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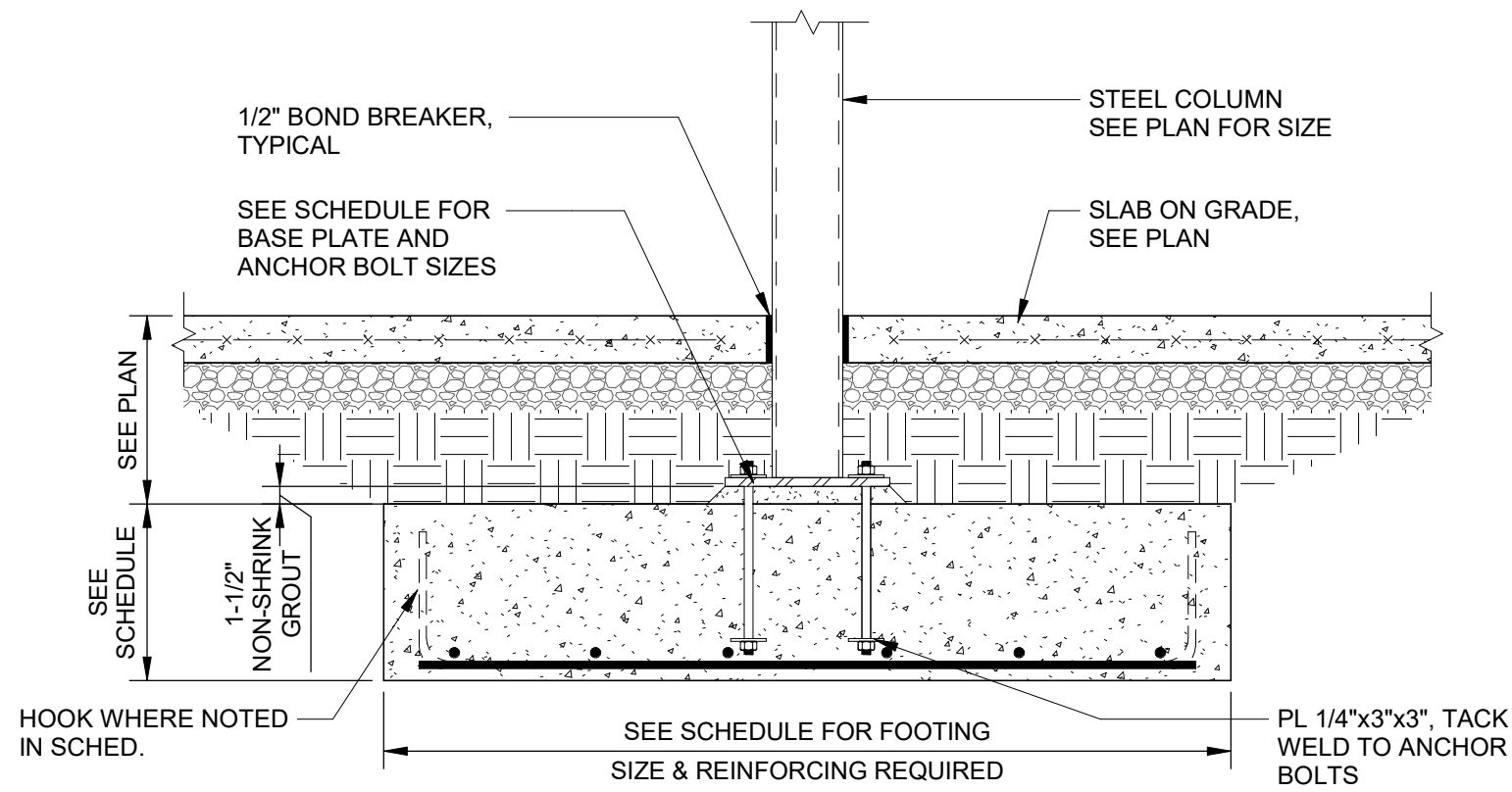
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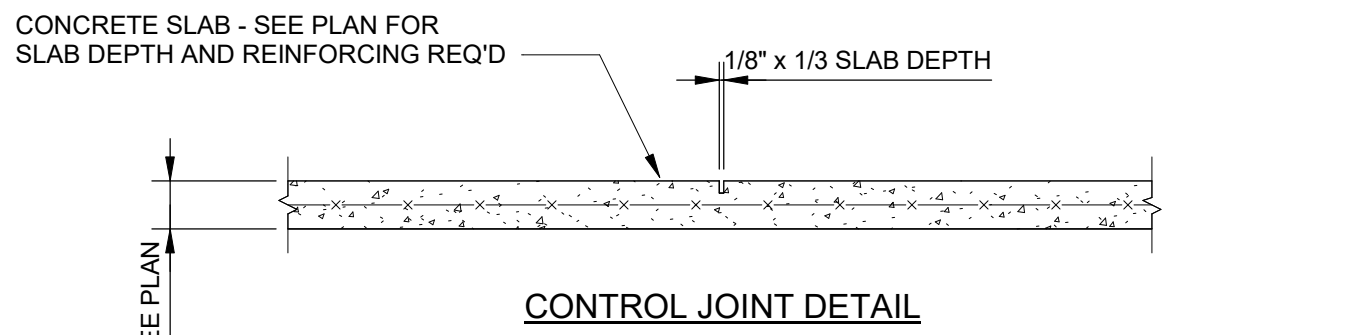
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APPENDIX "B"			
STRUCTURAL DESIGN			
DESIGN LOADS:			
IMPORTANCE FACTORS	WIND	(1) _w	1.0
	SNOW	(1) _s	1.0
	SEISMIC	(1) _e	1.0
DEAD LOADS	ROOF	30	PSF
	FLOOR	35	PSF
LIVE LOADS	ROOF	20	PSF
	COMMON AREA	100	PSF
	GUESTROOM	40	PSF
GROUND SNOW LOAD: 10			
WIND LOAD: BASIC WIND SPEED V_{ULT} 110 mph (ASCE 7-10)			
EXPOSURE CATEGORY C			
WIND BASE SHEARS (FOR MWRFs) V_{E1} 100 KIPS V_{E2} 289 KIPS			

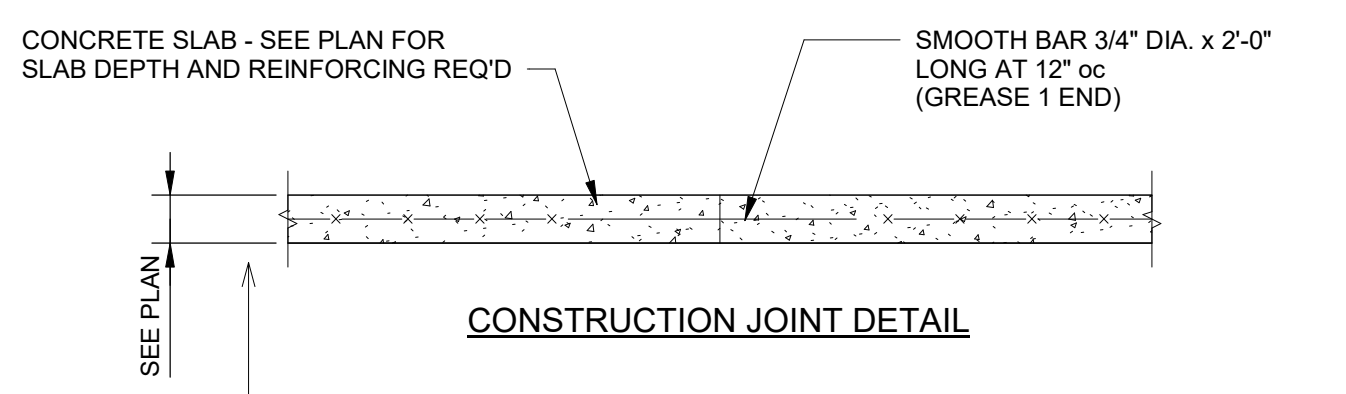
COMPLIANCE WITH SECTION 1616.4 ONLY
FOR SEISMIC DESIGN CATEGORY A? ☒ NO ☐ YES
SEISMIC DESIGN CATEGORY ☐ A ☐ B ☒ C ☐ D
PROVIDE THE FOLLOWING SEISMIC DESIGN PARAMETERS:
RISK CATEGORY II
SPECTRAL RESPONSE ACCELERATION S_s 30.1 kg S_1 15.5 kg
SITE CLASSIFICATION D ☒ FIELD TEST ☐ PRESUMPTIVE ☐ HISTORICAL DATA
BASIC STRUCTURAL SYSTEM (CHECK ONE)
☒ BEARING WALL ☐ DUAL W/ SPECIAL MOMENT FRAME
☐ BUILDING FRAME ☐ DUAL W/ INTERMEDIATE R/C OR SPECIAL STEEL
☐ MOMENT FRAME ☐ INVERTED PENDULUM
SEISMIC BASE SHEAR V_{E1} 204 KIPS V_{E2} 204 KIPS
ANALYSIS PROCEDURE ☐ SIMPLIFIED ☒ EQUIVALENT LATERAL FORCE ☐ MODAL
ARCHITECTURAL, MECHANICAL, COMPONENTS ANCHORED? YES
LATERAL DESIGN CONTROL? ☒ DIR EARTHQUAKE ☐ DIR WIND



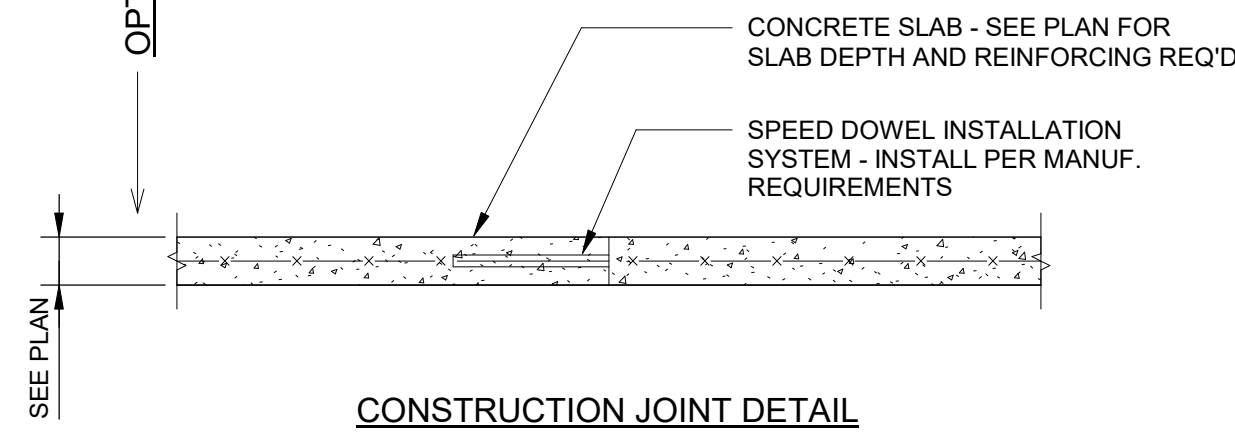
TYPICAL COLUMN BASE DETAIL



CONTROL JOINT DETAIL

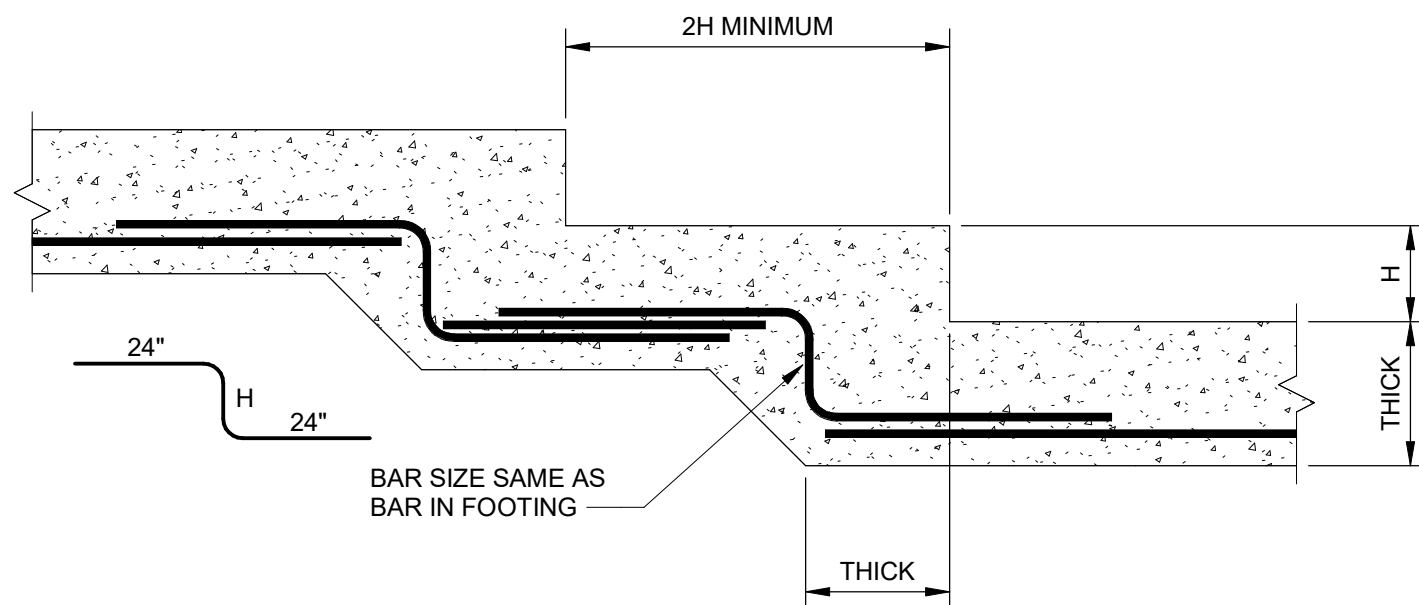


CONSTRUCTION JOINT DETAIL



CONSTRUCTION JOINT DETAIL

CONSTRUCTION / CONTROL JOINT DETAILS



TYPICAL FOOTING STEP DETAIL

SUBMITTALS:

SUBMIT FOR REVIEW:
SUBMITTALS FOR SHOP DRAWINGS, PRODUCT DATA, AND MILL TESTS ARE REQ'D FOR ITEMS NOTED IN THE INDIVIDUAL MATERIALS SECTIONS AND FOR BIDDER DESIGNED ELEMENTS.

SUBMITTAL REVIEW PERIOD:
SUBMITTALS SHALL BE MADE IN TIME TO PROVIDE A MINIMUM OF (2) TWO WEEKS OR (10) TEN WORKING DAYS TO REVIEW BY THE ARCHITECT/ENGINEER PRIOR TO THE ONSET OF FABRICATION.

GENERAL CONTRACTORS PRIOR REVIEW:
PRIOR TO THE SUBMISSION TO THE ARCHITECT/ENGINEER, THE CONTRACTOR SHALL REVIEW THE SUBMITTAL FOR COMPLETENESS, DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE S.E.R. AND THEREFORE, MUST BE VERIFIED BY THE GENERAL CONTRACTOR. CONTRACTOR SHALL PROVIDE ANY NECESSARY DIMENSIONAL DETAILS REQUESTED BY THE DETAILER AND PROVIDE THE CONTRACTOR'S REVIEW STAMP AND SIGNATURE BEFORE FORWARDING TO THE ARCHITECT /ENGINEER.

SHOP DRAWING REVIEW:
ONCE THE CONTRACTOR HAS COMPLETED HIS REVIEW, THE STRUCTURAL ENGINEER OF RECORD (S.E.R.) WILL REVIEW THE SUBMITTAL FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT AND THE CONTRACT DOCUMENTS OF THE BUILDING AND WILL STAMP THE SUBMITTAL ACCORDINGLY. MARKINGS OR COMMENTS SHALL NOT BE CONSTRUED AS RELIEVING THE CONTRACTOR FROM COMPLIANCE WITH THE PROJECT PLANS AND SPECIFICATIONS, NOR DEPARTURES THERE FROM. THE S.E.R. WILL RETURN SUBMITTALS IN THE FORM THEY ARE SUBMITTED IN (EITHER HARD COPIES OR ELECTRONIC) FOR HARD COPY SUBMITTALS. THE CONTRACTOR IS RESPONSIBLE FOR SUBMITTING THE REQUIRED NUMBER OF COPIES TO THE S.E.R. FOR REVIEW.

SHOP DRAWING DEVIATIONS:
WHEN SHOP DRAWINGS (COMPONENT DESIGN DRAWINGS) DIFFER FROM OR ADD TO THE REQUIREMENTS OF THE STRUCTURAL DRAWINGS THEY SHALL BE DESIGNED AND STAMPED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED.

DEFERRED SUBMITTALS:

BIDDER DESIGNED ELEMENTS:
SUBMIT "BIDDER-DESIGNED" DEFERRED SUBMITTALS TO THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD (S.E.R.).

FOR REVIEW. THE DEFERRED SUBMITTALS SHALL ALSO BE SUBMITTED TO THE CITY FOR APPROVAL, IF REQUIRED BY THE CITY.

DESIGN OF PREFABRICATED, "BIDDER-DESIGNED", MANUFACTURED, PRE-ENGINEERED, OR OTHER FABRICATED PRODUCTS SHALL COMPLY WITH THE FOLLOWING REQUIREMENTS.
1. DESIGN CONSIDERS TRIBUTARY DEAD, LIVE, WIND AND EARTHQUAKE LOADS IN COMBINATIONS REQUIRED BY IBC 2021
2. DESIGN WITHIN THE DEFLECTION LIMITS NOTED HEREIN AND AS SPECIFIED OR REFERENCED IN THE IBC 2021
3. DESIGN SHALL CONFORM TO THE SPECIFICATIONS AND REFERENCE STANDARDS OF THE GOVERNING CODE.
4. SUBMITTAL SHALL INCLUDE:
a. CALCULATIONS PREPARED, STAMPED AND SIGNED BY THE SPECIALTY STRUCTURAL ENGINEER (S.S.E.) DEMONSTRATING CODE CONFORMANCE
b. ENGINEERED COMPONENT DESIGN DRAWINGS, ARE PREPARED, STAMPED AND SIGNED BY THE S.S.E.
c. PRODUCT DATA, TECHNICAL INFORMATION, AND MANUFACTURER'S WRITTEN REQUIREMENTS AND AGENCY APPROVALS AS APPLICABLE.
d. S.S.E. MAY SUBMIT TO THE ARCHITECT / ENGINEER, A REQUEST TO UTILIZE RELEVANT ALTERNATE DESIGN CRITERIA OF SIMILAR NATURE AND GENERAL EQUIVALENCY WHICH IS RECOGNIZED BY THE CODE AND ACCEPTABLE TO THE AUTHORITY HAVING JURISDICTION. SUBMIT ADEQUATE DOCUMENTATION OF DESIGN.

DEFLECTION LIMITS FOR S.S.E. / BIDDER DESIGNED ELEMENTS	
VERTICAL	LIMIT
*ROOF MEMBERS, DEAD + LIVE OR RAIN, SNOW, OR WIND TOTAL LOAD (TL) DEFL.	L/240 WHERE (L IS SPAN LENGTH IN INCHES)
ROOF LIVE OR RAIN OR SNOW OR WIND LOAD (RL)	L/360
FLOOR MEMBERS TOTAL LOAD (TL) U.N.O.	L/360
FLOOR LIVE LOAD (LL) U.N.O.	L/480
MEMBERS SUPPORTING MASONRY	L/600 OR 1/4" MAX MASONRY (DL) ONLY
OPERABLE PARTITION SUPPORT MEMBERS	L/600 OR 1/4" MAX
HORIZONTAL	LIMIT
MEMBERS SUPPORTING BRITTLE FINISHES	L/240 (NOTE 1)
MEMBERS SUPPORTING FLEXIBLE FINISHES	L/180 (NOTE 1)
MEMBERS SUPPORTING MASONRY	L/600 @ 0.7 x CLADDING WIND OR 0.7E (NOTE 1)
INTERSTORY DRIFT	0.020 x STORY HEIGHT

*ROOF MEMBERS SHALL BE DESIGNED TO CONTROL PONDING SUCH THAT CHORD MEMBER STRESSES DO NOT EXCEED LIMITS INDICATED ON PLANS UNDER DEAD + RAIN LOADING.
(NOTE 1) WIND LOAD IS REDUCIBLE TO 0.42 TIMES COMPONENT AND CLADDING LOADS PER TABLE 1604.3 FOOT NOTE 1

GENERAL CONTRACTOR'S PRIOR REVIEW:

ONCE THE CONTRACTOR HAS COMPLETED HIS REVIEW OF THE S.S.E. COMPONENT DRAWINGS, THE S.E.R. WILL REVIEW THE SUBMITTAL FOR GENERAL CONFORMANCE WITH THE DESIGN OF THE BUILDING AND WILL STAMP THE SUBMITTAL ACCORDINGLY. REVIEW OF THE SPECIALTY STRUCTURAL ENGINEERS' (S.S.E.) SHOP DRAWINGS (COMPONENT DESIGN DRAWINGS) IS FOR COMPLIANCE WITH DESIGN CRITERIA AND COMPATIBILITY WITH THE DESIGN OF THE PRIMARY STRUCTURE AND DOES NOT RELIEVE THE S.E. OF RESPONSIBILITY FOR THAT DESIGN. ALL NECESSARY BRACING, TIES, ANCHORAGE, PROPRIETARY PRODUCTS SHALL BE FURNISHED AND INSTALLED PER MANUFACTURER'S INSTRUCTIONS OR THE S.S.E.'S DESIGN DRAWINGS AND CALCULATIONS. THE ELEMENTS INCLUDE BUT ARE NOT LIMITED TO:

- * STEEL STAIRS
- * HANDRAIL GUARDRAIL, CATWALK FRAMING, BALCONY RAIL ANCHORAGES
- * METAL DECK EDGE FORMS
- * EXTERIOR CLADDING SYSTEMS, CURTAIN WALL SYSTEMS, PRE-ENGINEERED PANELS
- * WINDOW WASHING SYSTEM TIE-DOWN ANCHORAGE
- * TEMPORARY SHORING SYSTEMS
- * COLD-FORMED METAL STUDS - EXTERIOR WALL ASSEMBLIES
- * COLD-FORMED METAL STUDS - FRAMING INTERIOR WALLS INCLUDING PARTIAL HEIGHT
- * COLD-FORMED METAL STUDS & JOISTS FRAMING OVERHEAD SOFFITS
- * TILT-UP WALL PANELS INCLUDING BUT NOT LIMITED TO PANEL-TO-PANEL CONNECTIONS, FOUNDATION CONNECTIONS, CONNECTIONS BETWEEN OTHER STRUCTURAL ELEMENTS (BEAMS, JOISTS, ETC.) AND EMBEDMENTS.
- * SOIL IMPROVEMENTS (RAMMED-AGGREGATE PIER, RIGID INCLUSIONS, GROUT INJECTIONS, ETC.) TO MEET THE REQUIREMENTS OF THE GEOTECHNICAL REPORT

GENERAL MASONRY NOTES:

CODES: MASONRY DESIGN AND CONSTRUCTION IS BASED ON AND SHALL CONFORM TO THE PROVISIONS OF ACI A530-LATEST EDITION / ASCE 5-LATEST EDITION / TMS 402-LATEST EDITION INCLUDING SEISMIC REQUIREMENTS OF APPENDIX A.

MASONRY: CONCRETE UNITS CONFORMING TO:
ASTM C90 FOR HOLLOW LOADBEARING UNITS
ASTM C145 FOR SOLID LOADBEARING UNITS
ASTM C55 FOR CONCRETE BRICK
MINIMUM F_m = 2000 PSI

ALL MASONRY TO BE TYPE "N" AND MEET ASTM C90 GUIDELINES.

BRICK SHALL CONFORM TO ASTM C62.

8" CMU SHALL BE 7 5/8" X 7 5/8" X 15 5/8" W/ 3/8" JOINTS

MORTAR AND GROUT: USE MORTAR FOR MASONRY CONFORMING TO ASTM C270 TYPE S. GROUT SHALL CONFORM TO ASTM C478 MIN. F_c = 3000 PSI GROUT SHALL CONSIST OF 1 PART PORTLAND CEMENT MIXED TO DESIRED CONSISTENCY.

MASONRY WALLS MADE OF CMU ARE TO BE REINFORCED USING 3/16" TRUSS TYPE REINFORCEMENT (GALVANIZED AFTER FABRICATION) AT 16" O.C.

MASONRY WALLS MADE OF BRICK AND CMU WITH INSULATED CAVITY ARE TO BE REINFORCED USING 3/16" CAVITY-TRUSS TRIPOD TYPE REINFORCEMENT (GALVANIZED AFTER FABRICATION) AT 16" O.C.

MASONRY COURSING TO BE A RUNNING BOND, PERPENDICULAR JOINTS SHALL BE TOOTHED TOGETHER

REINFORCING BARS SHOWN IN MASONRY WALLS SHALL BE LAPPED AS FOLLOWS, ALL LAP SPLICES SHALL BE ACCORDING TO IBC SECTION 2107.2.3

#3 BARS-LAP 18" MIN
#4 BARS-LAP 24" MIN
#5 BARS-LAP 36" MIN
#6 BARS-LAP 56" MIN
#7 BARS-LAP 76" MIN
#8 BARS-LAP 107" MIN

REINFORCING BARS SHALL CONFORM TO ASTM A-615 GRADE 60

BACK-FILL AT THE BASEMENT AND RETAINING WALLS SHALL BE DONE UNDER CONTROLLED CONDITIONS ONLY AFTER WALL HAS SET UP.

ANY CMU MASONRY WITH COMPRESSIVE STRENGTH OF GREATER THAN 1500PSI SHALL BE VERIFIED BY TEST ACCORDING TO ASTM C1314 OR BY ASTM C140. THESE TEST RESULTS SHALL BE AVAILABLE TO THE BUILDING INSPECTOR.

ALL FILLED CELL MASONRY TO BE BUILT TO PRESERVE THE UNOBSTRUCTED VERTICAL CONTINUITY OF THE CELLS TO BE FILLED. LAY UNITS WITH FULL FACE SHELL MORTAR BEDS AND CROSS WEBS ADJACENT TO VERTICAL CORES TO BE FILLED.

FILL CELLS WITH GROUT TO MAXIMUM OF 8 FEET. IF TOTAL POUR IS GREATER THAN 8 FEET, POUR IN FOUR (4) FOOT LIFTS STOPPING 1 1/2" BELOW UPPERMOST UNIT FOR CONSTRUCTION JOINTS.

GENERAL STRUCTURAL STEEL NOTES:

THE DESIGN, DETAILING, FABRICATION, AND ERECTION OF STRUCTURAL AND MISCELLANEOUS STEEL SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE AISC "SPECIFICATIONS FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS."

STEELS: STRUCTURAL STEEL IS TO CONFORM TO THE FOLLOWING SPECIFICATIONS:

STRUCTURAL STEEL (WIDE FLANGE SHAPES): ASTM A992
STRUCTURAL TUBING: ASTM A500, GRADE B, FY=46 KSI
STRUCTURAL STEEL (ALL OTHER SHAPES): ASTM A36
STRUCTURAL STEEL (PLATES): ASTM A36

ROOF DECK TO BE 1 1/2" DEEP, 6" WIDE RIB, PRIME PAINTED, VULCRAFT 1.5B, 18GA OR EQUAL. FASTEN ROOF DECK TO SUPPORTS W/ #12 HEX-HEAD SCREWS USING 3/4" PATTERN MINIMUM WITH SIDE-LAP CONNECTION 12" TO ALLOW ROOF TO ACT AS A DIAPHRAGM.
WELDING: ALL WELDING IS TO CONFORM TO ANSI/AWS D1.1 STRUCTURAL WELDING CODE USING 370XX ELECTRODES. ALL WELDING IS TO BE DONE BY WELDERS QUALIFIED UNDER ANSI/AWS D1.1 PROVIDE EVIDENCE OF QUALIFICATION TO ARCHITECT

BOLTS: ALL BOLTED CONNECTIONS ARE TO BE MADE USING 3/4" A325 BOLTS IN BEARING TYPE CONNECTIONS UNLESS SHOWN OR NOTED OTHERWISE ON THE DRAWINGS. A307 BOLTS MAY BE USED FOR ERECTION BOLTS.

ANCHOR RODS: ANCHOR RODS ARE TO BE OF A36 MATERIAL. ALL ANCHOR RODS ARE TO BE OF A36 MATERIAL. ALL ANCHOR RODS ARE TO BE SET WITH A TEMPLATE PRIOR TO POURING CONCRETE.

GENERAL TIMBER NOTES:

WOOD CONSTRUCTION SHALL BE IN ACCORDANCE WITH CHAPTER 23 OF THE SOUTH CAROLINA STATE BUILDING CODE.

ALL SHOP FABRICATED TRUSSES SHALL BE DESIGNED, FABRICATED, AND INSTALLED IN ACCORDANCE WITH THE NATIONAL DESIGN SPECIFICATION, PUBLISHED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION. DESIGN AND FABRICATION DRAWINGS SHALL BE SEALED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN SOUTH CAROLINA. THESE DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER FOR REVIEW BEFORE FABRICATION AND INSTALLATION OF THE MEMBERS.

ALL TRUSSES SHALL BE Laterally Braced As Recommended By The Manufacturer.

ALL NAILING AND STAPLING FOR CONNECTION OF WOOD MEMBERS, NOT NOTED ON DRAWING, SHALL BE IN ACCORDANCE WITH THE SOUTH CAROLINA STATE BLDG. CODE TABLE 2304.10.2 (FASTENING SCHEDULE). FOR NAILING AND STAPLING CONDITIONS NOT COVERED IN THIS TABLE, THE MANUFACTURER'S RECOMMENDATIONS UNLESS THE BUILDING CODE REQUIREMENTS ARE MORE STRINGENT.

WOOD CONNECTORS SHALL BE BY SIMPSON STRONG-TIE COMPANY, INC. SUBSTITUTIONS WILL BE ACCEPTED WITH APPROVAL OF THE ENGINEER. ALL PRODUCTS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS UNLESS THE BUILDING CODE REQUIREMENTS ARE MORE STRINGENT.

ALL ROOF TRUSSES SHALL BE ANCHORED AGAINST UPLIFT AT EACH BEARING POINT BY MEANS OF ANCHORS AS INDICATED ON THE DRAWINGS.

ALL LVL BEAMS SHALL HAVE A MODULUS OF ELASTICITY OF 3000KSI. LVL MANUFACTURERS: BOISE CASCADE; GEORGIA-PACIFIC; "TRUS JOIST" WEYERHAEUSER, OR EQUAL.

GENERAL NOTES:

COMPLETE SHOP DRAWINGS FOR CONSTRUCTION OF ALL SPECIALTY ITEMS INCLUDING, BUT NOT LIMITED TO: PRECAST CONCRETE, HOLLOW CORE CONCRETE SLABS, CURTAIN WALL GLAZING SYSTEMS, LIGHT GAUGE STEEL ROOF AND WALL FRAMING, HEAVY TIMBER TRUSSES, GEOPHORS (FOUNDATIONS IMPROVEMENTS), GUARDRAILS AND STAIRS, SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF SOUTH CAROLINA AND SHALL BE MADE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.

RESULTS FOR ALL CONCRETE COMPRESSIVE STRENGTH TESTS SHALL BE AVAILABLE ON THE JOB SITE FOR REVIEW BY THE INSPECTOR.

FOUNDATION NOTES:

AN ALLOWABLE SOIL BEARING PRESSURE OF 3000 PSF WAS USED FOR THE DESIGN OF SHALLOW FOUNDATIONS. BASED ON THE REPORT OF SUBSURFACE EXPLORATION AND GEOTECHNICAL EVALUATION DATED APRIL 19, 2024 PREPARED BY ECS SOUTHEAST, LLC (ECS PROJECT No. 14-10819). IF EXISTING CONDITIONS OR TESTS INDICATE THAT THE SOIL IS SOFT OR OTHERWISE QUESTIONABLE, CONTACT THE ENGINEER BEFORE PLACING CONCRETE.

THE SOILS ENGINEER OR HIS REPRESENTATIVE SHALL INSPECT ALL SUBGRADE WORK PRIOR TO THE PLACEMENT OF ANY REINFORCING STEEL OR CONCRETE AND SHALL PERFORM TESTS TO VERIFY THAT SUCH WORK IS IN CONFORMANCE WITH THE PROCEDURES NOTED IN THE CONTRACT DOCUMENTS.

GENERAL CONCRETE NOTES:

ALL REINFORCING BARS TO CONFORM TO ASTM-A615, GRADE 60. ALL ACCESSORIES ARE TO BE INCLUDED. BARS ARE TO BE COLD BENT IN SHOP. BAR SUPPORTS ARE TO BE SPACED IN ACCORDANCE WITH ACI 318 AND CRSI. ALL BAR SPLICES ARE TO BE CONSIDERED CLASS "B". CLASS "B" SPLICES ARE TO BE LAPPED A DISTANCE OF 1.3 L_d. COMPRESSION REINFORCING SHALL BE LAPPED A MIN. OF 40 BAR DIAMETERS.

WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 GRADE 60 OR 70. PLACEMENT IN CONCRETE SHALL BE IN THE CENTER OF THE SLAB, UNLESS NOTED OTHERWISE ON PLAN.

CONCRETE FOR FOOTINGS AND SLABS ON GRADE SHALL ATTAIN A 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI.

SEE FOUNDATION NOTES FOR SOIL REPORT AND DESIGN CRITERIA

CONCRETE PLACEMENT AND QUALITY: PER RECOMMENDATIONS IN ACI SP-15.

DEBRIS: REMOVE ALL DEBRIS FROM FORMS BEFORE PLACING CONCRETE.

SEGREGATION OF AGGREGATES: DO NOT DROP CONCRETE THROUGH REINFORCING STEEL SO AS TO CAUSE SEGREGATION OF AGGREGATES.

INSERTS: SECURELY POSITION ALL ITEMS TO BE CAST IN CONCRETE SUCH AS REINFORCING DOWELS, BOLTS, ANCHORS, PIPES AND SLEEVES IN THE FORMS BEFORE PLACING CONCRETE.

CONSTRUCTION JOINTS: OBTAIN THE ARCHITECTS APPROVAL OF JOINT LOCATIONS IN ALL SLABS, BEAMS, AND SHEAR WALLS. REMOVE LAITANCE AND CLEAN SURFACE OF CONCRETE CONSTRUCTION JOINTS.

PIPES: DO NOT EMBED PIPES OTHER THAN ELECTRICAL CONDUITS IN STRUCTURAL CONCRETE EXCEPT WHERE SPECIFICALLY APPROVED BY THE ARCHITECT. MAXIMUM PIPE SIZE SHALL BE 1/3 OF THE SLAB THICKNESS AND LOCATED AT THE MID DEPTH. MINIMUM SPACING SHALL BE 3 TIMES THE PIPE DIAMETER. PIPES SHALL NOT IMPAIR THE STRENGTH OF THE MEMBER. ANY PIPES OF ALUMINUM ARE PROHIBITED IN STRUCTURAL CONCRETE.

REBAR COVER: ALL DIMENSIONS SHOWING THE LOCATION OF REINFORCING STEEL NOT NOTED AS "CLEAR" ARE TO CENTER OF STEEL MINIMUM REBAR COVER (CLEAR) FOR NON-PRESTRESSED CONCRETE SHALL BE AS FOLLOWS:

LOCATION	MIN. COVERAGE (CLEAR)	TOLERANCES
CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3"	3/8"
EXPOSED TO EARTH OR WEATHER:		
#5 AND SMALLER BARS	1 1/2"	3/8"
#6 AND LARGER BARS	2"	3/4"
NOT EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND:		
STRUCTURAL SLABS & WALLS	3/4"	1/8"
SLABS ON GRADE	1 1/2"	1/4"
BEAMS AND COLUMNS (PRIMARY REINFORCEMENT, TIES, STIRRUPS AND SPIRALS)	1 1/2"	3/8"

TOLERANCES OF REBAR PLACEMENT: TOLERANCE FOR LONGITUDINAL LOCATION OF BENDS AND ENDS OF REINFORCEMENT SHALL BE PLUS OR MINUS 2 INCHES EXCEPT AT DISCONTINUING ENDS OF MEMBERS WHERE TOLERANCES SHALL BE PLUS OR MINUS 1/2 INCH.

DO NOT ADD WATER TO CONCRETE MIX AT SITE UNLESS APPROVED BY ARCHITECT

CONCRETE: NORMAL WEIGHT CONCRETE SHALL HAVE A MINIMUM UNIT WEIGHT OF 145 POUNDS PER CUBIC FOOT.

AGGREGATE: NORMAL WEIGHT AGGREGATE CONFORMING TO ASTM C33.

AGGREGATE SIZE CONFORMING TO:
3/4" ASTM C67
1" ASTM C67
1 1/2" ASTM C67

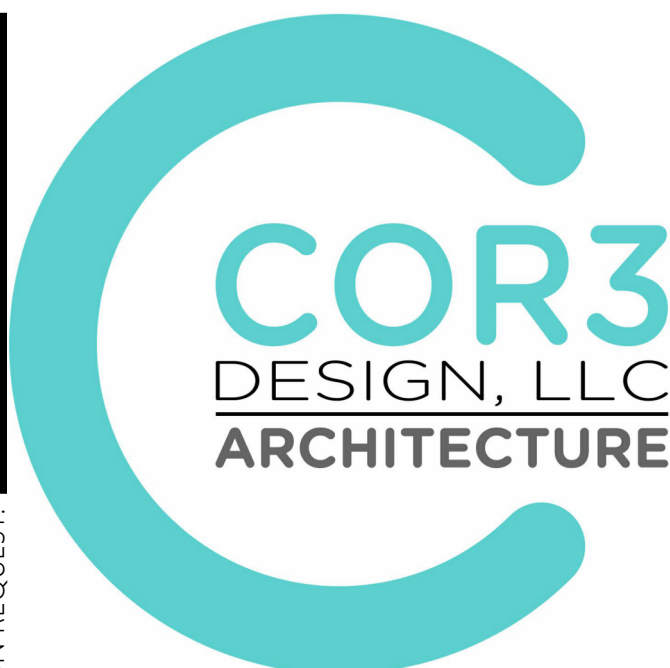
CEMENT: TYPE I

CONCRETE AGE: NO MORE THAN 90 MINUTES SHALL ELAPSE BETWEEN CONCRETE BATCHING AND CONCRETE PLACEMENT UNLESS APPROVED BY ARCHITECT.

MINIMUM REQUIRED LAP SPLICE LENGTHS					
BAR SIZE	TENSION SPLICES				COMPRESSION SPLICES
	CONCRETE STRENGTH				
	2000 psi	3000 psi	4000 psi	5000 psi	
#3	27"	22"	20"	17"	15"
#4	36"	29"	25"	22"	20"
#5	44"	36"	31"	29"	25"
#6	53"	43"	38"	34"	30"
#7	77"	62"	55"	49"	35"
#8	88"	72"	62"	56"	40"
#9	99"	81"	70"	63"	45"
#10	110"	90"	78"	70"	50"

CONCRETE QUALITY

CONCRETE USE	STRENGTH AT 28 DAYS	SLUMP	AIR	AGGREGATE SIZE	AGGREGATE TYPE
FOUNDATIONS	3000 PSI	4"	---	1 1/2"	ASTM C33
EXTERIOR SLABS	4500 PSI	4"	4 - 6%	1"	ASTM C33
STRUCTURAL WALLS	4000 PSI	3"	---	1"	ASTM C33
YARD WALLS	4500 PSI	3"	---	1 1/2"	ASTM C33
SLAB ON GRADE	3000 PSI	4"	---	1"	ASTM C33
ELEVATED SLAB ON METAL DECK	4000 PSI (LIGHT WEIGHT)	4"	---	1"	ASTM C33
POST TENSION SLAB	5000 PSI	4"	---	1"	ASTM C33
CIP ELEVATED SLABS	5000 PSI	4"	---	1"	ASTM C33



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864.583.6274

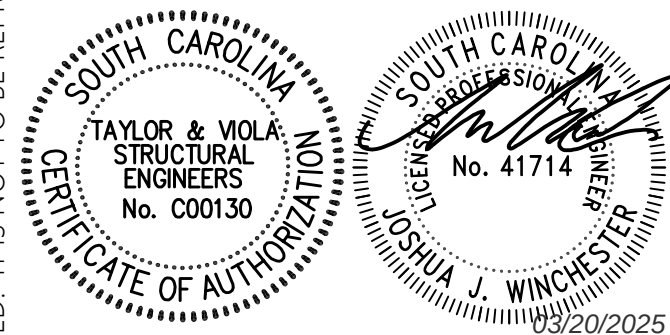
Project Title :

**HOME 2 SUITES
GREENWOOD SC**
475 HOSPITALITY BLVD,
GREENWOOD, SC 29649

Client Logo:



Seals :



Revisions :

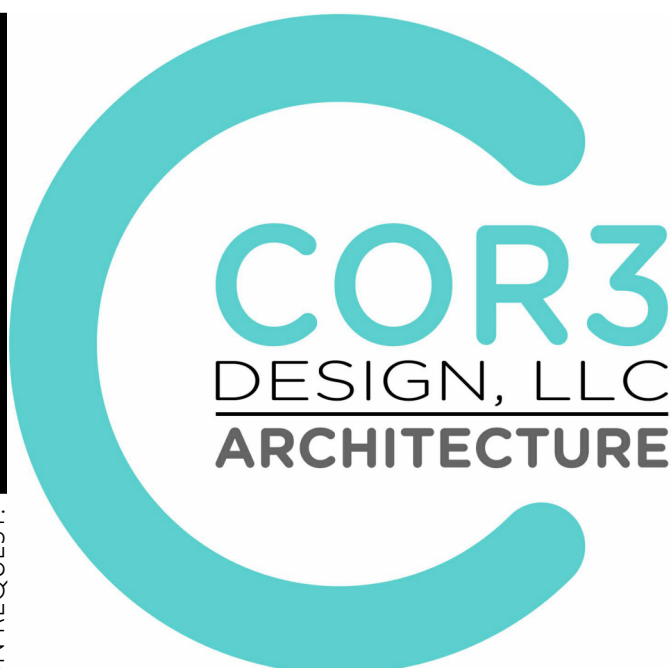
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Checked By :
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10.16.2024 Issue For Permit
A 03.20.2025 100% Issue For Hilton

Project Number :



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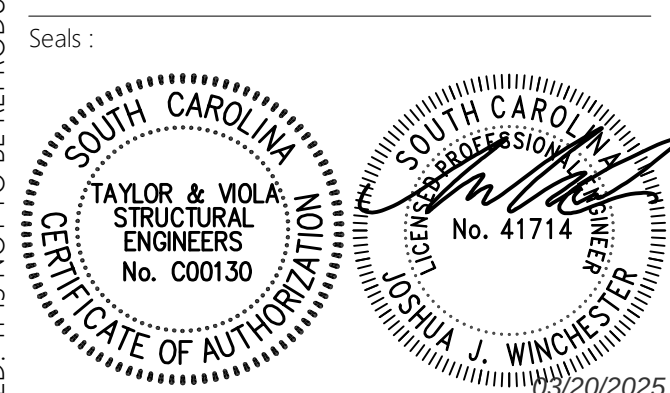
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864.583.6274

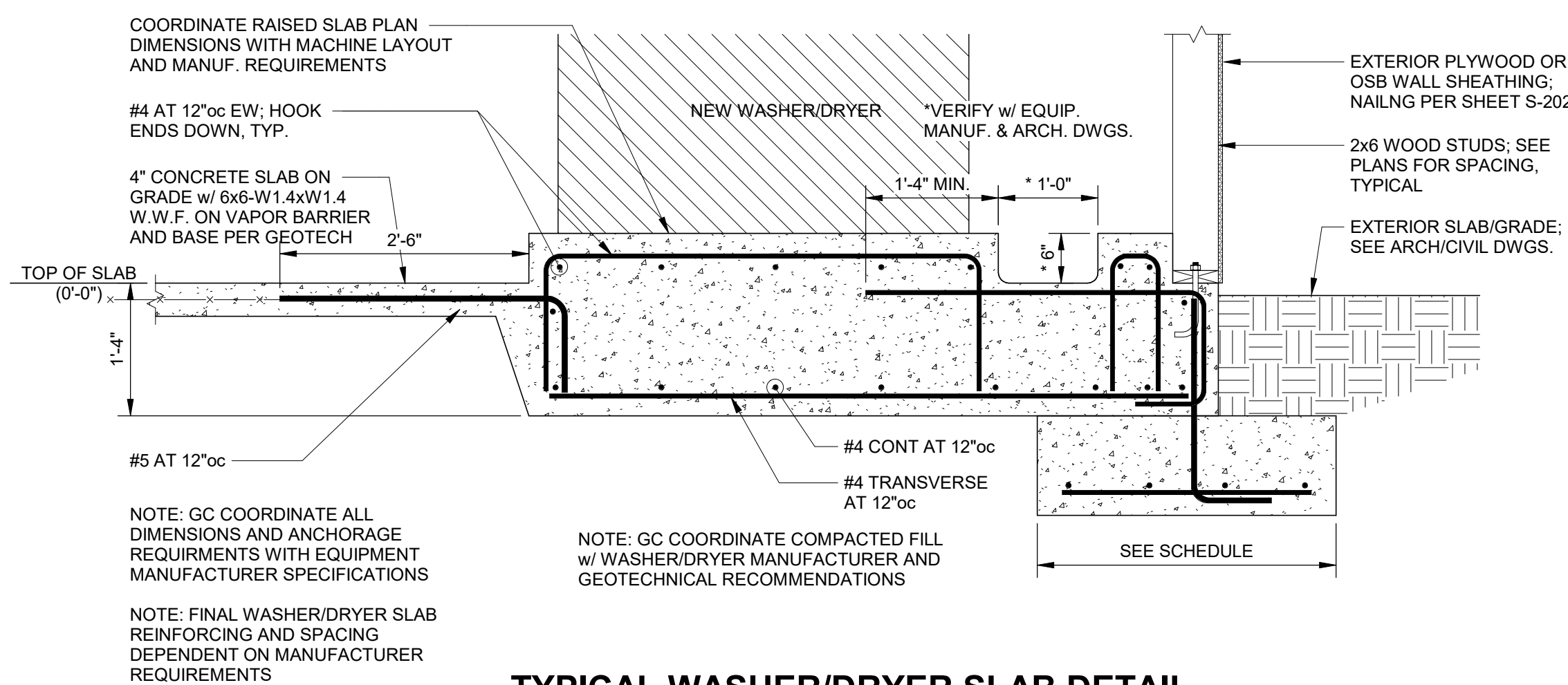
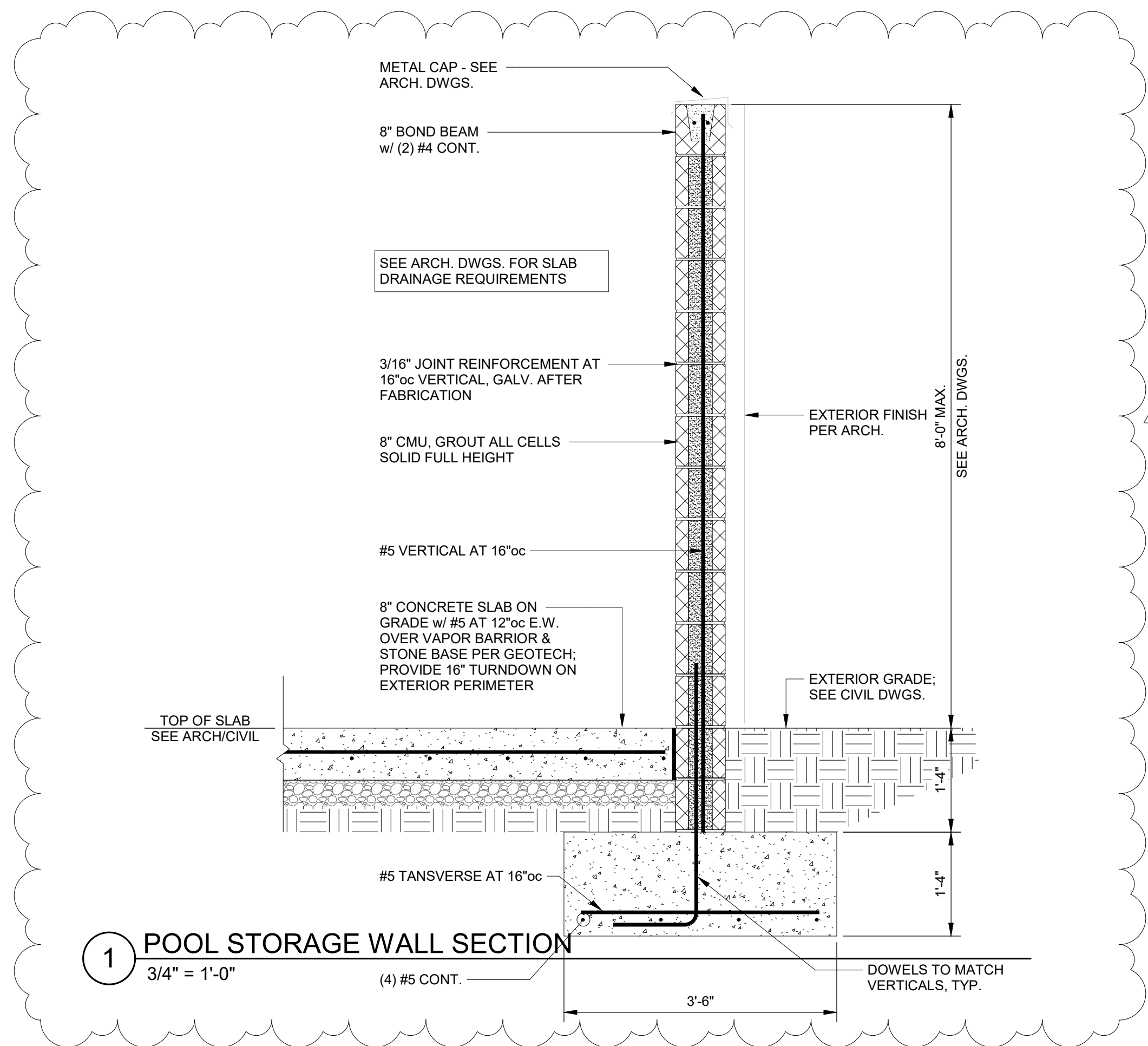
Project Title:
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475 HOSPITALITY BLVD,
GREENWOOD, SC 29649



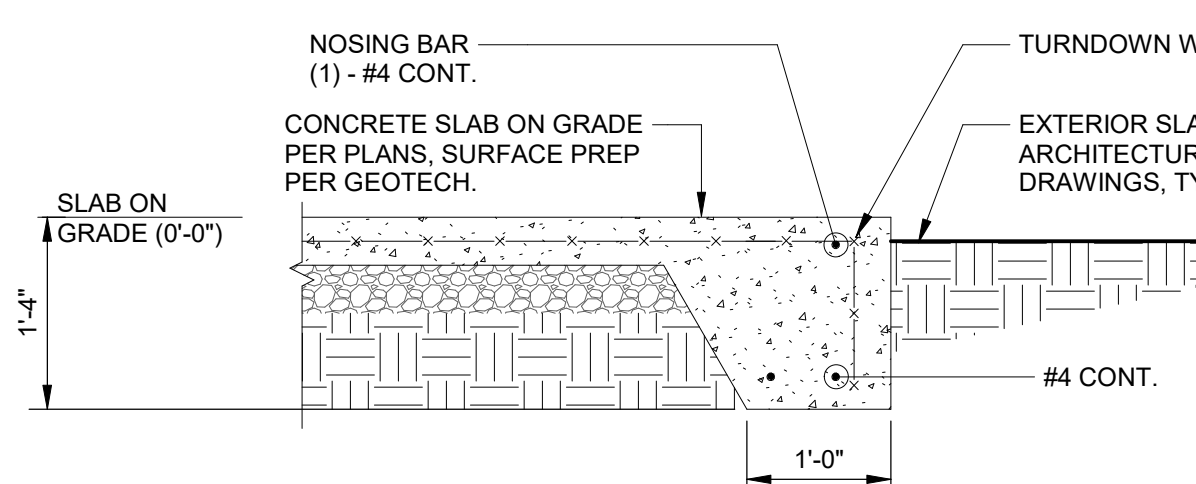
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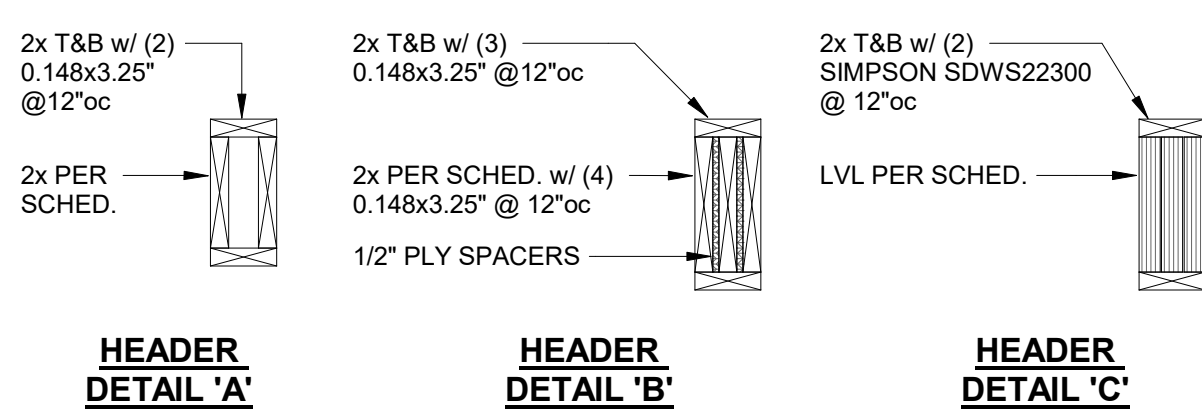
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S-002
Sheet Title:
SCHEDULES AND TYPICAL DETAILS



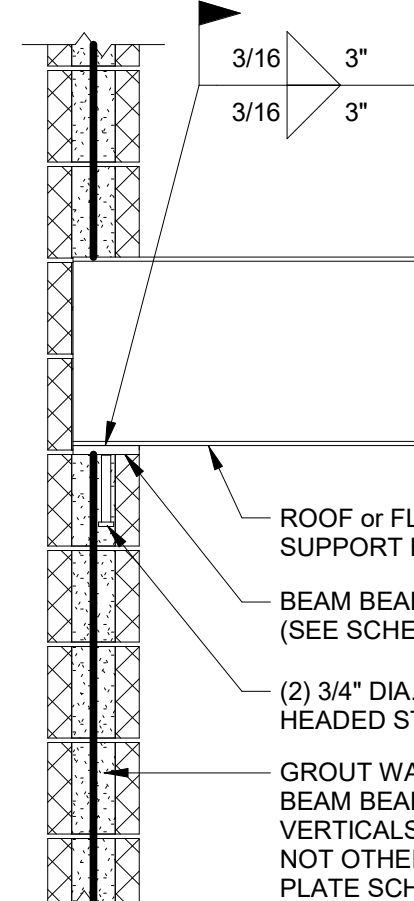
TYPICAL WASHER/DRYER SLAB DETAIL



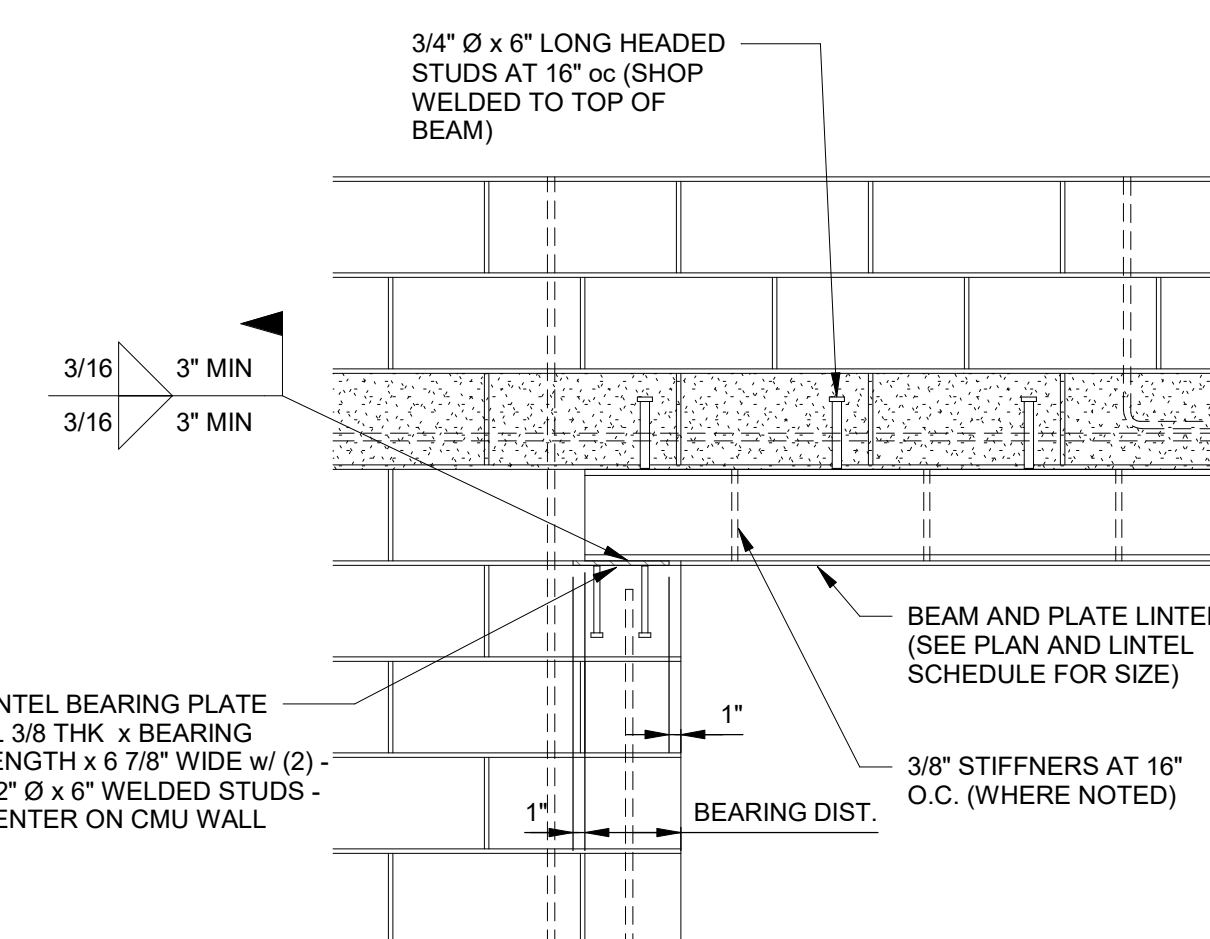
TYPICAL TURNDOWN SLAB DETAIL



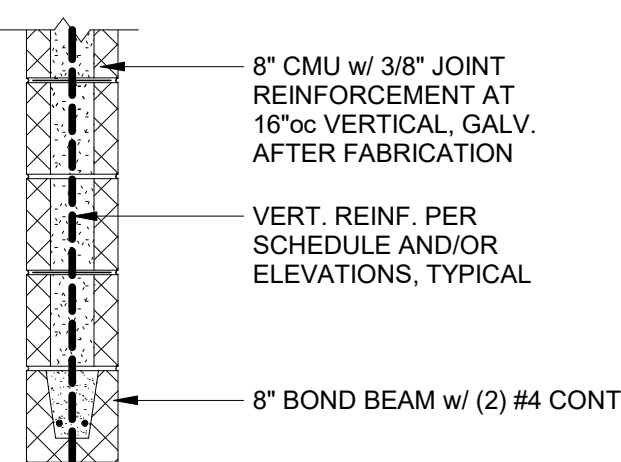
WOOD HEADER SCHEDULE			
MARK	HEADER SIZE	No. OF STUDS AT EACH END	HEADER DETAIL
H1	(2) 2x8	(2) JACK + (2) FULL HEIGHT	HEADER DETAIL 'A'
H2	(3) 1-3/4"x7-1/4" LVL	(2) JACK + (2) FULL HEIGHT	HEADER DETAIL 'C'
H3	(4) 2x12	(2) JACK + (3) FULL HEIGHT	HEADER DETAIL 'B'
H4	(3) 1-3/4"x11-1/4" LVL	(2) JACK + (3) FULL HEIGHT	HEADER DETAIL 'C'



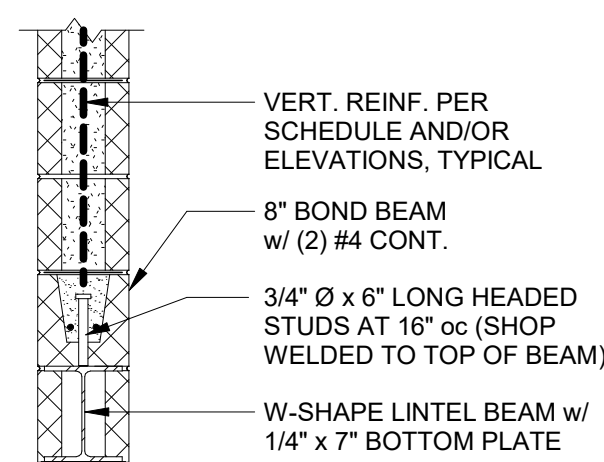
BEAM BEARING SECTION



TYPICAL BEAM AND PLATE LINTEL BEARING DETAIL



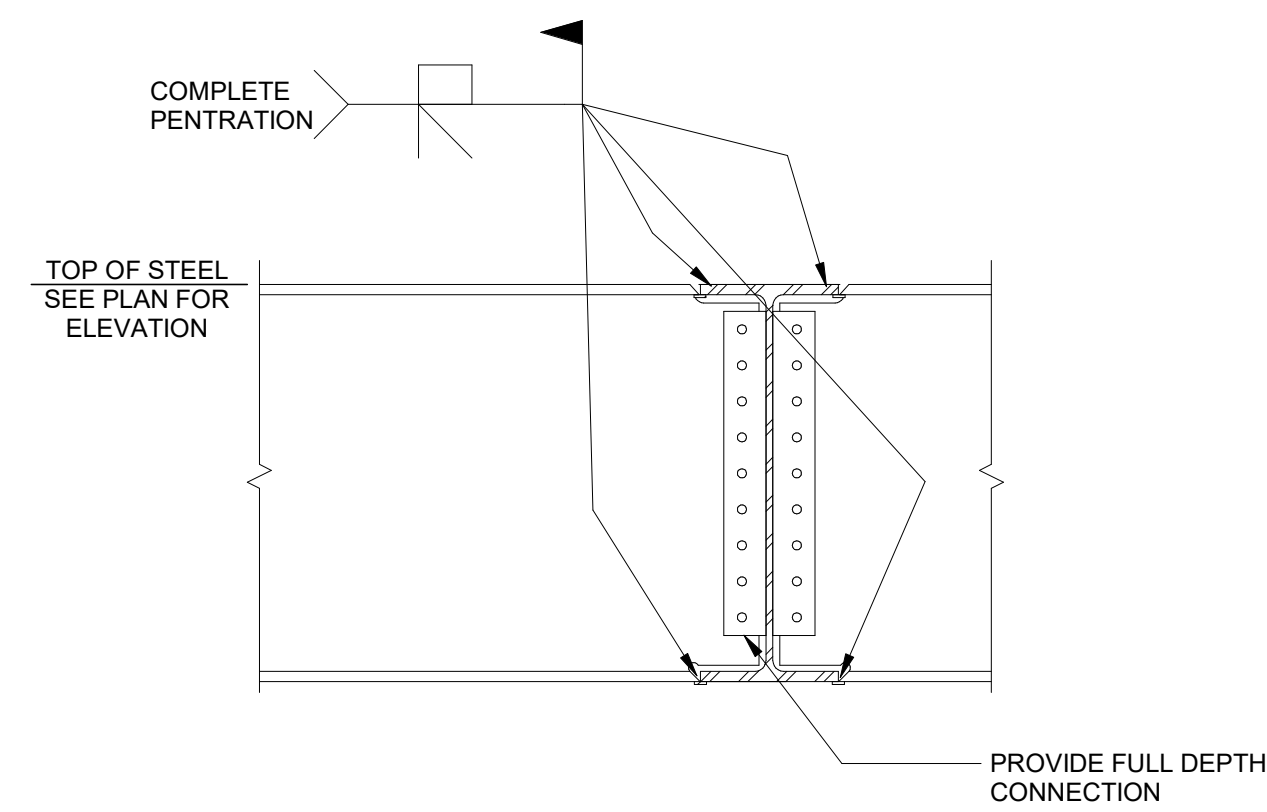
TYPICAL "BB8" DETAIL



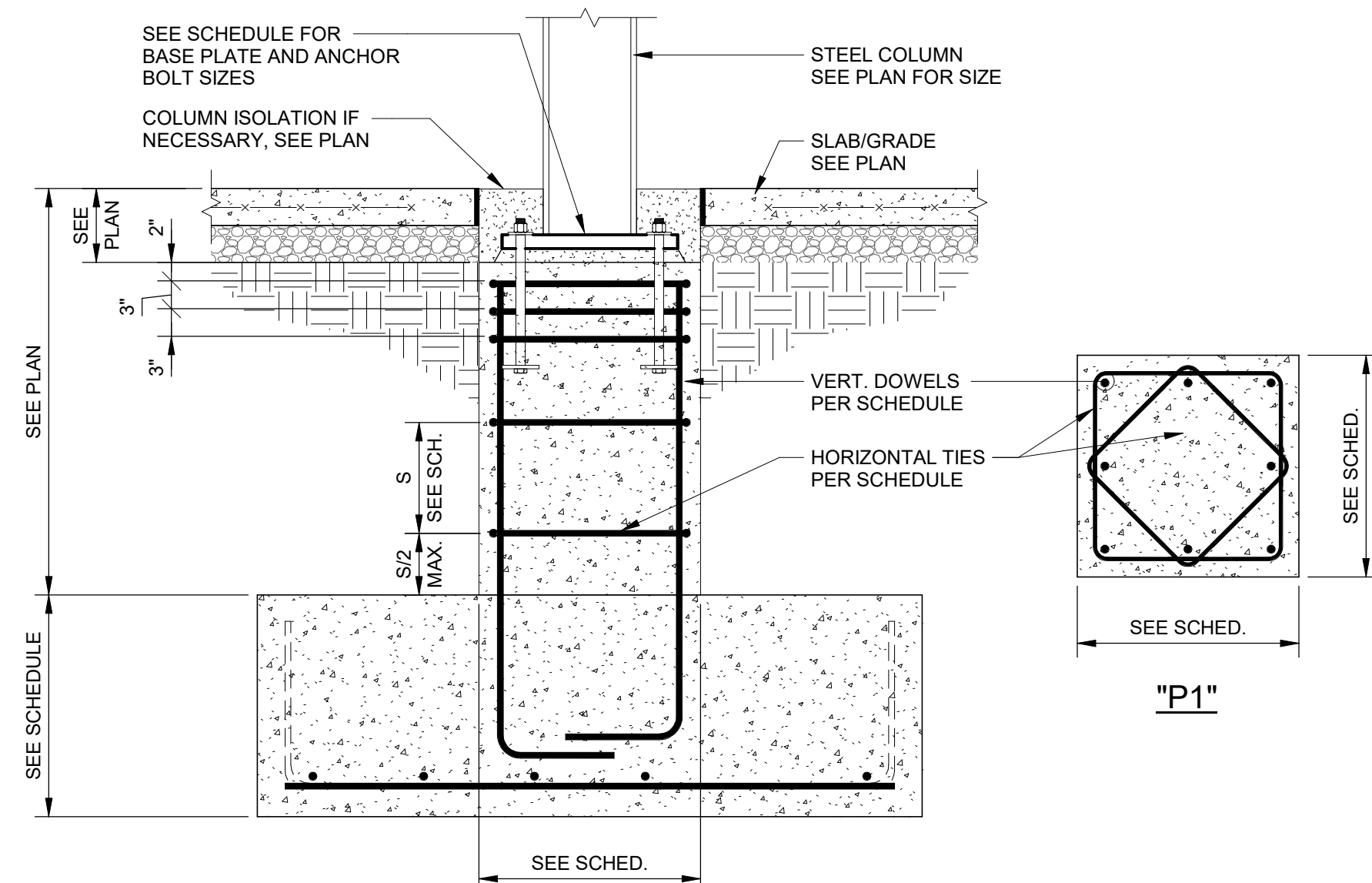
TYPICAL "LX" DETAIL

BEARING PLATE SCHEDULE				
MARK	MAX REACTION (KIPS)	W	T	No. OF GROUTED CELLS
BP1	20	8"	3/4"	2
BP2	30	12"	3/4"	2
BP3	50	16"	1 1/4"	3
BP4	60	16"	1 3/4"	4

NOTES:
1. ALL BEARING PLATES TO BE GALVANIZED
2. PAINT 1'-0" LENGTH AT BEAM END W/ ZINC RICH PRIMER
3. WELD NOT REQ'D WHERE SLAB IS PRESENT BOTH SIDES OF WALL & ROOF EXTEND THROUGH BEAM FLANGE
4. WHERE BEAM FLANGE IS LESS THAN 6", SPACE BOLTS OUTSIDE OF BEAM FLANGES AND INCREASE "W" AS NEEDED



TYPICAL BEAM MOMENT CONNECTION 'MC' DETAIL



TYPICAL CONCRETE PIER DETAIL

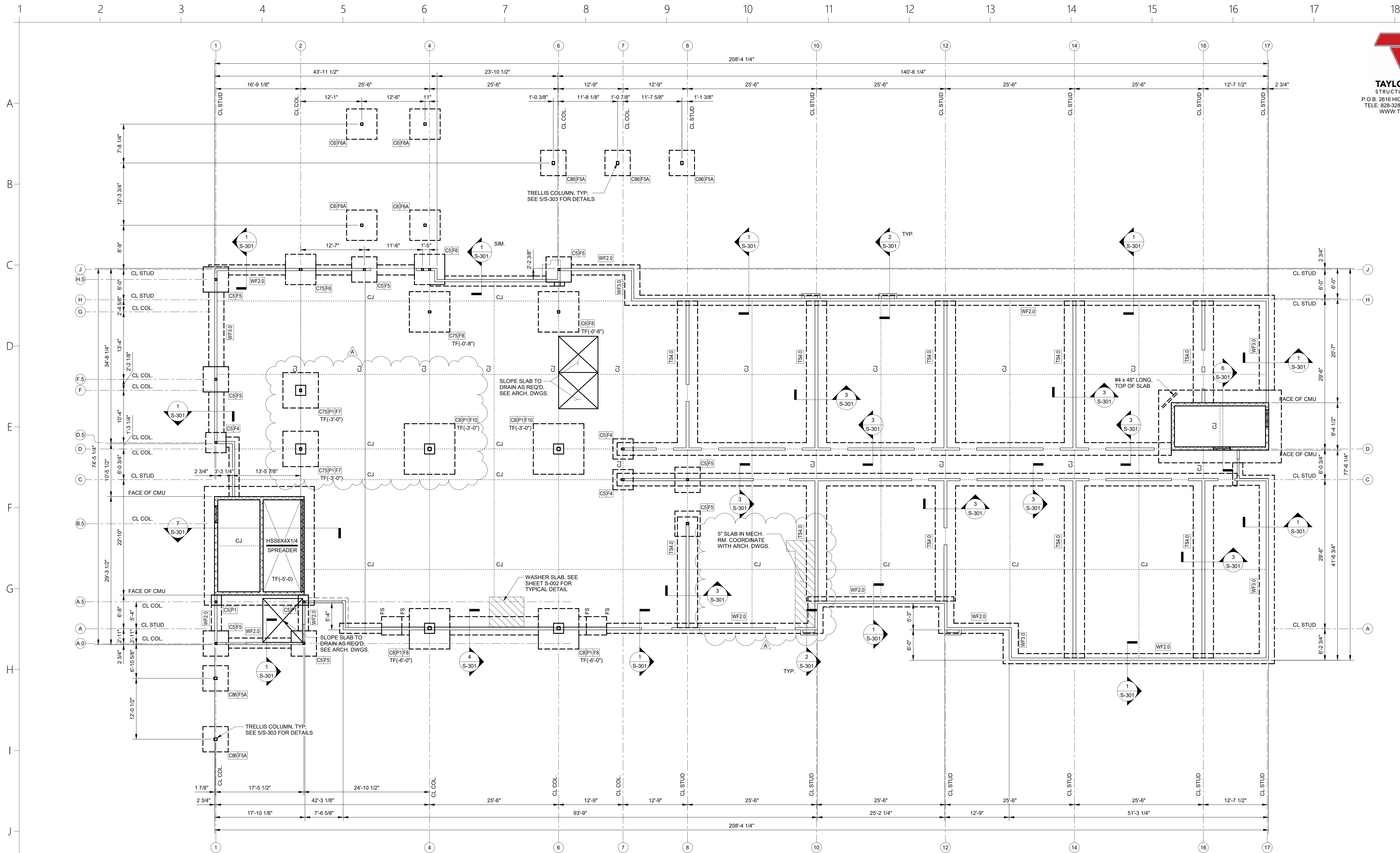
CONCRETE PIER SCHEDULE				
MARK	SIZE	DOWELS	VERTICALS	TIES
P1	24" x 24" PIER	(8) #6	(8) #6	#3 AT 8"oc

STEEL COLUMN SCHEDULE						
MARK	SIZE	BASEPLATE	CAP PLATE	ATTACHMENT	EMBED LENGTH	COMMENTS
C5	HSS5X5X3/8	3/4"x12"x12"	N/A	(4) 3/4"	9"	N/A
C6	HSS6X6X1/2	1"x14"x14"	PER DETAILS	(4) 1"	12"	N/A
C8	HSS8X8X1/2	1-1/4"x16"x16"	N/A	(4) 1"	12"	N/A
C75	HSS7X5X1/2	1"x16"x12"	N/A	(4) 1"	12"	N/A
C86	HSS8X6X3/8	1"x16"x16"	N/A	(4) 1"	12"	N/A

FOOTING SCHEDULE			
MARK	SIZE	BOTTOM REINFORCING	TOP REINFORCING
F4	4'-0" x 4'-0" x 16" THICK	(5) - #5 E.W.	N/A
F5	5'-0" x 5'-0" x 16" THICK	(6) - #5 E.W.	N/A
F5A	5'-0" x 5'-0" x 18" THICK	(6) - #5 E.W.	(6) - #5 E.W.
F6	6'-0" x 6'-0" x 18" THICK	(6) - #6 E.W.	N/A
F6A	6'-0" x 6'-0" x 20" THICK	(6) - #6 E.W.	(6) - #6 E.W.
F7	7'-0" x 7'-0" x 18" THICK	(7) - #6 E.W.	N/A
F8	8'-0" x 8'-0" x 24" THICK	(9) - #6 E.W.	N/A
F10	10'-0" x 10'-0" x 24" THICK	(8) - #6 E.W.	N/A

THICKENED SLAB SCHEDULE			
MARK	SIZE	CONT. BARS	TRANSVERSE BARS
TS2.5	2'-6" WIDE x 12" THICK	(3) #5 CONT.	#5 AT 16"oc
TS4.0	4'-0" WIDE x 12" THICK	(5) #5 CONT.	#5 AT 12"oc

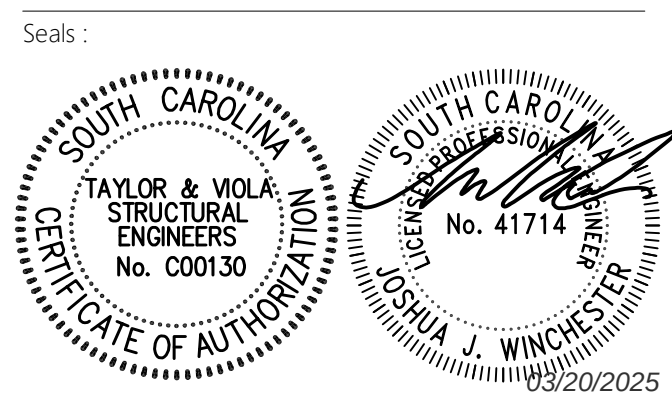
WALL FOOTING SCHEDULE			
MARK	SIZE	CONT. BARS	TRANSVERSE BARS
WF2.0	2'-0" WIDE x 12" THICK	(3) #5 CONT.	#5 AT 16"oc
WF3.0	3'-0" WIDE x 12" THICK	(3) #5 CONT.	#5 AT 16"oc



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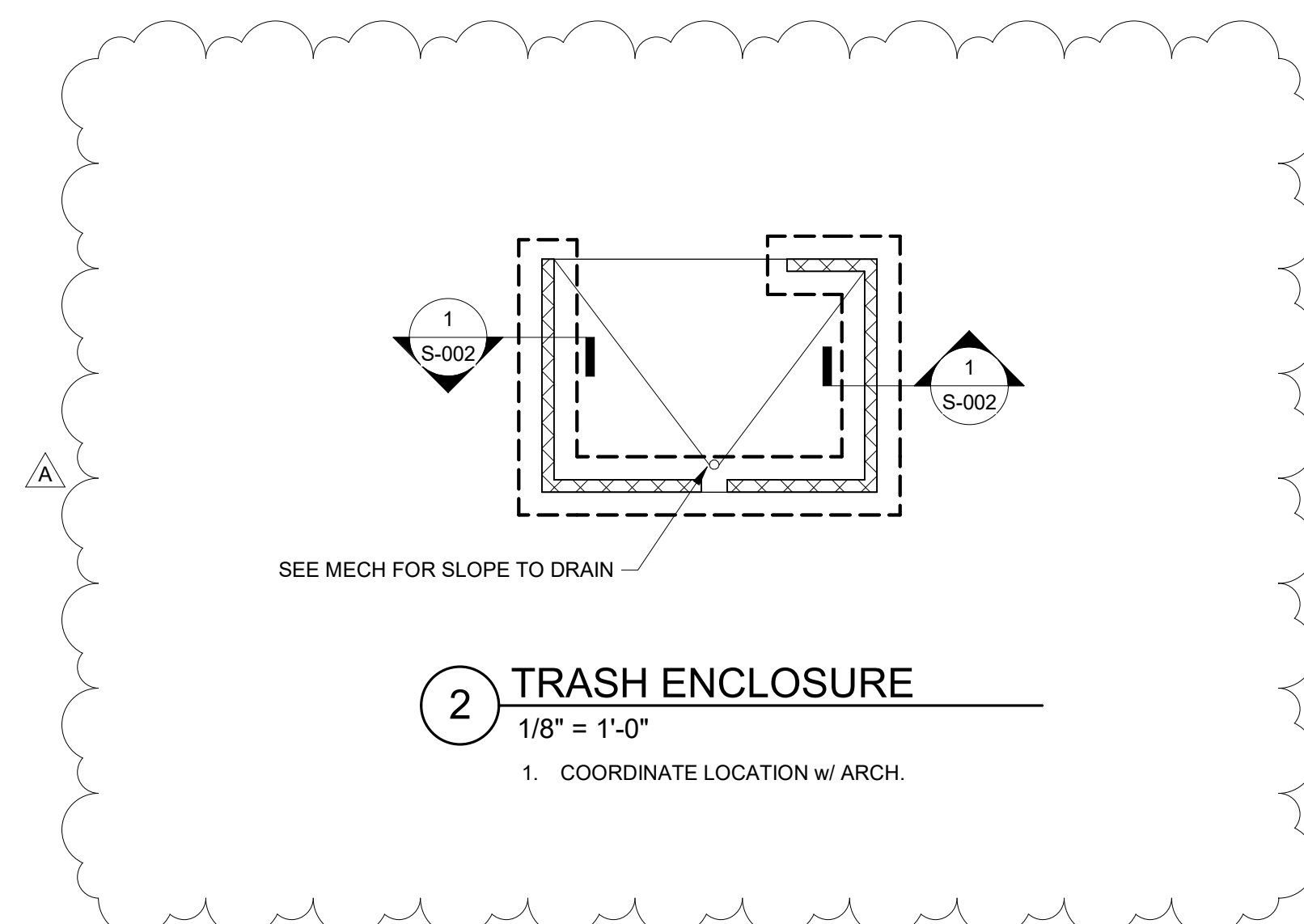
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Checked By: VDC
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S-101
Sheet Title:
FOUNDATION PLAN

1 FOUNDATION PLAN

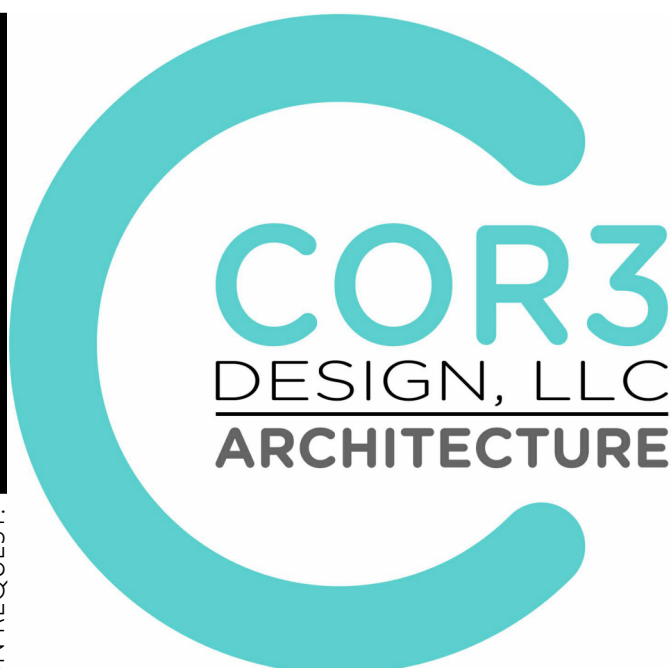
1/8" = 1'-0"

- TOP OF FINISHED FIRST FLOOR SLAB ON GRADE ELEVATION TO BE REFERENCE ELEVATION (0'-0") UNLESS OTHERWISE NOTED.
- SLAB ON GRADE TO BE 4" CONCRETE w/ 6x6 W1.4 x W1.4 W.W.F. AND SUBGRADE PREPARED PER SITE GEOTECHNICAL REPORT. VAPOR BARRIER SHALL BE PLACED AS REQUIRED BY ARCHITECT AND/OR GEOTECHNICAL ENGINEER WHERE MOISTURE SENSITIVE COVERINGS ARE PLACED ON TOP OF THE SLAB.
- TOP OF FOOTING ELEVATION TO BE TF(-1'-4") BELOW FINISHED SLAB ON GRADE ELEVATION UNLESS OTHERWISE NOTED ON PLAN.
- SEE SHEET S-202 FOR WOOD STUD WALL FRAMING (STUD SIZE AND SPACING REQ'D). ALL WOOD STUDS SHALL BE MINIMUM OF #2 SOUTHERN YELLOW PINE UNLESS OTHERWISE NOTED OR APPROVED BY EOR PRIOR TO ERECTION.
- "FX" INDICATES CONCRETE FOOTING; SEE FOOTING SCHEDULE ON SHEET S-002.
- "CX" INDICATES CONSTRUCTION / CONTROL JOINTS; SEE SHEET S-001 FOR TYPICAL SLAB DETAILS.
- "CJ" INDICATES STEEL COLUMN; SEE SHEET S-002 FOR STEEL COLUMN SCHEDULE.
- "FS" INDICATES CONCRETE FOOTING STEP; SEE SHEET S-001 FOR TYPICAL DETAIL.
- ALL DIMENSIONS TO FACE OF STUD UNLESS OTHERWISE NOTED. SEE ARCH DRAWINGS FOR DIMENSION AT ALL INTERIOR NON-LOAD BEARING WALLS.
- VERIFY TOP OF FOOTING ELEVATIONS w/ FINAL CIVIL DRAWINGS PRIOR TO PLACEMENT OF CONCRETE.
- SEE SHEETS S-202 THRU S2.04 FOR CMU WALL REINFORCING INFORMATION.
- ALL BEAM FRAMING TO BE SUPPORTED BY STUD POSTS CONTINUOUS TO TRANSFER BEAM OR FOUNDATION BELOW. PROVIDE (3) 2x6 STUDS FOR SUPPORT UNLESS OTHERWISE NOTED ON PLAN.
- COORDINATE FLOOR DRAIN LOCATIONS WITH PLUMBING/ARCHITECTURAL DRAWINGS. SLOPE SLAB TO DRAIN AS REQUIRED.





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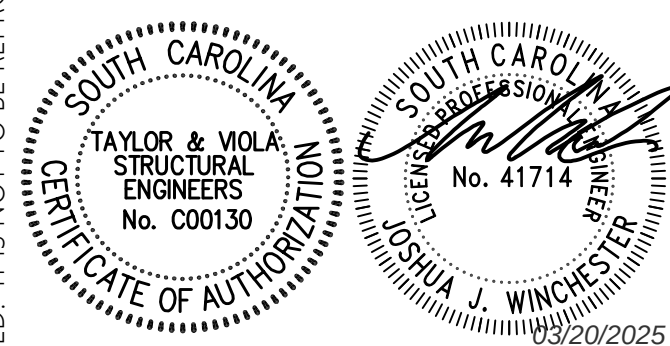
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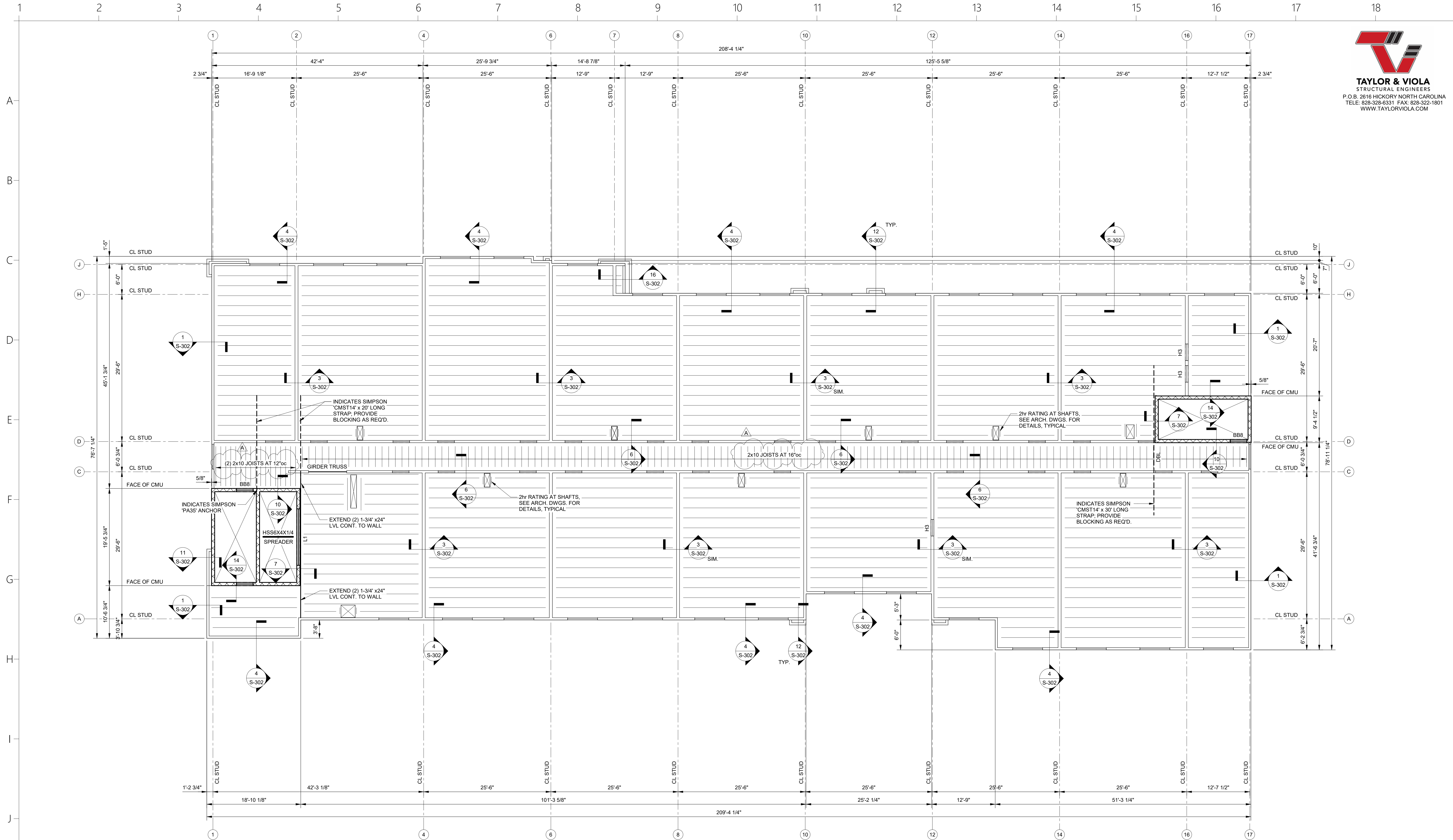
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SECOND LEVEL FRAMING PLAN

1 SECOND FLOOR FRAMING PLAN

1/8" = 1'-0"

- TOP OF 2ND FLOOR SUBFLOOR ELEVATION TO BE (+13'-10 1/2") ABOVE FINISHED FIRST FLOOR SLAB ON GRADE REFERENCE ELEVATION (0'-0") UNLESS OTHERWISE NOTED.
- TOP OF STEEL ELEVATIONS SHALL BE (-0'-3 3/4") BELOW TOP OF SUBFLOOR ELEVATION, TYPICAL UNLESS OTHERWISE MARKED TS(-X'-X").
- UNLESS OTHERWISE NOTED, FLOOR FRAMING TO BE ENGINEERED 33'-18" FLOOR TRUSSES AT 24"oc (MAX.). FLOOR TRUSS DESIGN IS A DEFERRED SUBMITTAL AND SHOP DRAWINGS INCLUDING DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED TO THE EOR AND ARCHITECT AND SHALL BE SEALED BY A LICENSED PROFESSIONAL ENGINEER IN THE STATE OF SOUTH CAROLINA.
- FLOOR DIAPHRAGM TO BE 3/4" T&G SHEATHING/SINGLE FLOOR GRADE; GLUE PER MANUFACTURER AND ATTACH WITH 10d NAILS AT 6"oc AT BOUNDARY & ALL PANEL EDGES AND 10d NAILS AT 12"oc IN THE FIELD. STRAPS NOTED ON PLAN TO BE NAILED PER MANUFACTURER TO BLOCKING OR FLOOR FRAMING. VERIFY MIN. BLOCKING OR FRAMING WIDTHS w/ STRAP MANUFACTURER.
- ALL STRUCTURAL WOOD FRAMING (BEAMS, HEADERS, ETC.) TO BE MINIMUM OF #2 SPRUCE-PINE-FIR OR #2 SOUTHERN YELLOW PINE UNLESS OTHERWISE NOTED OR APPROVED BY EOR PRIOR TO ERECTION.
- SEE SHEET S-2012 FOR WOOD STUD WALL FRAMING (STUD SIZE AND SPACING REQ'D). ALL WOOD STUDS SHALL BE MINIMUM OF #2 SOUTHERN YELLOW PINE UNLESS OTHERWISE NOTED OR APPROVED BY EOR PRIOR TO ERECTION.
- "HX" INDICATES WOOD HEADERS; SEE HEADER SCHEDULE ON SHEET S-002 FOR DETAILS. ALL HEADERS NOT INDICATED ON PLAN TO BE MINIMUM OF "H1".
- "BBX" AND "LX" INDICATES CMU BOND BEAM AND STEEL LINTELS RESPECTIVELY; SEE LINTEL SCHEDULE ON SHEET S-002 FOR DETAILS.
- "BPX" INDICATES BEAM BEARING PLATE; SEE SCHEDULE ON SHEET S-002 FOR DETAILS.
- COORDINATE FLOOR OPENINGS w/ ARCHITECTURAL AND MECHANICAL DRAWINGS.
- ALL DIMENSIONS TO FACE OF STUD UNLESS OTHERWISE NOTED. SEE ARCH DRAWINGS FOR DIMENSION AT ALL INTERIOR NON LOAD BEARING WALLS.
- SEE SHEETS S-202 THRU S2-04 FOR CMU WALL REINFORCING INFORMATION.
- ALL BEAM FRAMING TO BE SUPPORTED BY STUD POSTS CONTINUOUS TO TRANSFER BEAM OR FOUNDATION BELOW. PROVIDE (3) 2x6 STUDS FOR SUPPORT UNLESS OTHERWISE NOTED ON PLAN.



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STRUCTURAL ENGINEERS
No. 000130
JONATHAN J. WINCHESTER
No. 41714
03/20/2025

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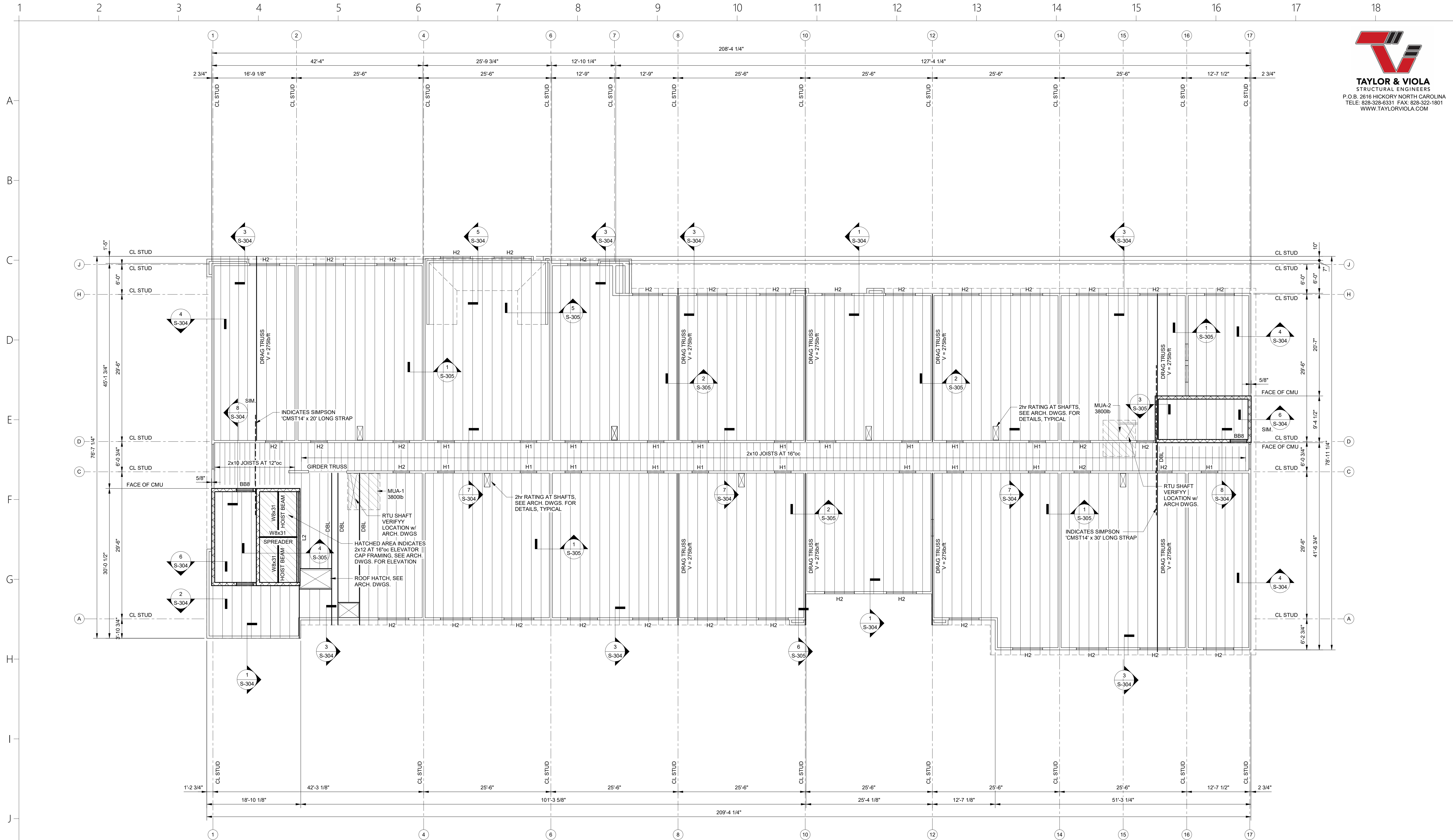
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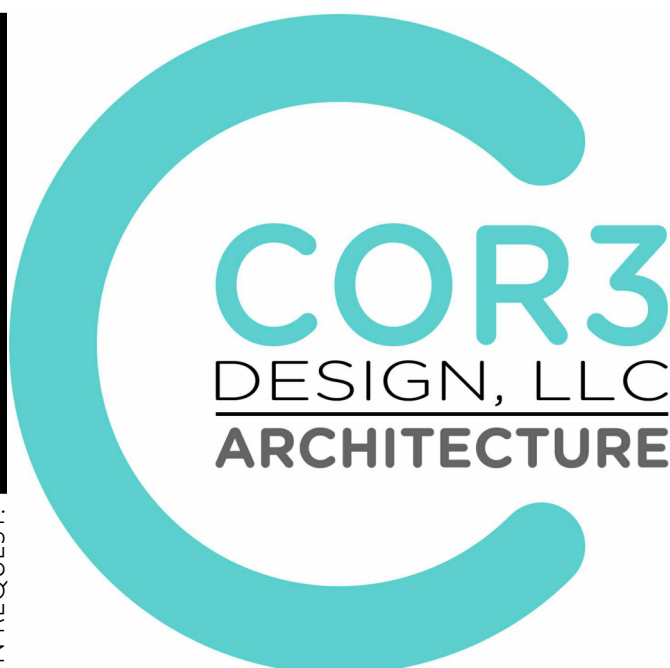
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1 ROOF FRAMING PLAN
1/8" = 1'-0"

- INTERIOR TRUSS BEARING ELEVATION TO BE (+44'-1 1/2") AND EXTERIOR TRUSS BEARING ELEVATION TO BE (+44'-6 1/2") ABOVE FINISHED FIRST FLOOR SLAB ON GRADE REFERENCE ELEVATION (0'-0") UNLESS OTHERWISE NOTED ON PLAN OR IN SECTIONS.
- UNLESS OTHERWISE NOTED, ROOF FRAMING TO BE WOOD TRUSSES AT 24"oc (MAX). ROOF TRUSS DESIGN IS A DEFERRED SUBMITTAL AND SHOP DRAWINGS INCLUDING DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED TO THE EOR AND ARCHITECT AND SHALL BE SEALED BY A LICENSED PROFESSIONAL ENGINEER IN THE STATE OF SOUTH CAROLINA.
- ROOF DIAPHRAGM TO BE 3/4" T&G SHEATHING/SINGLE FLOOR GRADE; GLUE PER MANUFACTURER AND ATTACH WITH 10d NAILS AT 6"oc AT BOUNDARY & ALL PANEL EDGES AND 10d NAILS AT 12"oc IN THE FIELD. STRAPS NOTED ON PLAN TO BE NAILED PER MANUFACTURER TO BLOCKING OR FLOOR FRAMING. VERIFY MIN. BLOCKING OR FRAMING WIDTHS w/ STRAP MANUFACTURER.
- ROOF DRAG TRUSSES TO BE DESIGNED FOR SHEAR LOADING NOTED ON PLAN (ASD - SERVICE LOADS). ALL DRAG TRUSSES WITHOUT SHEAR LOADINGS NOTED TO BE DESIGNED FOR MINIMUM OF 100lb/ft.
- ALL STRUCTURAL WOOD FRAMING (BEAMS, HEADERS, ETC.) TO BE MINIMUM OF #2 SPRUCE-PINE-FIR OR #2 SOUTHERN YELLOW PINE UNLESS OTHERWISE NOTED OR APPROVED BY EOR PRIOR TO ERECTION.
- SEE SHEET S-202 FOR WOOD STUD WALL FRAMING (STUD SIZE AND SPACING REQ'D). ALL WOOD STUDS SHALL BE MINIMUM OF #2 SOUTHERN YELLOW PINE UNLESS OTHERWISE NOTED OR APPROVED BY EOR PRIOR TO ERECTION.
- "HX" INDICATES WOOD HEADERS; SEE HEADER SCHEDULE ON SHEET S-002 FOR DETAILS. ALL HEADERS NOT INDICATED ON PLAN TO BE MINIMUM OF "H1".
- "BBX" AND "LX" INDICATES CMU BOND BEAM AND STEEL LINTELS RESPECTIVELY; SEE LINTEL SCHEDULE ON SHEET S-002 FOR DETAILS.
- COORDINATE ROOF OPENINGS w/ ARCHITECTURAL AND MECHANICAL DRAWINGS. ROOF TRUSS DESIGN TO ALSO ACCOUNT FOR MECHANICAL EQUIPMENT AS NECESSARY. SEE ARCH/MECH DWGS. FOR WEIGHTS AND LOCATIONS.
- ALL DIMENSIONS TO FACE OF STUD UNLESS OTHERWISE NOTED. SEE ARCH DRAWINGS FOR DIMENSION AT ALL INTERIOR NON LOAD BEARING WALLS.
- SEE SHEETS S-202 THRU S2-04 FOR CMU WALL REINFORCING INFORMATION.
- ALL BEAM FRAMING TO BE SUPPORTED BY STUD POSTS CONTINUOUS TO TRANSFER BEAM OR FOUNDATION BELOW. PROVIDE (3) 2x6 STUDS FOR SUPPORT UNLESS OTHERWISE NOTED ON PLAN.
- PRE-ENGINEERED ROOF SCREEN ARE DEFERRED SUBMITTALS AND SHOP DRAWINGS INCLUDING DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED TO THE EOR AND ARCHITECT PRIOR TO CONSTRUCTION. SEALED BY A LICENSED PROFESSIONAL IN THE STATE OF SOUTH CAROLINA FOR REVIEW TO ENSURE ADEQUACY OF THE SUPPORTING STRUCTURE.



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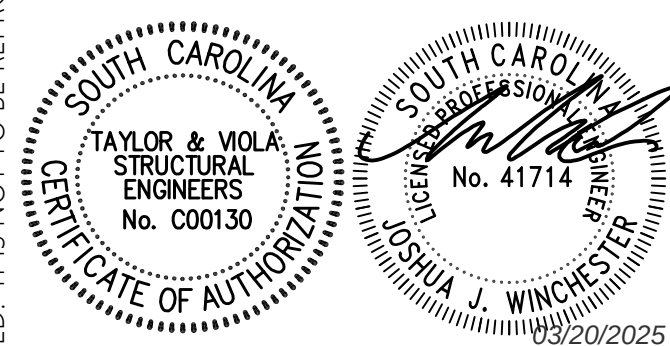
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Client Logo:



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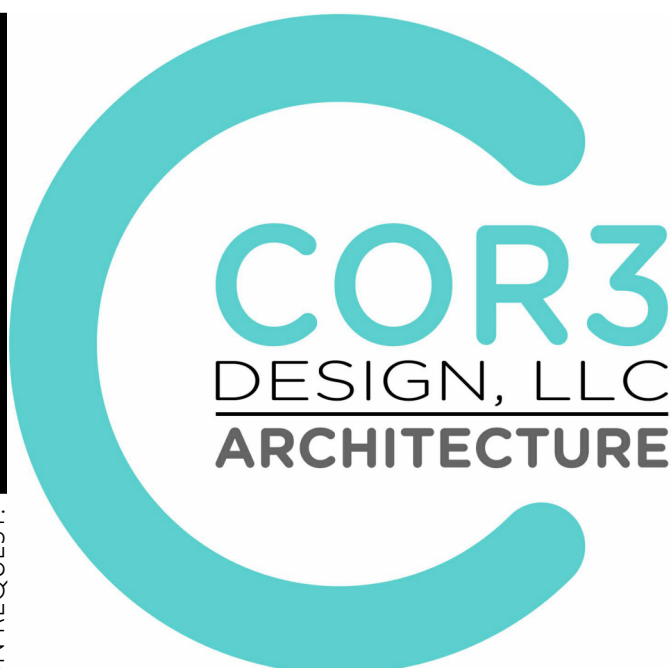
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ROOF FRAMING PLAN



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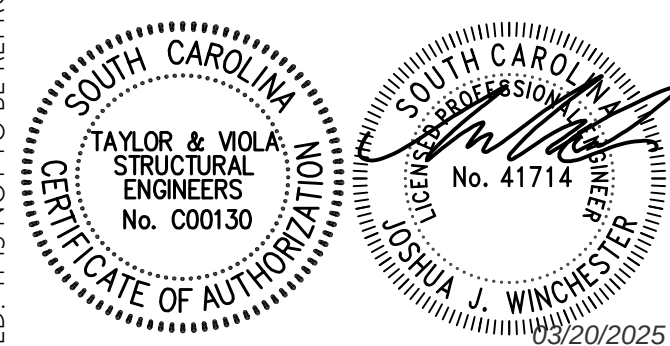
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Sheet Title:

WALL DESIGNATION PLAN

1 WALL DESIGNATION PLAN

1/8" = 1'-0"

1. "LB6-X" INDICATES 6" WOOD LOAD-BEARING WALL. SEE SCHEDULE ON SHEET S-202 FOR SIZE AND SPACING.
2. "SWX" INDICATES 6" WOOD SHEAR WALL. SEE SCHEDULE ON SHEET S-202. PLACE HOLDDOWNS PER SCHEDULE AS INDICATED ON PLAN UNLESS OTHERWISE NOTED.
3. "CMU8-X" INDICATES 8" CMU WALL. SEE SCHEDULE ON SHEET S-202 FOR REINFORCING SIZE AND SPACING. REFERENCE SHEETS S-203 AND S2.04 FOR TYPICAL REINFORCEMENT ELEVATIONS.



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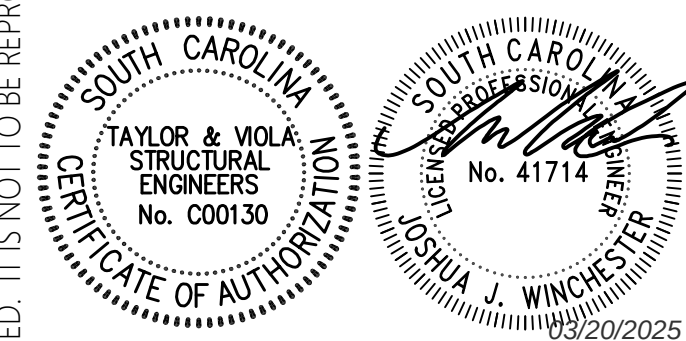
ELECTRICAL
Matrix Engineering, INC
910 S Pine Street
Spartanburg, SC 29302
864.583.6274

Project Title:
HOME 2 SUITES
GREENWOOD SC
475 HOSPITALITY BLVD,
GREENWOOD, SC 29649

Client Logo:



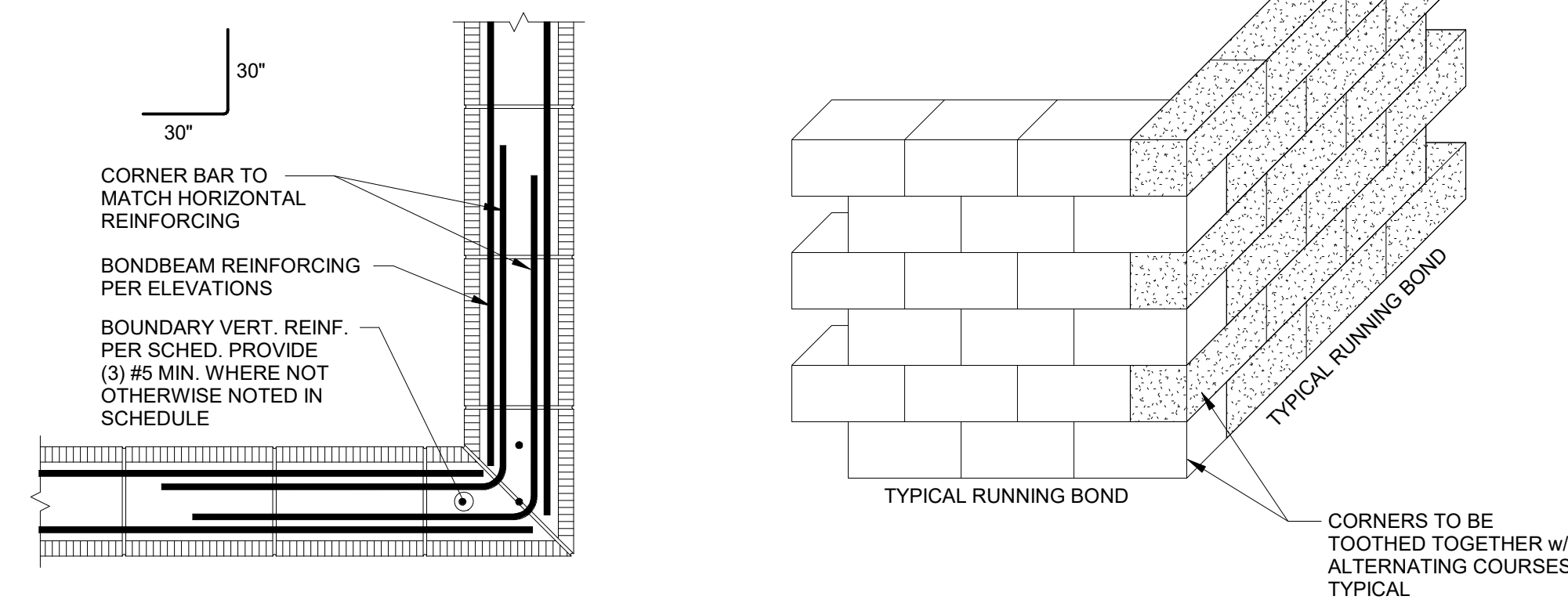
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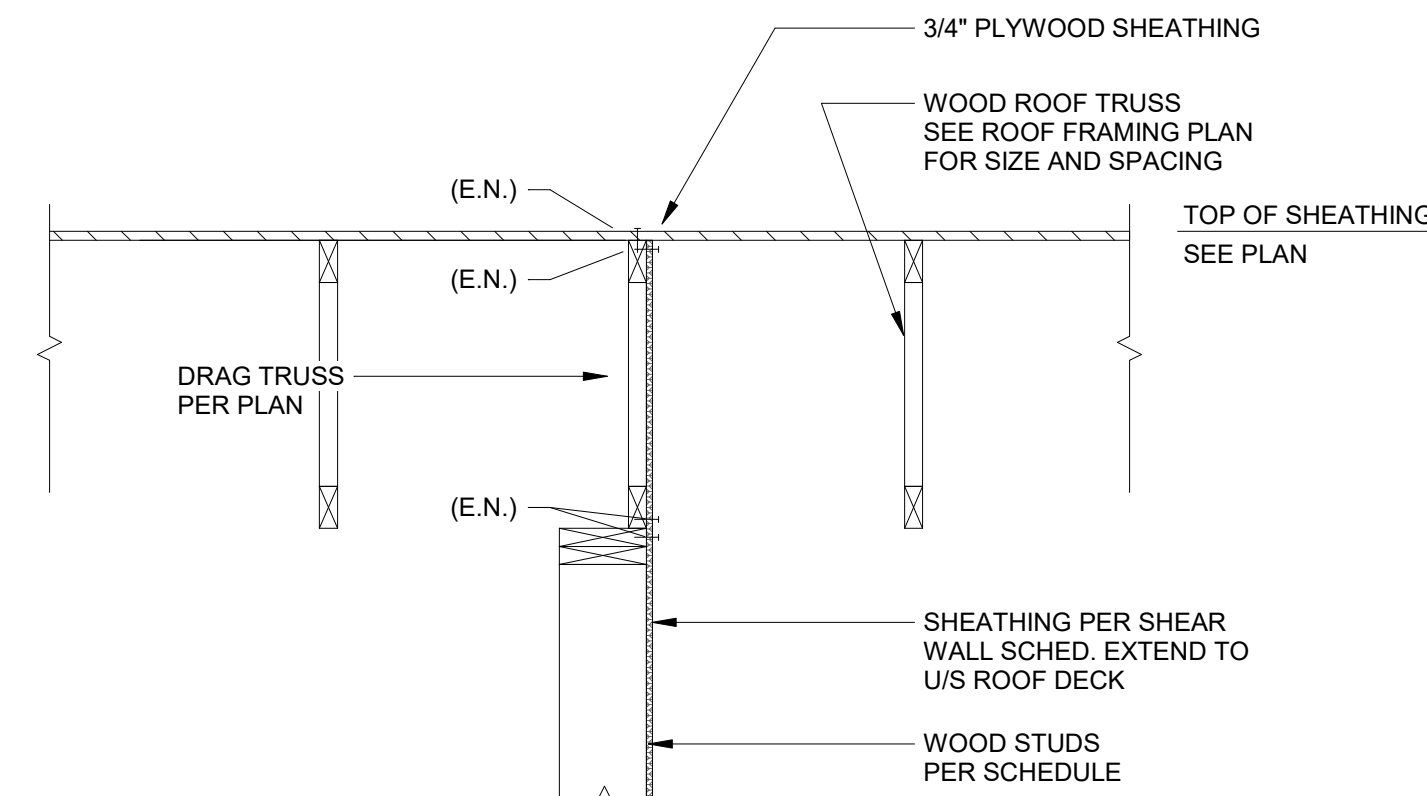
Project Number:
23112
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CD
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10.16.2024
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VDC
Sheet Number:
S-202

Sheet Title:
WOOD SHEAR WALL SCHEDULE
AND TYPICAL DETAILS

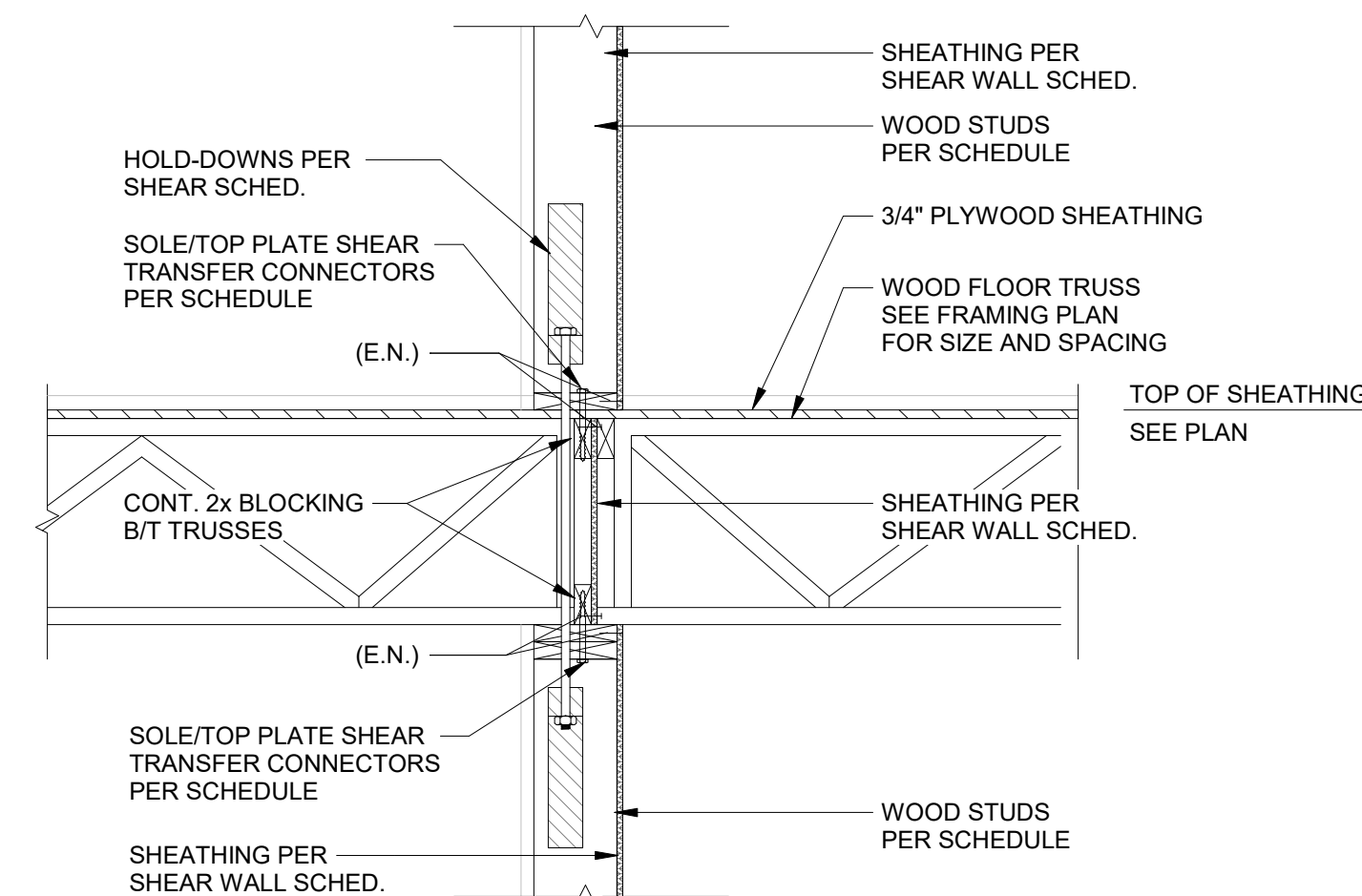


WALL CORNER DETAIL AT BOND BEAM

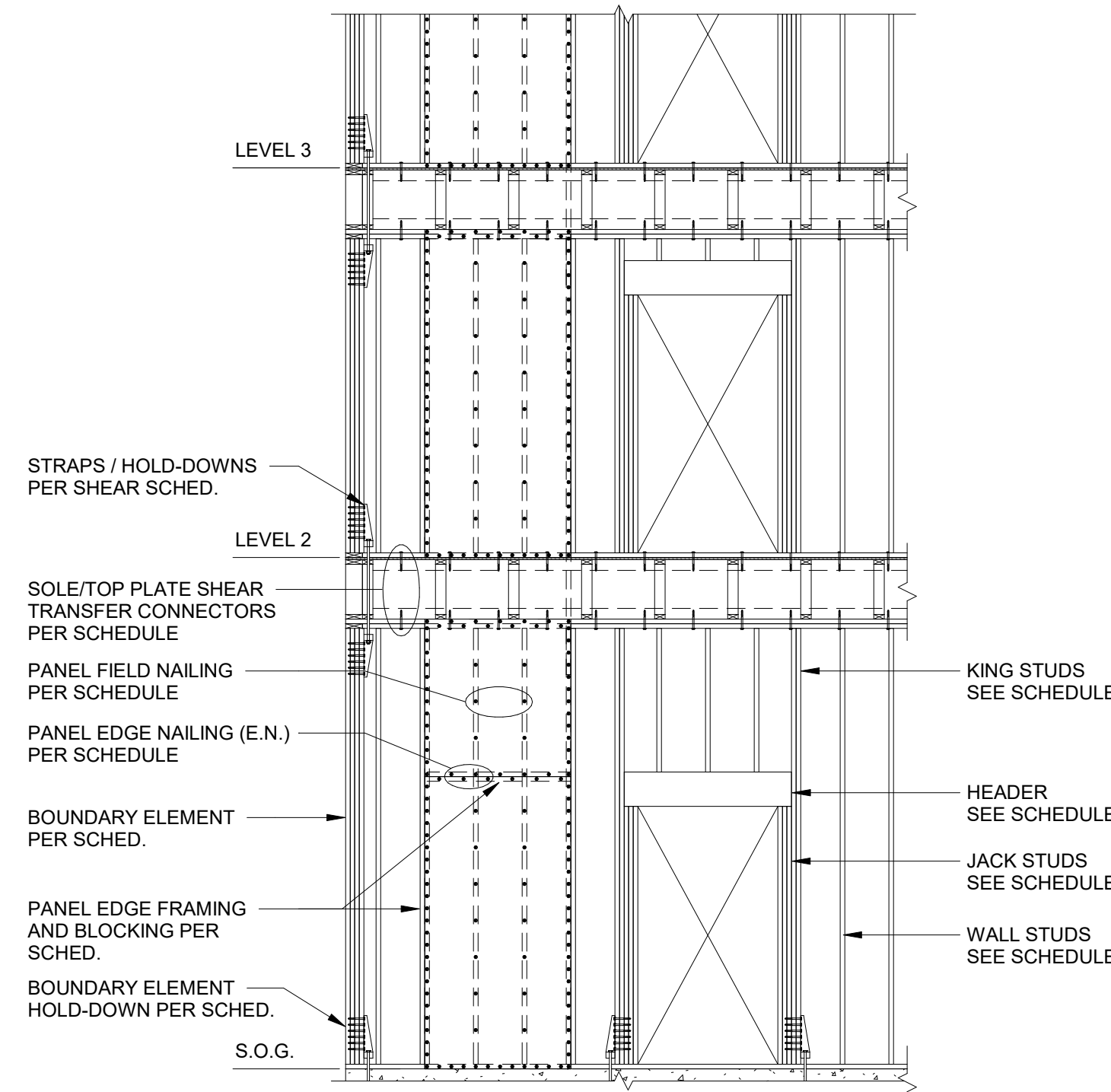
TYPICAL RUNNING BOND



SHEAR WALL FRAMING AT ROOF



SHEAR WALL FRAMING AT TYPICAL FLOOR LEVELS
(HOLDOWN DETAIL)



TYPICAL WOOD SHEAR WALL

WOOD SHEAR WALL FRAMING SCHEDULE

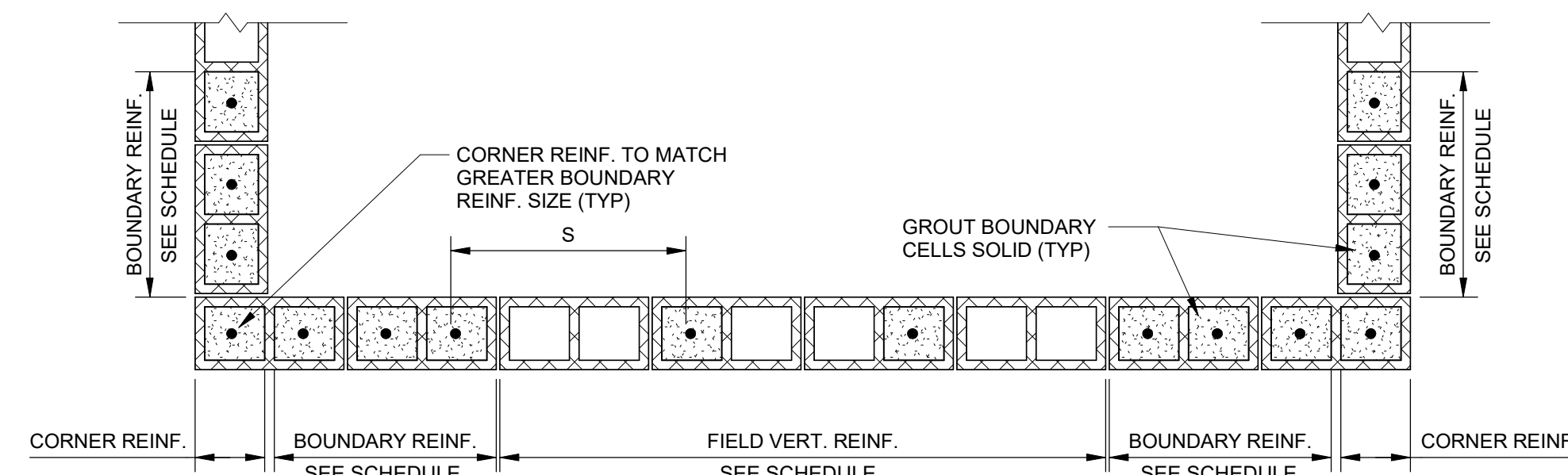
PLAN NOTATION	SHEATHING ²	EDGE NAILING	FIELD ⁶ NAILING	WIDTH OF NAILED FACE ⁷ FOR PANEL EDGE FRAMING/BLOCKING	BOUNDARY ELEMENT	BOUNDARY ELEMENT ⁸ HOLD-DOWN ANCHOR AT GRADE	INTERMEDIATE ANCHORS AT GRADE ⁴	STRAP/HOLD-DOWNS	SOLE/TOP PLATE SHEAR TRANSFER CONNECTORS	ROOF DRAG TRUSS TO TOP PLATE
SW6	1/2" (15/32") ONE FACE	10d @ 6"oc	10d @ 6"oc	2" NOMINAL	(3) 2x TO MATCH STUD	(1) SIMPSON HDU4-SDS2.5 w/ 5/8" Ø THREADED ROD OR SIMPSON PAB w/ 12" EMBED.	1/2" Ø ANCHOR BOLTS w/ 7" EMBED AT 48"oc	(1) SIMPSON HDU4-SDS2.5 w/ 5/8" Ø THREADED ROD	(1) SIMPSON 'SDWS22600DB' AT 16"oc	(1) SIMPSON A34 AT 16"oc
SW4	1/2" (15/32") ONE FACE	10d @ 4"oc	10d @ 6"oc	2" NOMINAL	(4) 2x TO MATCH STUD	(1) SIMPSON HDU8-SDS2.5 w/ 7/8" Ø THREADED ROD OR SIMPSON PAB w/ 20" EMBED.	1/2" Ø ANCHOR BOLTS w/ 7" EMBED AT 48"oc	(1) SIMPSON HDU5-SDS2.5 w/ 5/8" Ø THREADED ROD	(1) SIMPSON 'SDWS22600DB' AT 12"oc	(1) SIMPSON A35 AT 16"oc
SW3	1/2" (15/32") ONE FACE	10d @ 3"oc	10d @ 6"oc	3" NOM. OR (2) 2x CONNECTED W/ (2) 16d STAGGERED AT 3"oc	(5) 2x TO MATCH STUD	(1) SIMPSON HDU14-SDS2.5 w/ 1" Ø THREADED ROD OR SIMPSON PAB w/ 20" EMBED.	1/2" Ø ANCHOR BOLTS w/ 7" EMBED AT 32"oc	(1) SIMPSON HDU8-SDS2.5 w/ 7/8" Ø THREADED ROD	(1) SIMPSON 'SDWS22600DB' AT 8"oc	(1) SIMPSON HGA10 w/ 3" SDS AT 16"oc
SW2	1/2" (15/32") ONE FACE	10d @ 2"oc	10d @ 6"oc	3" NOM. OR (2) 2x CONNECTED W/ (2) 16d STAGGERED AT 3"oc	(6) 2x TO MATCH STUD	(1) SIMPSON HDU14-SDS2.5 w/ 1" Ø THREADED ROD OR SIMPSON PAB w/ 20" EMBED.	1/2" Ø ANCHOR BOLTS w/ 7" EMBED AT 16"oc	(1) SIMPSON HDU11-SDS2.5 w/ 1" Ø THREADED ROD	(1) SIMPSON 'SDWS22600DB' AT 6"oc	(1) SIMPSON HGA10 w/ 3" SDS AT 12"oc

- ALL WOOD STUDS AND BLOCKING IN SHEAR WALLS SHALL BE A MINIMUM OF #2 SOUTHERN PINE UNLESS OTHERWISE APPROVED BY EOR PRIOR TO INSTALLATION.
- ALL WOOD PANELS SHALL BE ZIP SYSTEM WALL SHEATHING (**STRUCTURAL 1 RATED OSB**). SEE ARCH. DWGS. FOR ADDITIONAL DETAILS.
- WHERE SPECIFIED NAIL SPACING 3" OR LESS AT PANEL EDGES, EDGE NAILING FOR ADJACENT PANELS SHALL BE STAGGERED.
- FOUNDATION ANCHOR BOLTS SHALL HAVE A STEEL PLATE WASHER UNDER EACH NUT NOT LESS THAN 0.229"x3"x3".
- PROVIDE SOLID BLOCKING UNDER ALL SHEAR WALL BOUNDARY ELEMENTS FOR FULL BEARING OF BOUNDARY ELEMENT. SEE WOOD STUD WALL FRAMING SCHEDULE THIS SHEET FOR STUD SIZE AND SPACING.
- WHERE STUDS ARE SPACED LESS THAN 24"oc, THE MAXIMUM NAIL SPACING IN PANEL FIELD MAY BE 12"oc.
- ALL SHEAR WALLS SHALL BE BLOCKED AT PANEL EDGES WHERE PANELS ARE INTERRUPTED BETWEEN FLOOR LEVELS. BLOCKING SHALL BE OF MINIMUM WIDTH PER SCHEDULE AND PANELS SHALL BE NAILED AT EDGES INTO BLOCKING PER SCHEDULE. SEE DETAILS GENERAL NOTES FOR GENERIC SHEAR WALL COMPONENTS.
- HOLD-DOWNS AT WALL SEGMENT ENDS SHALL BE AS LISTED IN SCHEDULE TYPICAL UNLESS NOTED OTHERWISE IN PLAN. HOLD-DOWNS SHALL BE PROVIDED AT EACH END OF EACH SHEAR WALL AND AT EACH SIDE OF EACH OPENING.

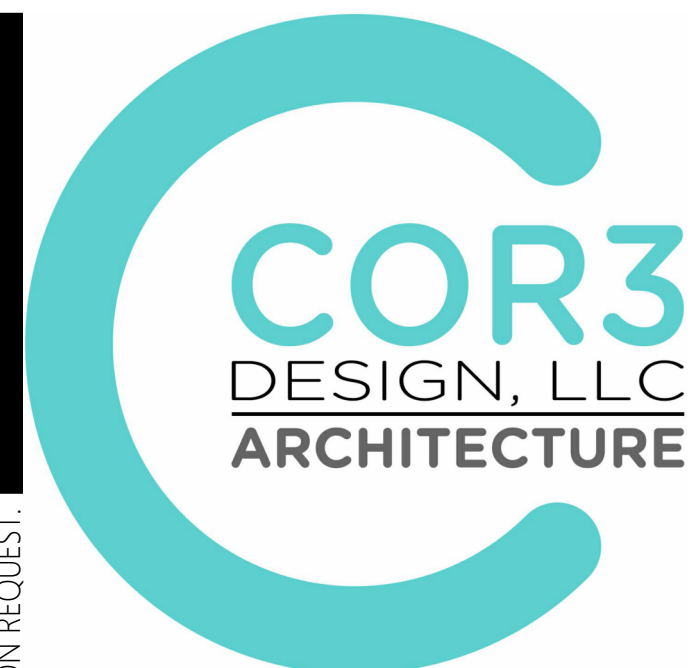
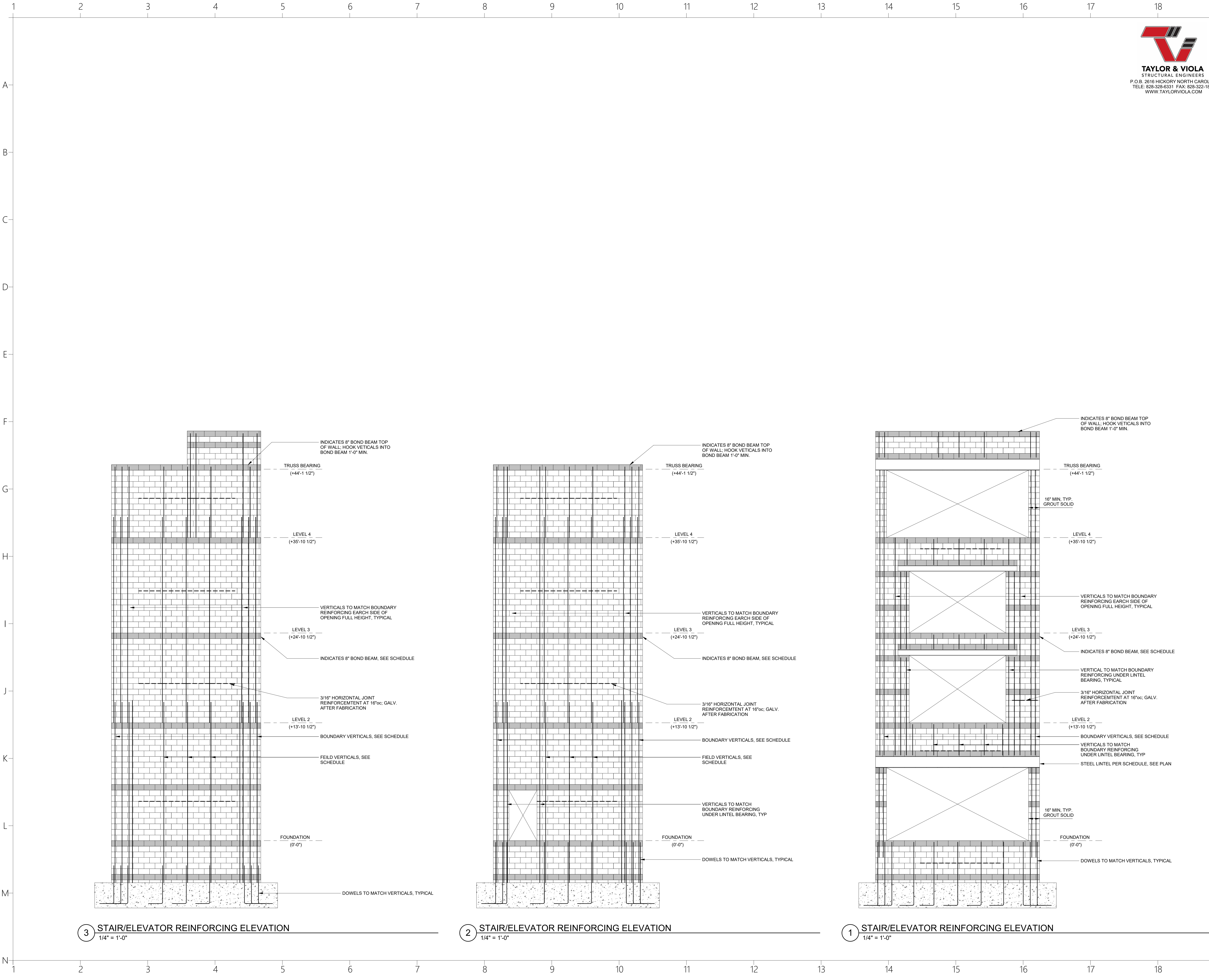
WOOD STUD WALL FRAMING SCHEDULE

LOCATION	INTERIOR LOAD-BEARING WALLS (LB6-1)	CORRIDOR WALLS (LB6-2)	EXTERIOR (END) LOAD-BEARING WALLS (LB6-3)	EXTERIOR (SIDE) LOAD BEARING WALLS (LB6-4)
4th FL - ROOF	2x6 STUDS AT 16"oc	2x6 STUDS AT 16"oc	2x6 STUDS AT 16"oc	2x6 STUDS AT 16"oc
3rd FL - 4th FL	2x6 STUDS AT 16"oc	2x6 STUDS AT 16"oc	2x6 STUDS AT 16"oc	2x6 STUDS AT 16"oc
2nd FL - 3rd FL	(2) 2x6 STUDS AT 16"oc	(2) 2x6 STUDS AT 16"oc	(2) 2x6 STUDS AT 16"oc	2x6 STUDS AT 16"oc
1st FL - 2nd FL	(2) 2x6 STUDS AT 16"oc	(2) 2x6 STUDS AT 16"oc	(2) 2x6 STUDS AT 16"oc	(2) 2x6 STUDS AT 16"oc

- ALL WOOD STUDS SHALL HAVE BLOCKING AT STUD MID-HEIGHT OR AT 4'-0"oc FOR WALLS TALLER THAN 10'-0"
- ALL WOOD STUDS AND BLOCKING TO BE MIN. OF SOUTHERN YELLOW PINE #2 UNLESS OTHERWISE NOTED. ANY SUBSTITUTION MUST BE APPROVED BY EOR PRIOR TO ERECTION.
- ALL EXTERIOR WOOD SHEATHING PANELS SHALL BE 1/2" (MIN.) STRUCTURAL SHEATHING GRADE APA PLYWOOD OR OSB. SEE ARCH. DWGS. FOR ADDITIONAL DETAILS.
- ALL EXTERIOR SHEATHING SHALL BE ATTACHED USING 10d NAILS w/ 6"oc EDGE SPACING & 12"oc FIELD SPACING UNLESS OTHERWISE NOTED IN WOOD SHEAR WALL FRAMING SCHEDULE.



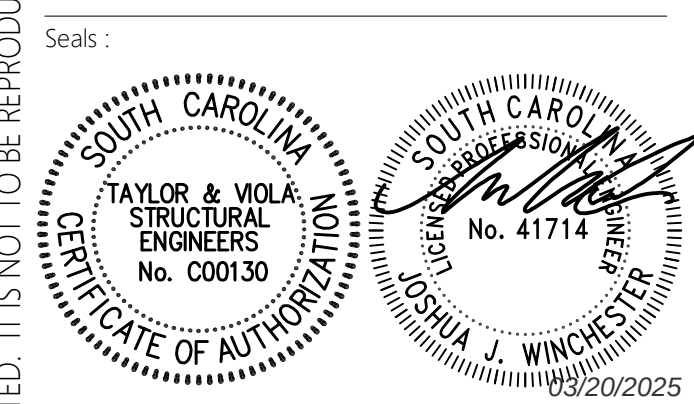
- WALLS SHALL BE REINFORCED AS INDICATED IN MASONRY WALL SCHEDULES. SEE ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATION OF ALL OPENINGS. GROUT FILL ALL CMU CELLS SOLID WHERE REINFORCING OCCURS.
- BOUNDARY VERTICALS TO OCCUR AT FIRST UNINTERRUPTED CELL FULL HEIGHT OR END OF WALL. BEARING PLATES MAY NOT INTERRUPT BOUNDARY VERTICALS. COORDINATE w/ BEARING PLATE SCHEDULE AND PROVIDE ADDITIONAL VERTICALS FOR EACH GROUTED CELL.
- HORIZONTAL REINFORCING SHALL SPAN CONTINUOUS w/ BONDED CORNERS AND SHALL HOOK AROUND VERTICAL REINFORCING AT ALL TERMINATIONS. SPACE REINFORCING SO THAT MAXIMUM O/C SPACING DOES NOT EXCEED VALUES SHOWN IN SCHEDULE AND SO THAT REINFORCING SPANS CONTINUOUS BETWEEN LAP SPICES.
- UNLESS OTHERWISE NOTED IN REINF. SCHEDULE, INSTALL (2) #4 HORIZONTAL BAR IN BONDS AT THE TOP AND BOTTOM OF WALL OPENINGS, EXTEND BEYOND THE VERTICAL REINFORCING ADJACENT TO THE OPENING 48 BAR DIAMETERS OR 24" MINIMUM.
- INSTALL (1) #4 (MIN.) AT THE FIRST OPEN CELL ADJACENT TO AN OPENING AND FIRST CELL BEYOND END OF LINTEL. DOWEL AT TOP AND BOTTOM INTO ADJACENT LEVEL BOND BEAMS AND/OR FOUNDATION.
- UNLESS MORE STRINGENT REINFORCING IS REQUIRED PER MASONRY REINFORCING SCHEDULE, HORIZONTAL REINFORCING SHALL CONSIST OF A BOND BEAM w/ (2) #4 CONTINUOUS UNDER EACH FLOOR AND ROOF PLANK w/ 3/16" HORIZONTAL JOINT REINFORCING AT 16"oc (GALV. AFTER FABRICATION) BETWEEN FLOORS.
- VERTICAL REINFORCING SHALL BE PROVIDED AT THE ENDS OF ALL WALLS AND IN THE FIRST CELL EACH SIDE OF ALL MASONRY CONTROL JOINTS. PROVIDE A MINIMUM OF (1) #6 VERTICAL BAR UNLESS OTHERWISE NOTED IN THE MASONRY REINFORCEMENT SCHEDULE.
- INSTALL ONE VERTICAL CORNER BAR AT ENDS OF ALL WALLS. TYPICAL CORNER REINFORCEMENT SHALL MATCH THE GREATER BAR DIAMETER IF TWO SEPARATE VERTICAL BOUNDARY BAR SIZES OCCUR IN ADJACENT WALLS.



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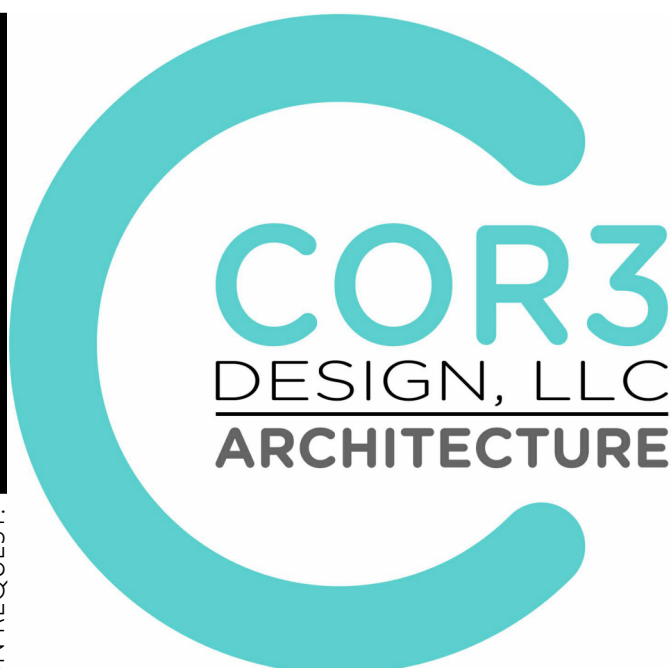


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VDC
Sheet Number:
S-203
Sheet Title:
CMU REINFORCEMENT ELEVATIONS



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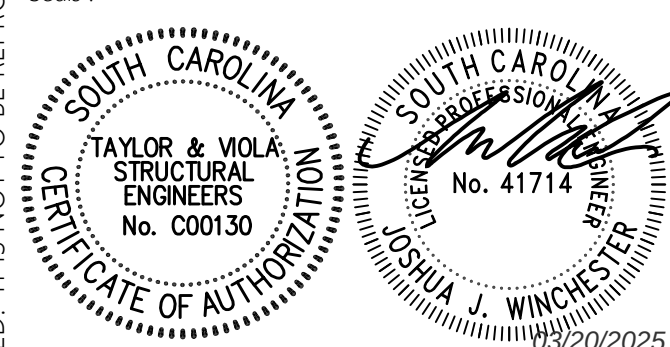
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Seals:



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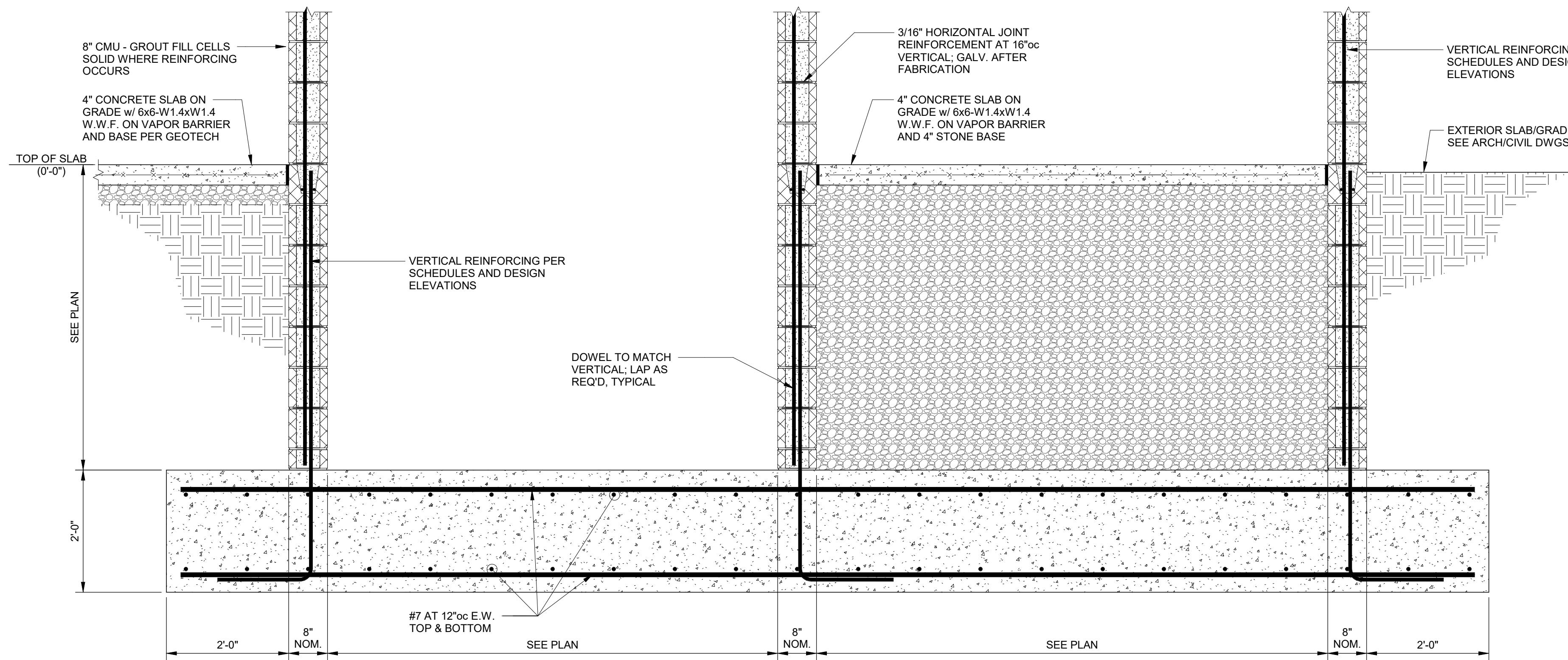
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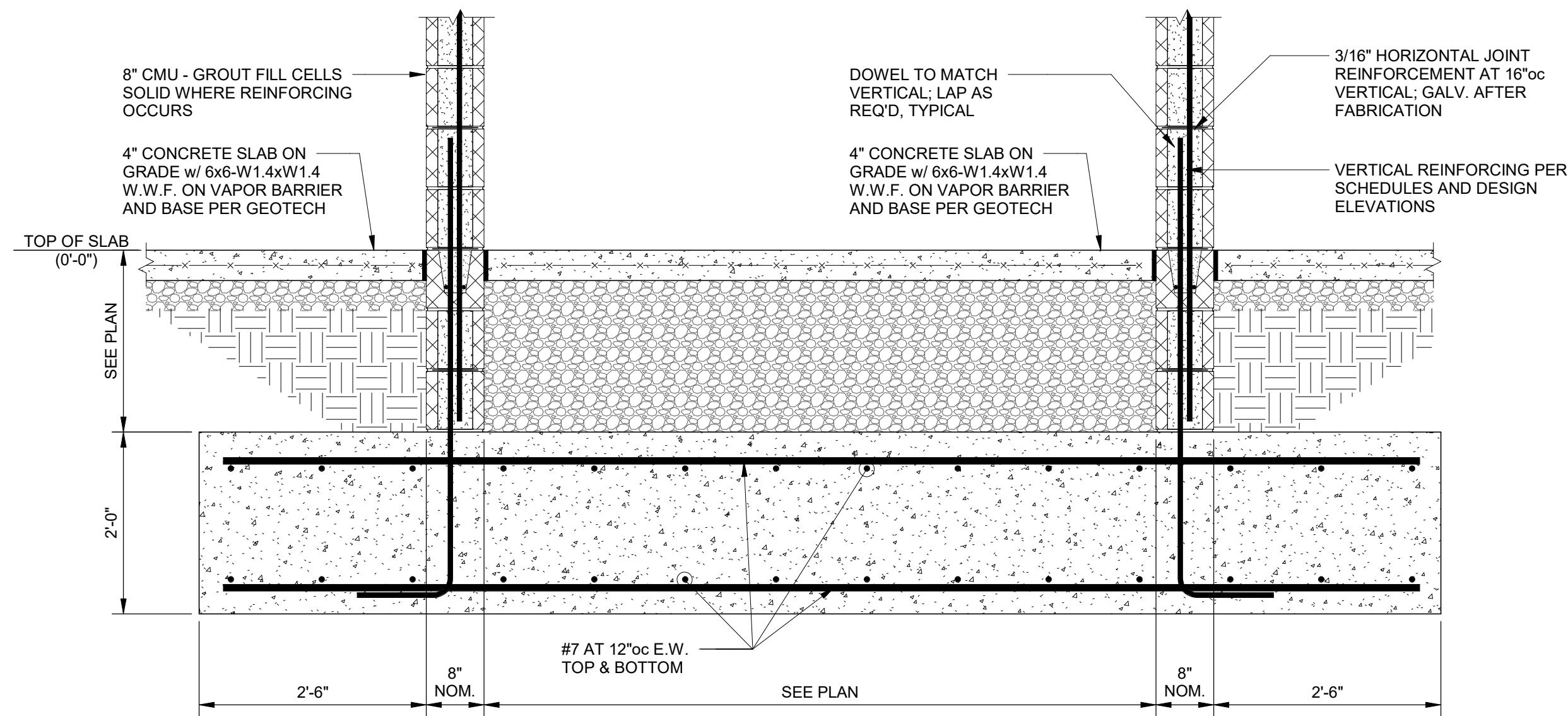
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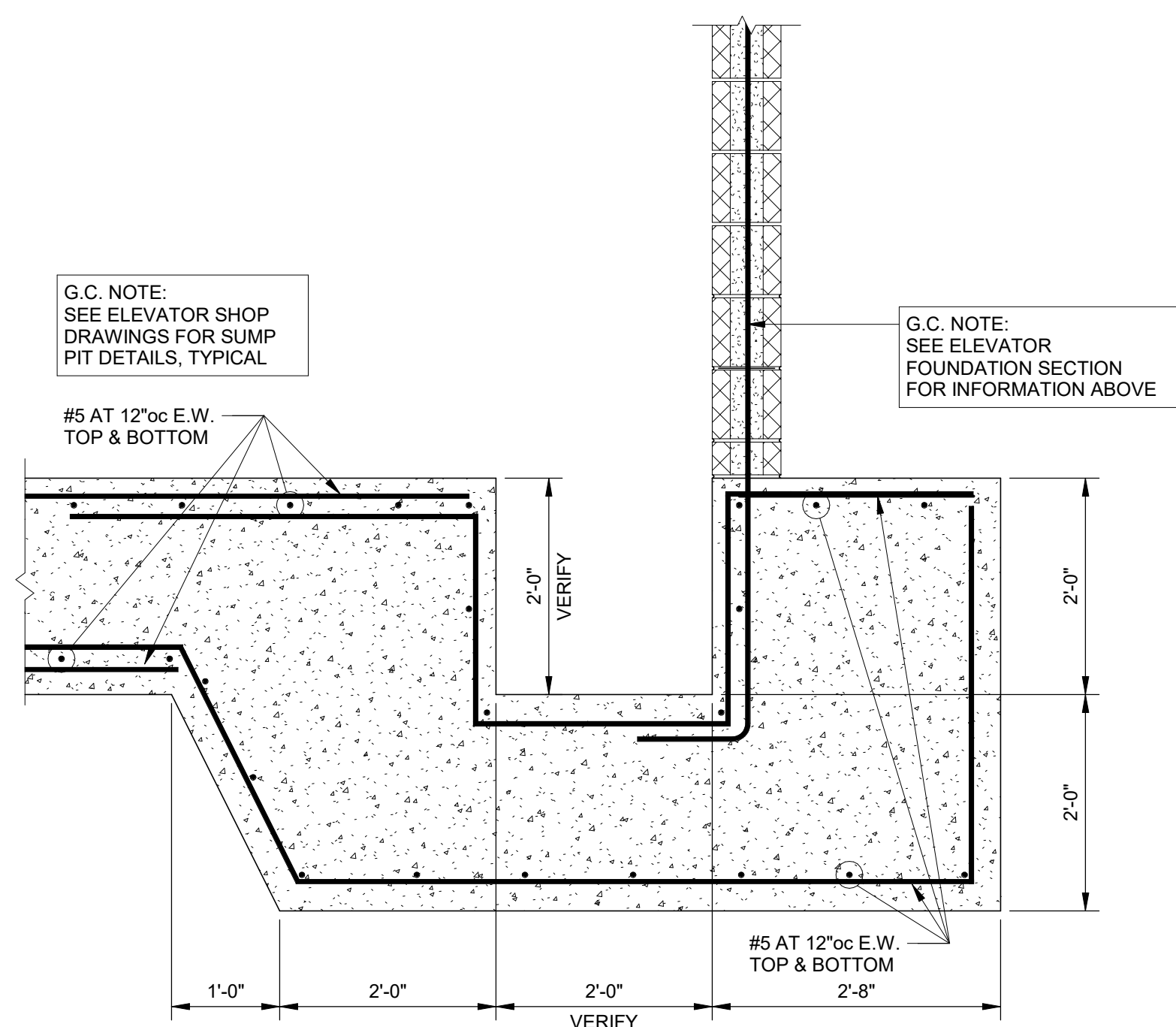
FOUNDATION SECTIONS



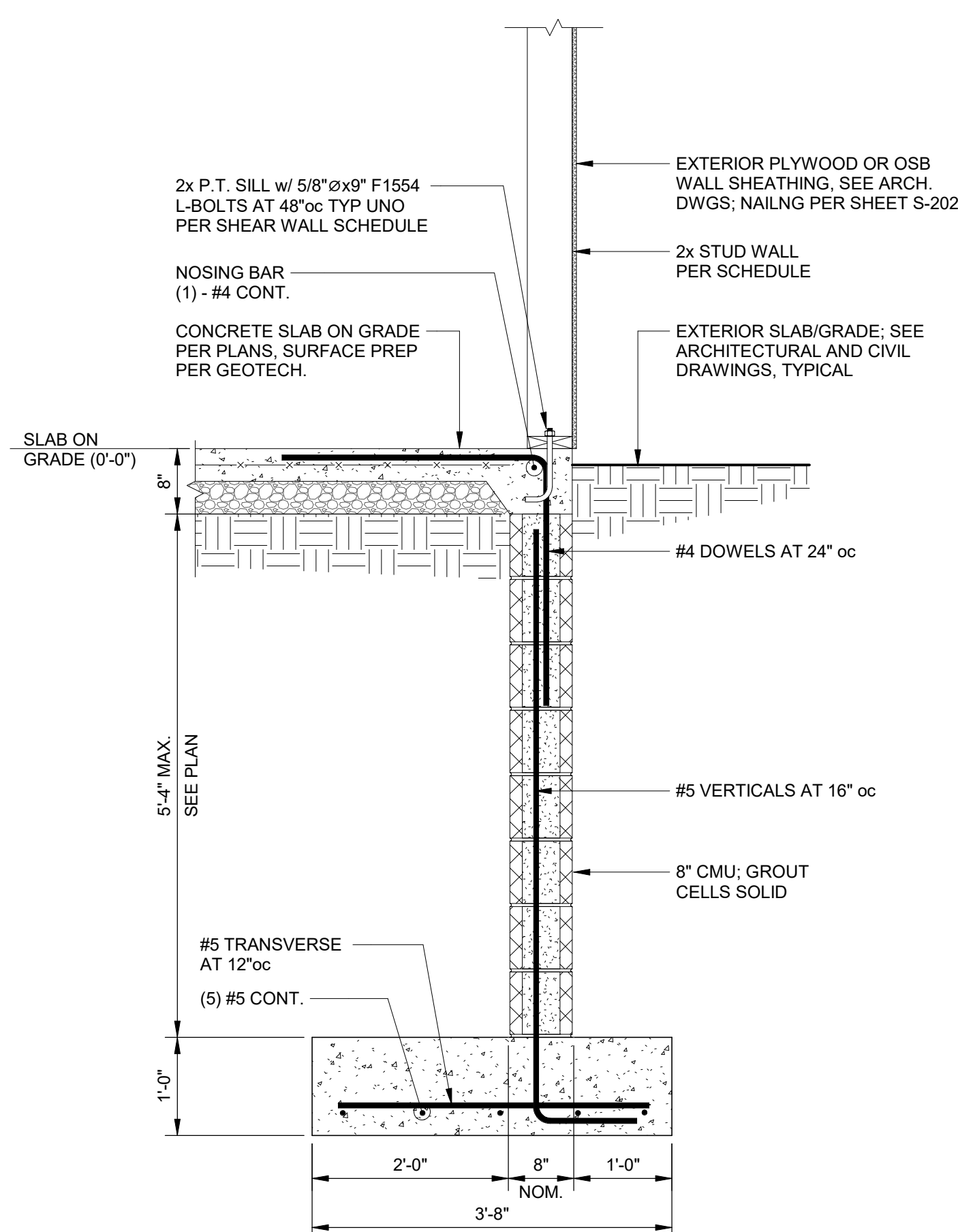
7 STAIR #1/ ELEVATOR FOUNDATION SECTION
3/4" = 1'-0"



6 STAIR #2 FOUNDATION SECTION
3/4" = 1'-0"

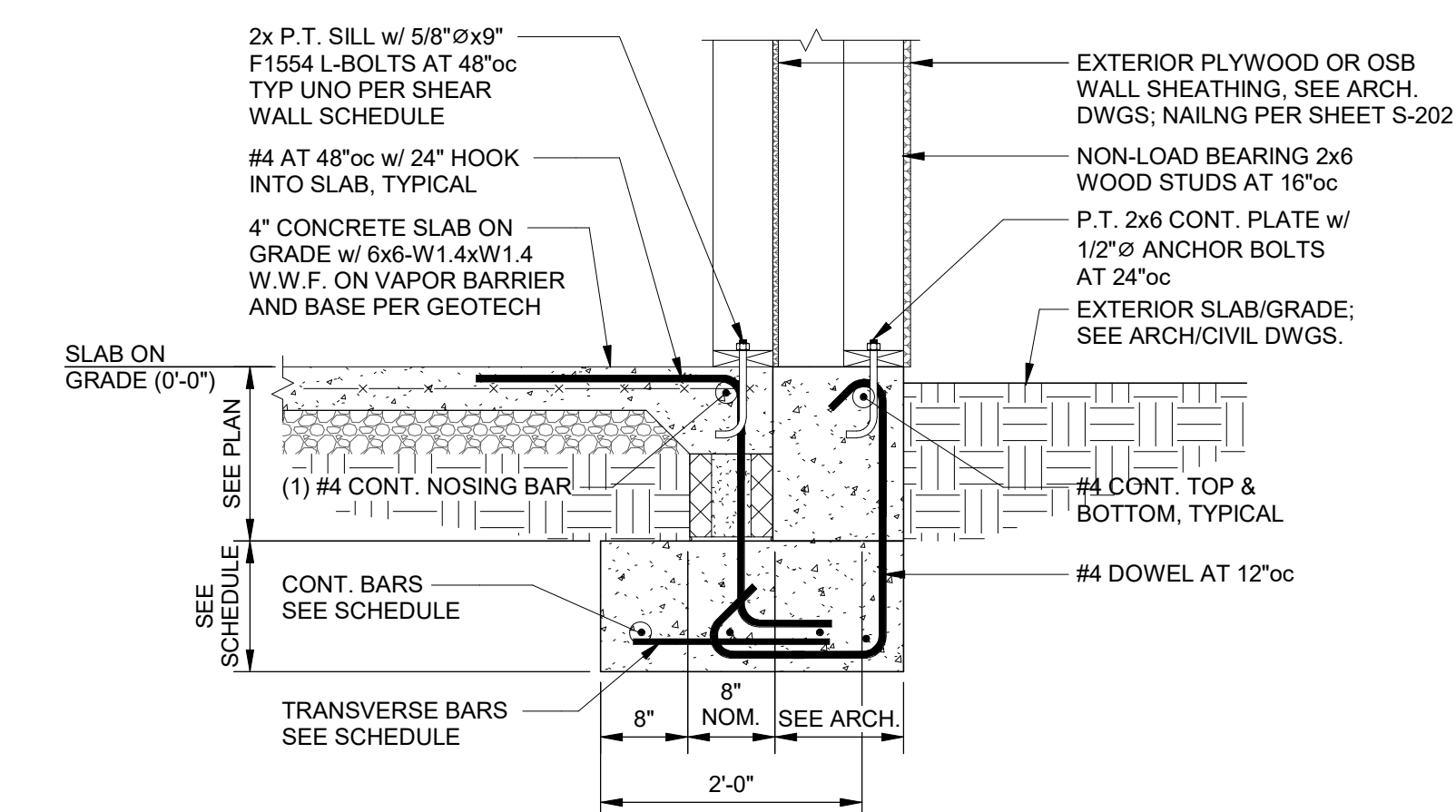


5 FOUNDATION SECTION AT ELEVATOR SUMP PIT
3/4" = 1'-0"

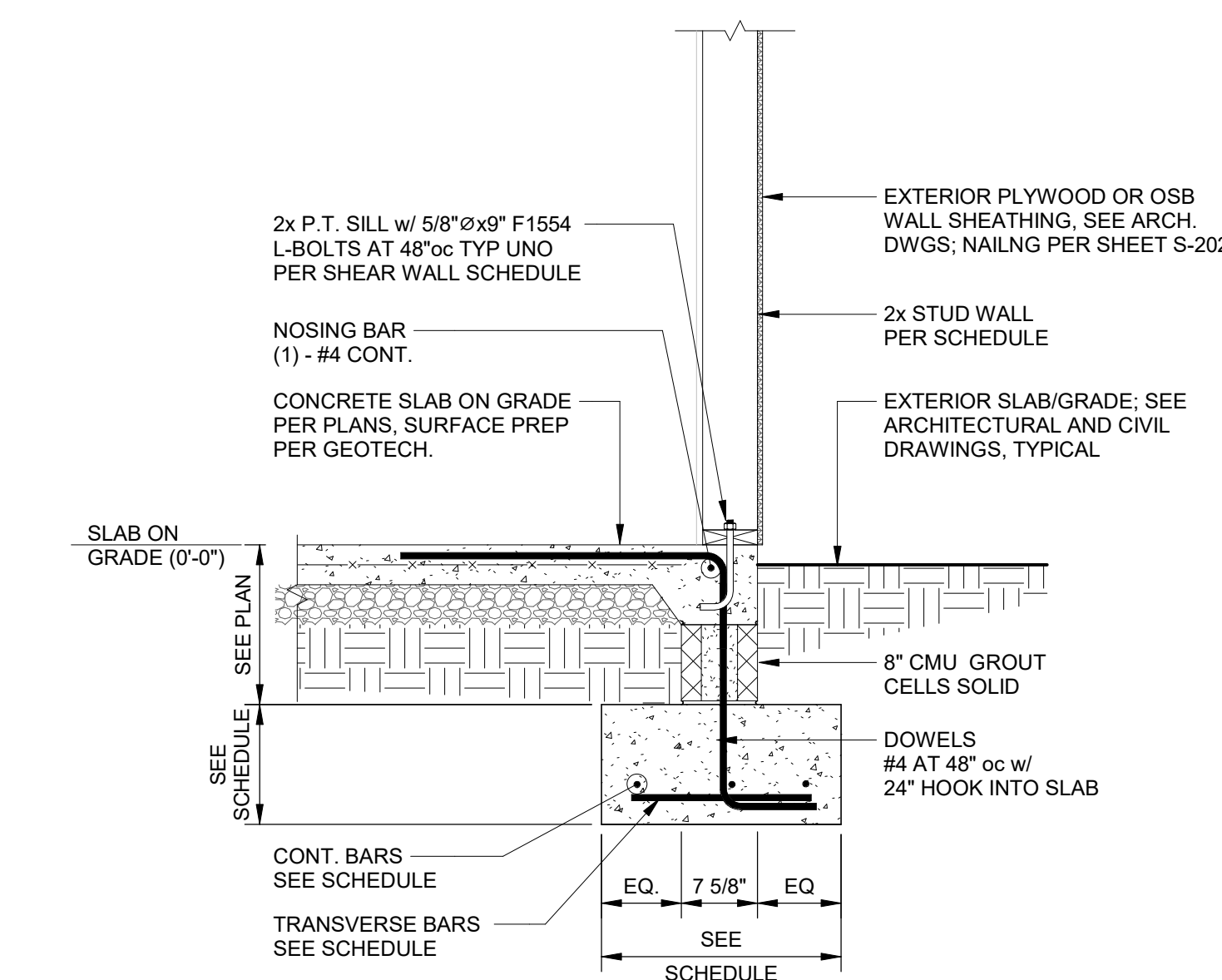


4 FOUNDATION SECTION
3/4" = 1'-0"

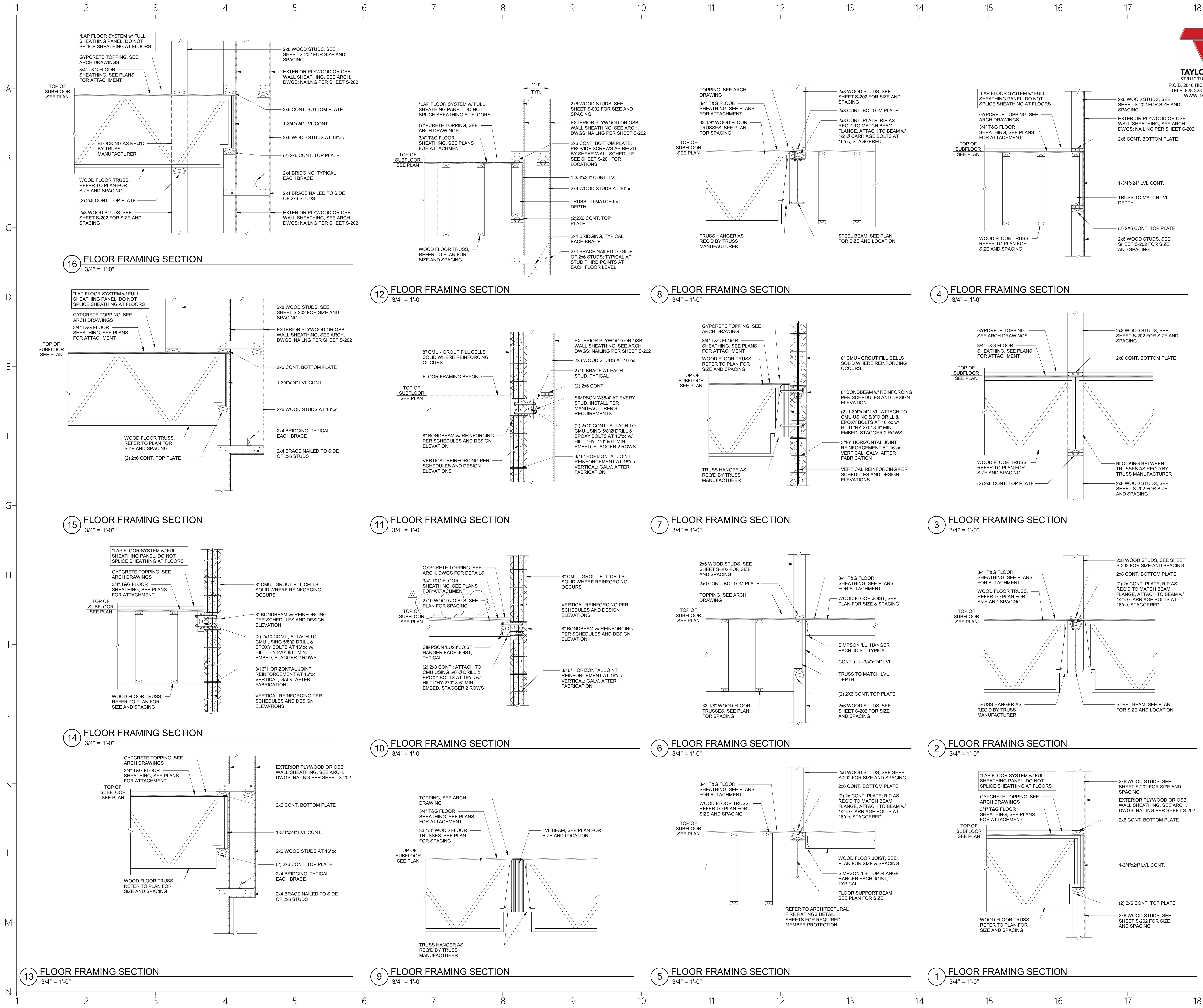
3 FOUNDATION SECTION
3/4" = 1'-0"



2 FOUNDATION SECTION
3/4" = 1'-0"



1 FOUNDATION SECTION
3/4" = 1'-0"



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SUITES BY HILTON

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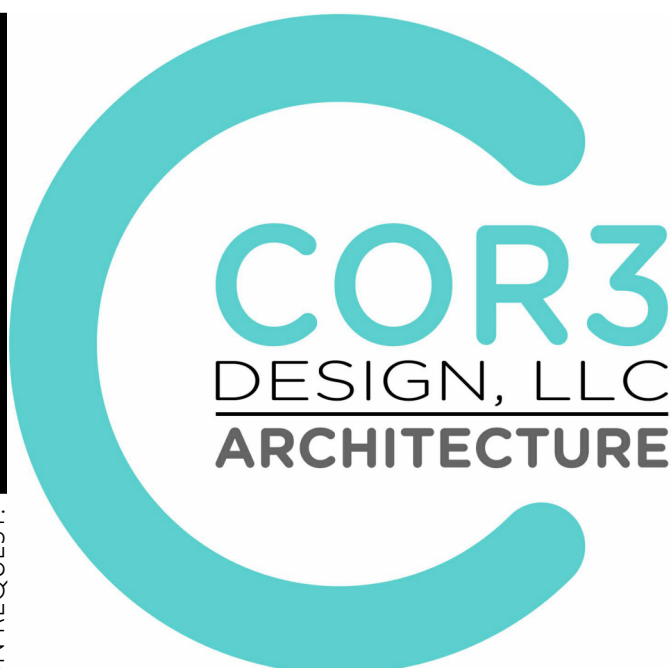
S-302

Sheet Title:

FLOOR FRAMING SECTIONS



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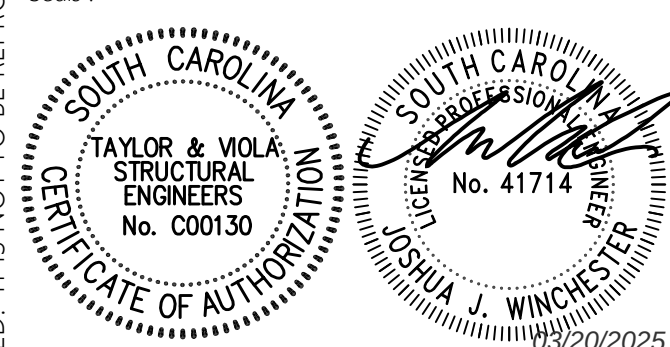
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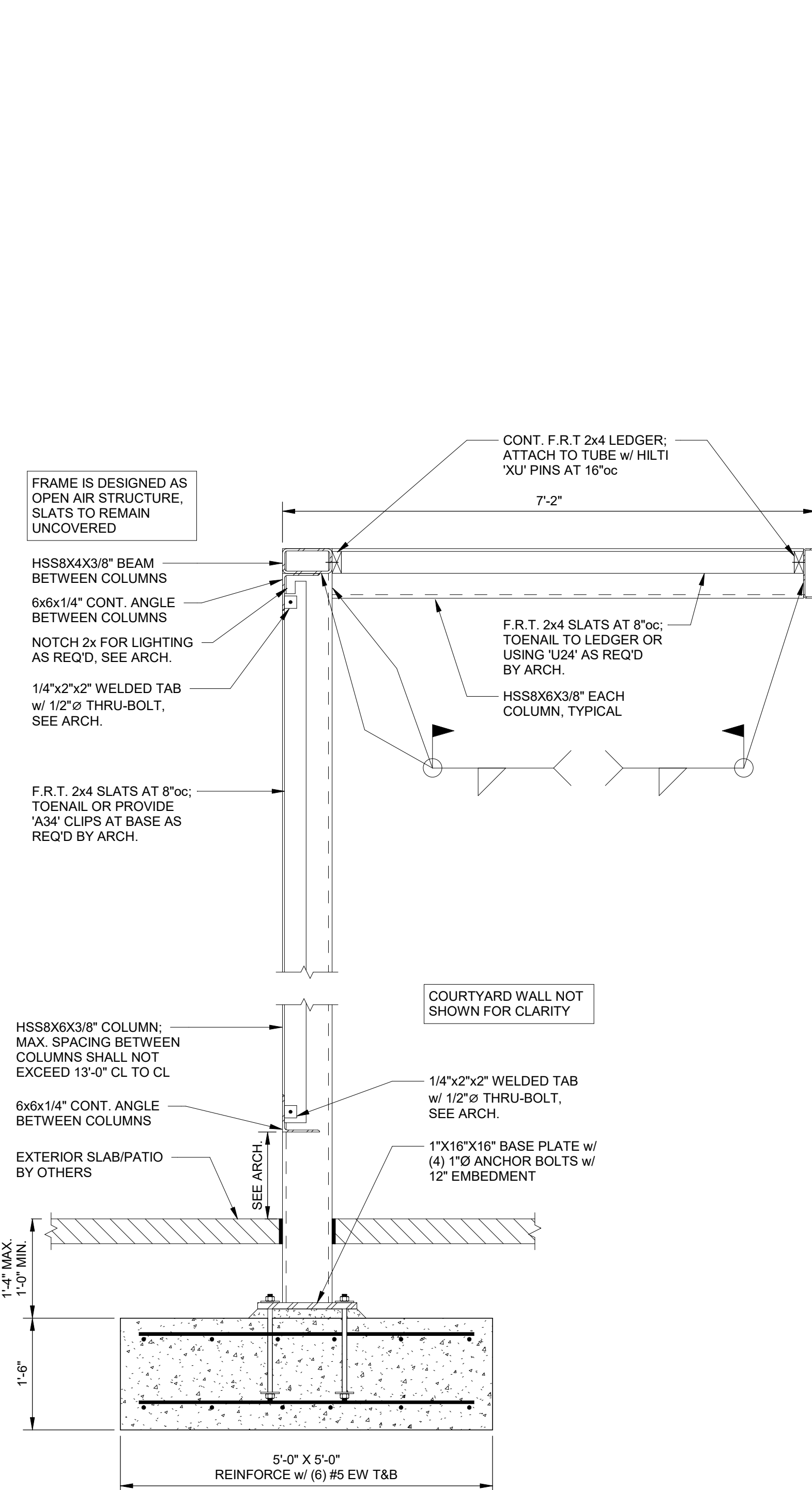
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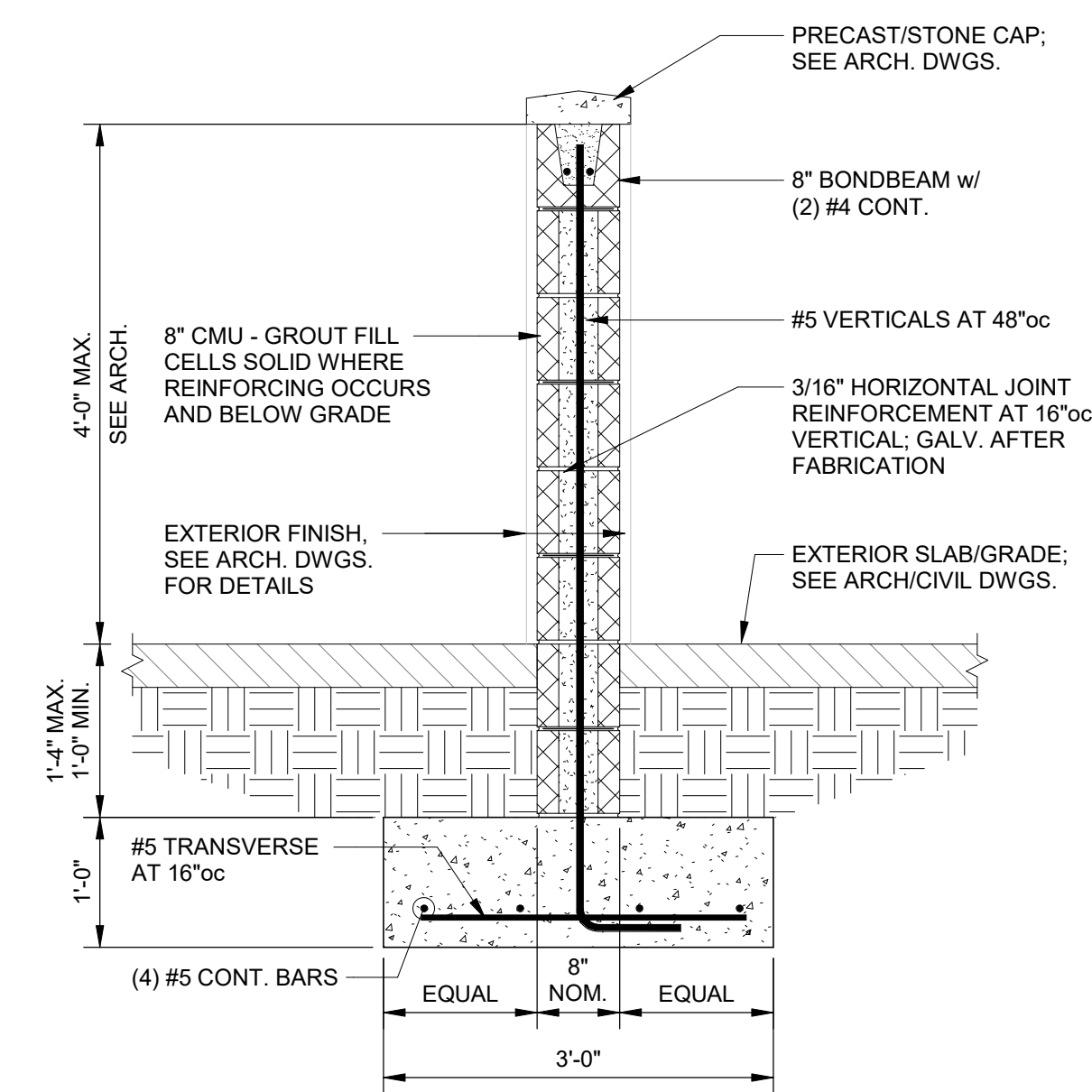
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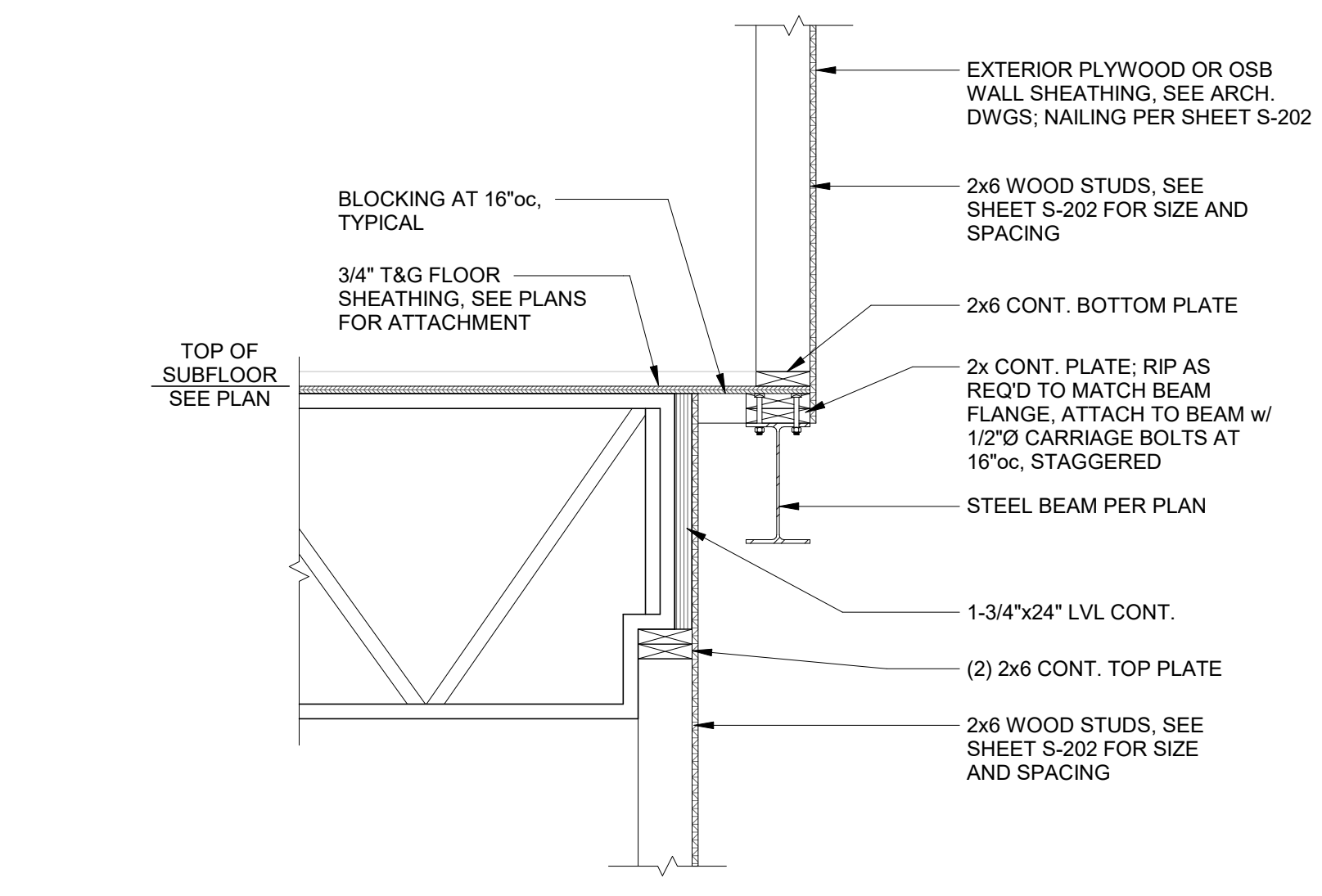
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FLOOR FRAMING SECTIONS



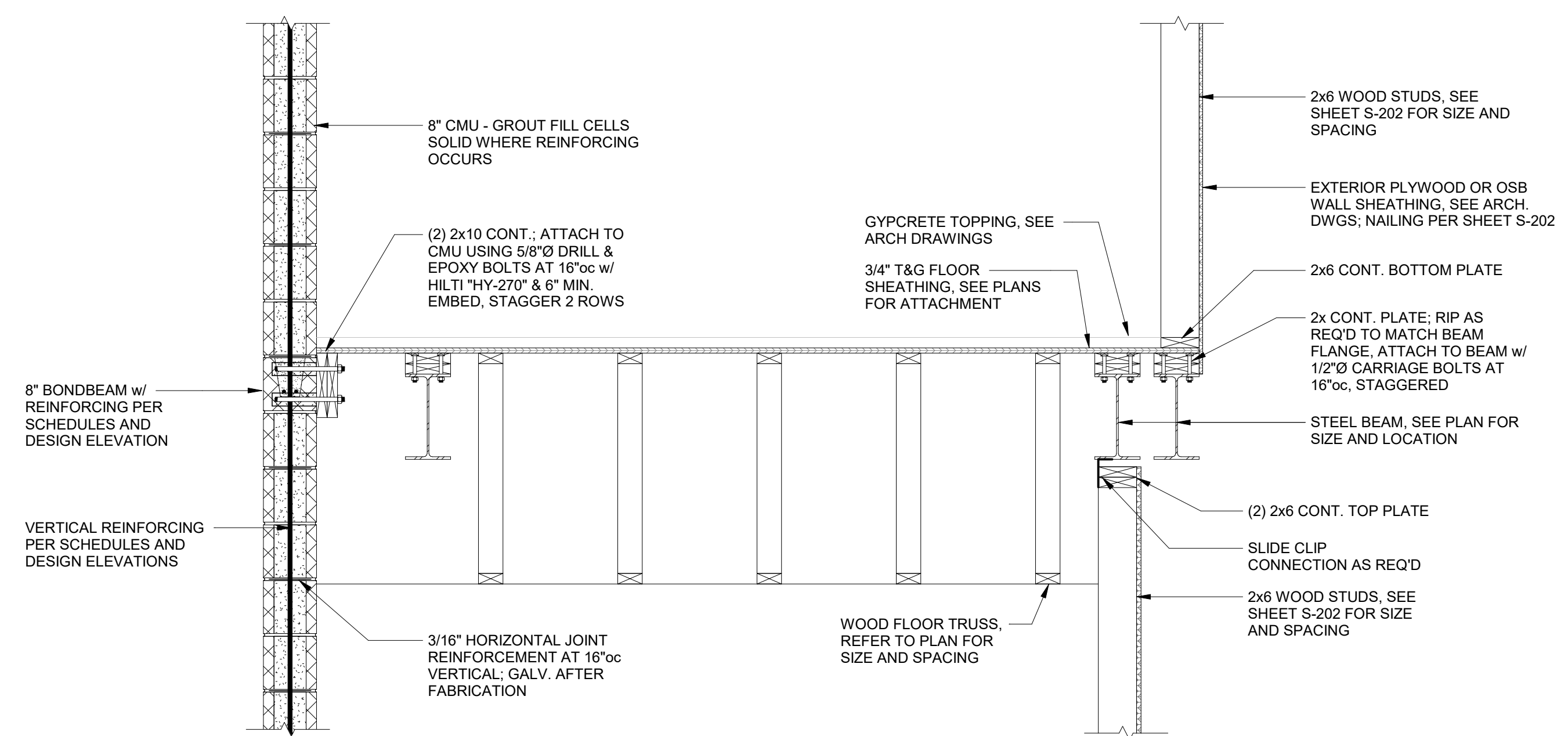
5 TYPICAL TRELLIS FRAMING SECTION
3/4" = 1'-0"



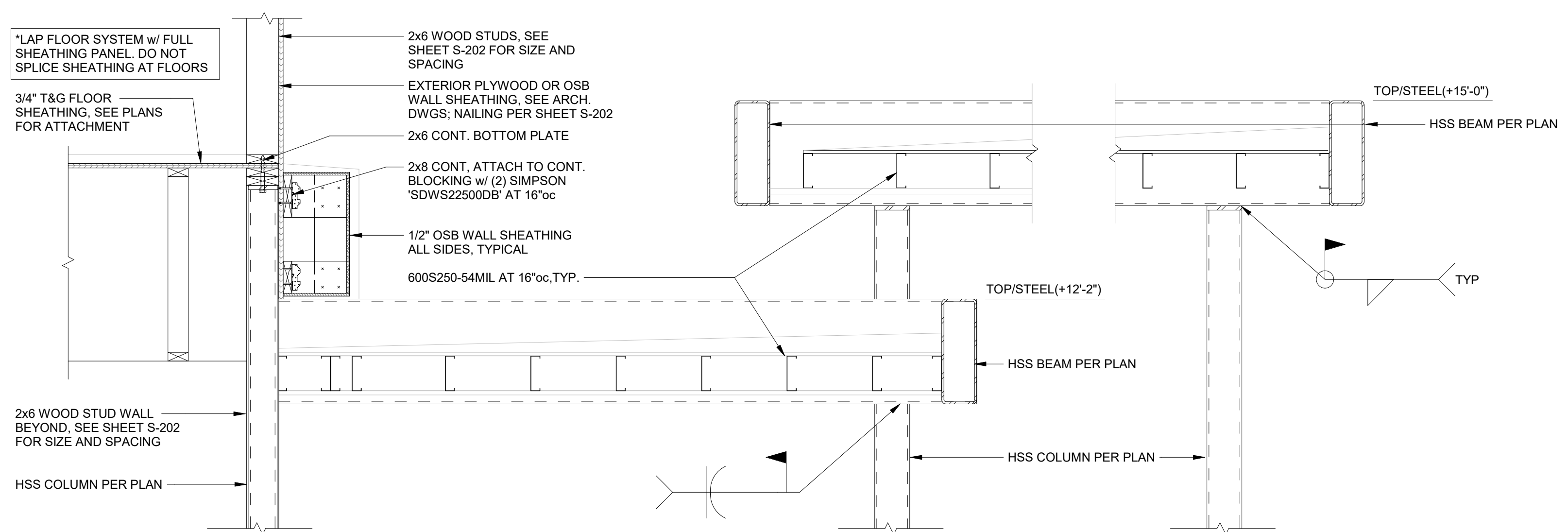
4 TRELLIS COURTYARD SITE WALL SECTION
3/4" = 1'-0"



3 FLOOR FRAMING SECTION
3/4" = 1'-0"



2 FLOOR FRAMING SECTION
3/4" = 1'-0"



1 PORTE COCHERE FRAMING SECTION
3/4" = 1'-0"



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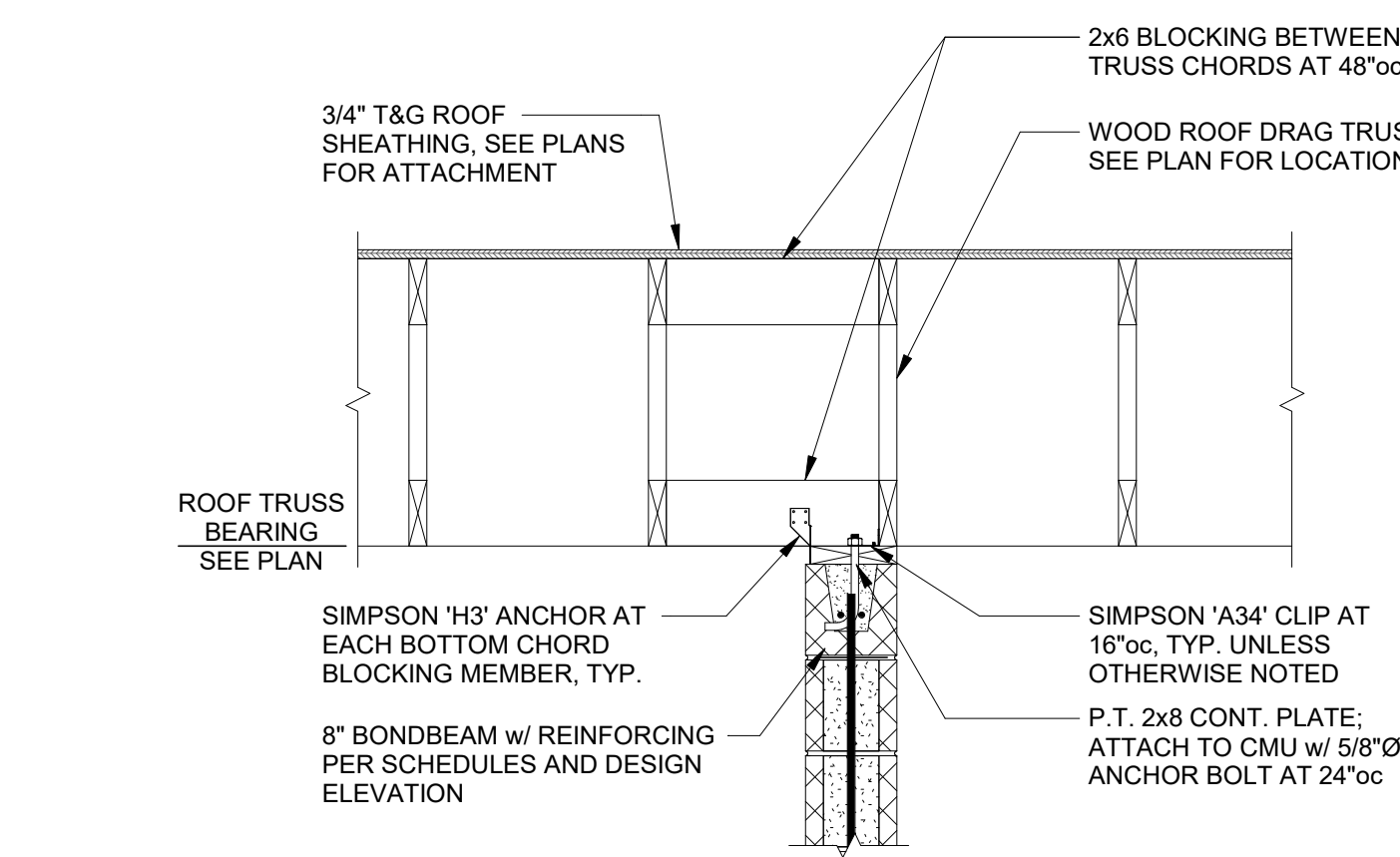
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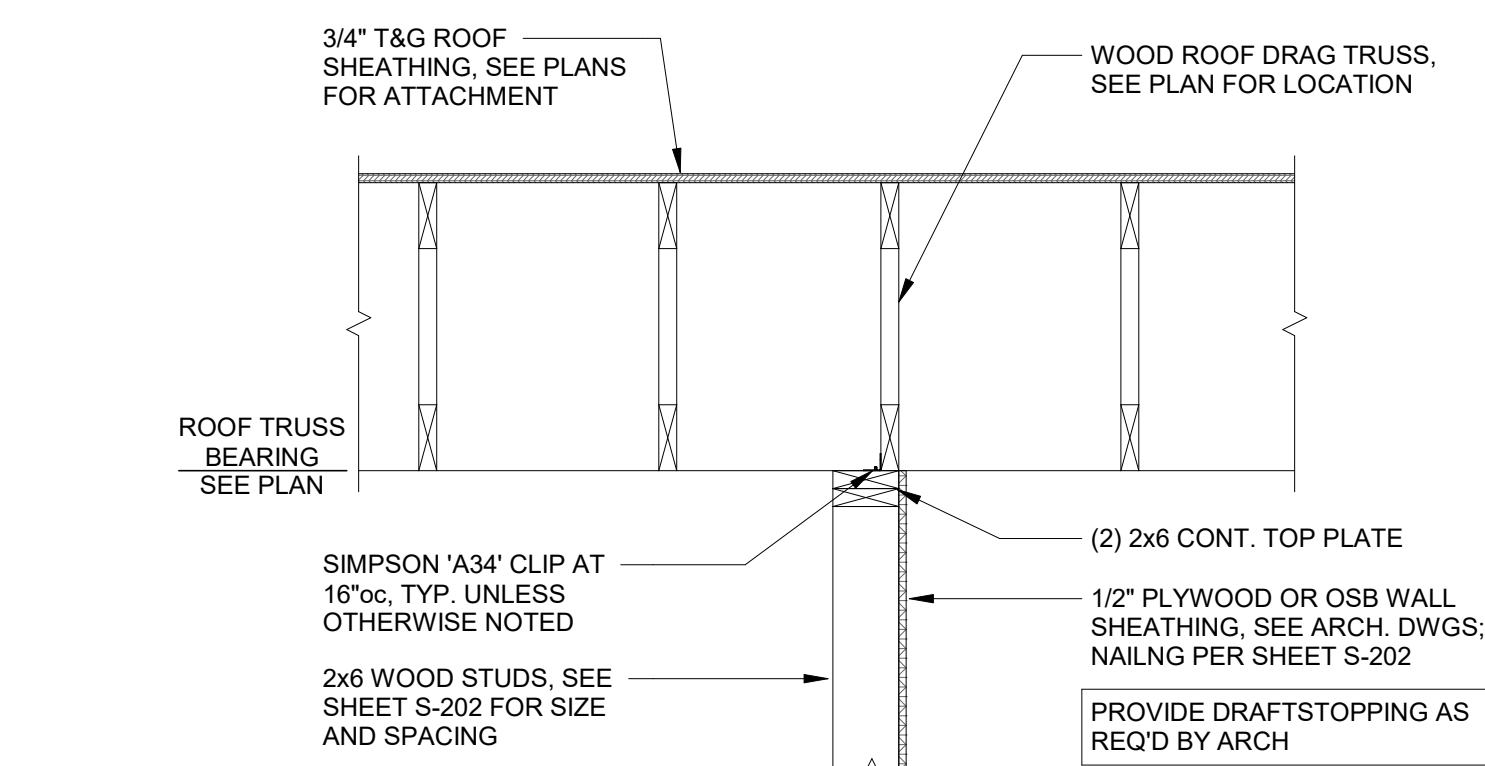
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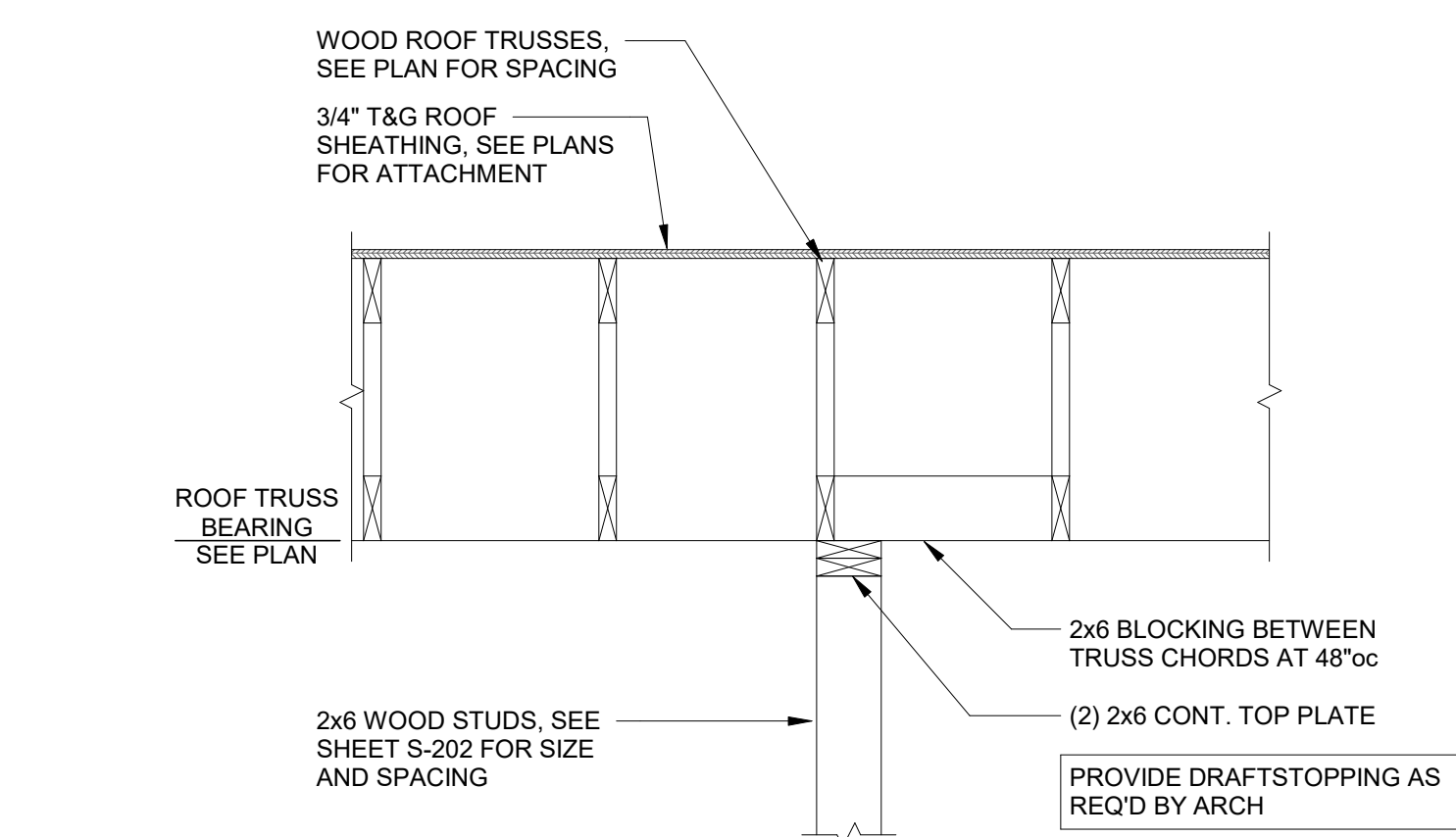
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S-305
Sheet Title :
ROOF FRAMING SECTIONS



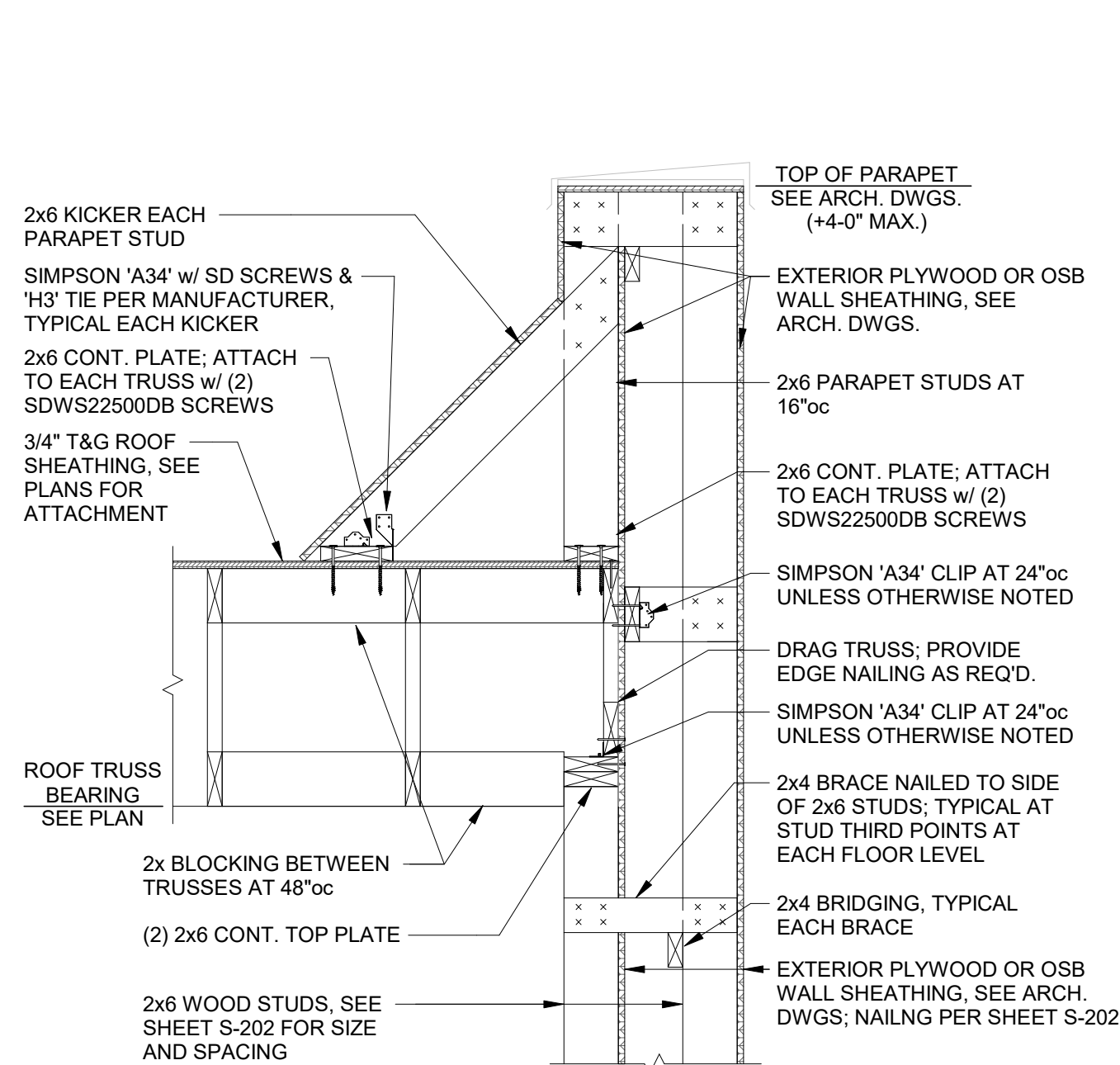
3 ROOF FRAMING SECTION
3/4" = 1'-0"



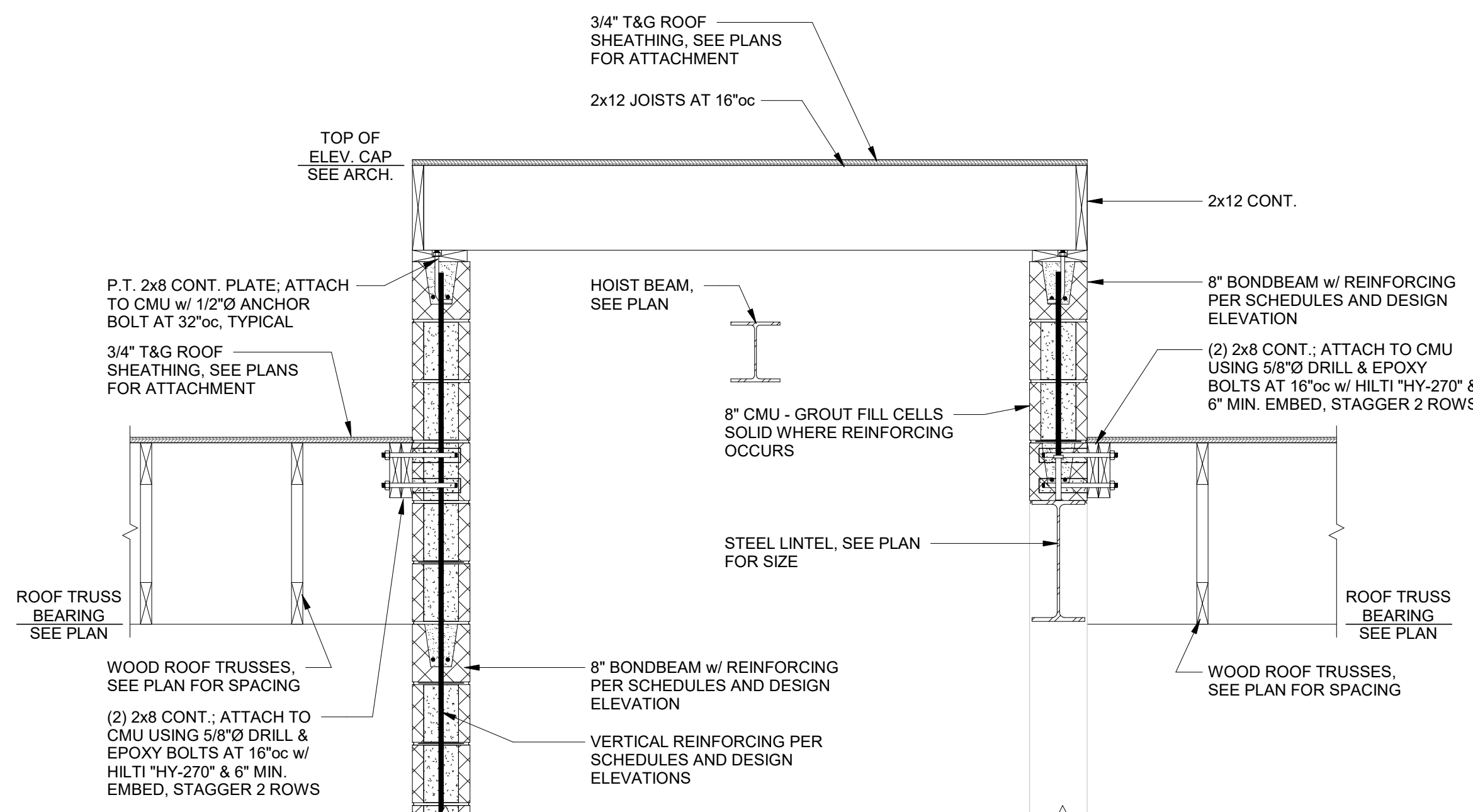
2 ROOF FRAMING SECTION
3/4" = 1'-0"



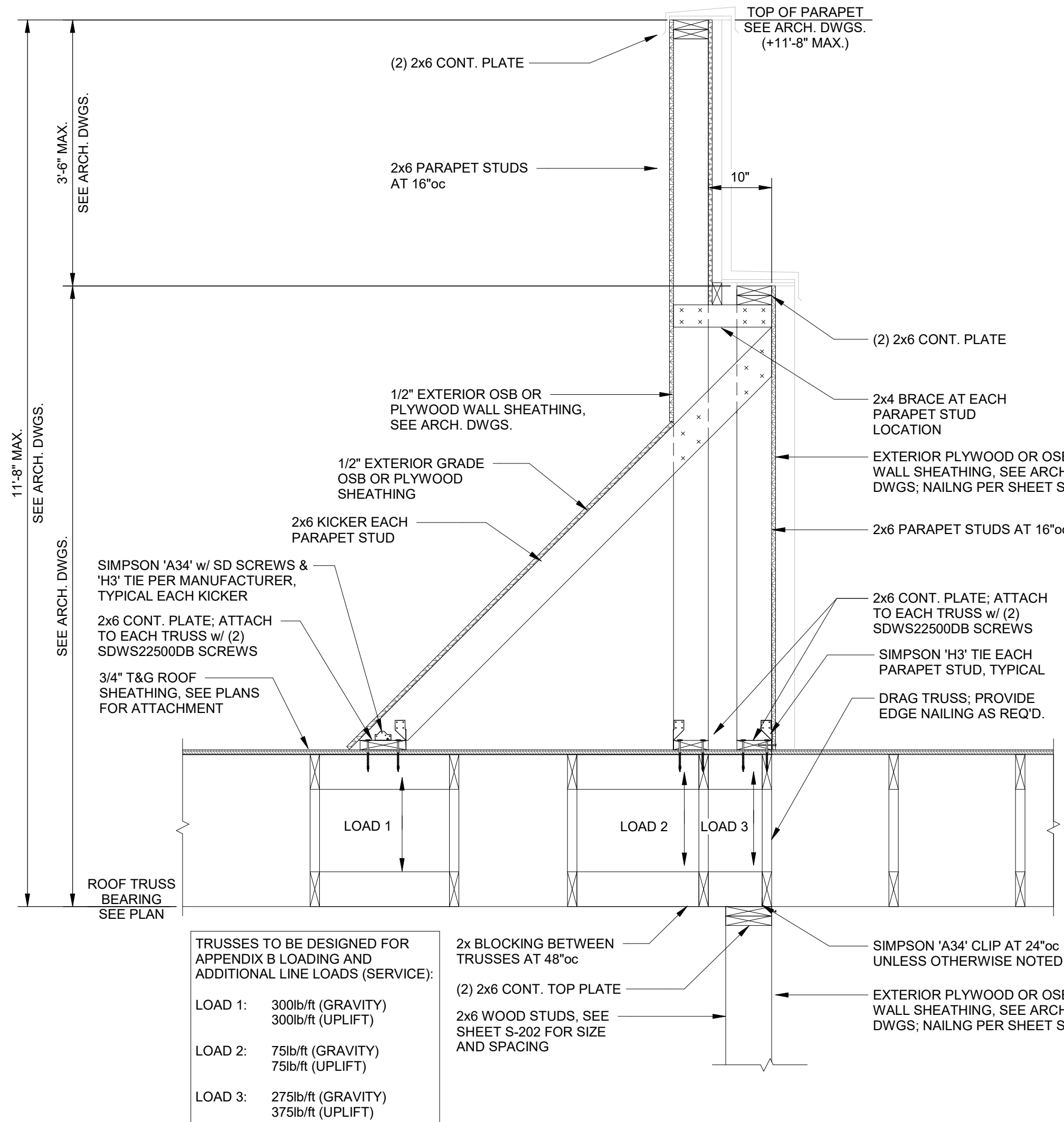
1 ROOF FRAMING SECTION
3/4" = 1'-0"



6 ROOF FRAMING SECTION
3/4" = 1'-0"



4 ROOF FRAMING SECTION
3/4" = 1'-0"



5 ROOF FRAMING SECTION
3/4" = 1'-0"