATION

DP3 ARCHITECTS
DP3 Architects, Ltd.
15 South Main Street, Suite 400
Greenville, SC 29601
864.232.8200
www.DP3architects.com

Project

MARIACHIS
RESTAURANT
LANCASTER, SC

Project Number	23213
Drawn By	BC
Checked By	ROG
Date	05/AUG/24

Revisions

Drawing

ROOF SECTIONS

S5.02