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SUBMITTALS FOR SHOP DRAWINGS, PRODUCT DATA, AND MILL TESTS ARE REQ'D FOR ITEMS NOTED IN THE INDIVIDUAL MATERIALS SECTIONS AND FOR BIDDER

SUBMITTALS SHALL BE MADE IN TIME TO PROVIDE A MINIMUM OF (2) TWO WEEKS OR (10) TEN WORKING DAYS TO REVIEW BY THE ARCHITECT/ENGINEER PRIOR TO

PRIOR TO THE SUBMISSION TO THE ARCHITECT/ENGINEER, THE CONTRACTOR SHALL REVIEW THE SUBMITTAL FOR COMPLETENESS. DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE S.E.R., AND THEREFORE, MUST BE VERIFIED BY THE GENERAL CONTRACTOR. CONTRACTOR SHALL PROVIDE ANY NECESSARY DIMENSIONAL DETAILS REQUESTED BY THE

ONCE THE CONTRACTOR HAS COMPLETED HIS REVIEW, THE STRUCTURAL ENGINEER OF RECORD (S.E.R.) WILL REVIEW THE SUBMITTAL FOR GENERAL COMFORMANCE WITH THE DESIGN CONCEPT AND THE CONTRACT DOCUMENTS OF THE BUILDING AND WILL STAMP THE SUBMITTAL ACCORDINGLY. MARKINGS OR COMMENTS SHALL NOT BE CONSTRUED AS RELIEVING THE CONTRACTOR FROM COMPLIANCE WITH THE PROJECT PLANS AND SPECIFICATIONS. NOR DEPARTURES THERE FROM. THE S.E.R. WILL RETURN SUBMITTALS IN THE FORM THEY ARE SUBMITTED IN (EITHER HARD COPIES OR ELECTRONIC) FOR HARD COPY SUBMITTALS, THE CONTRACTOR IS RESPONSIBLE FOR SUBMITTING THE REQUIRED NUMBER OF COPIES TO THE

WHEN SHOP DRAWINGS (COMPONENT DESIGN DRAWINGS) DIFFER FROM OR ADD TO THE REQUIREMENTS OF THE STRUCTURAL DRAWINGS THEY SHALL BE DESIGNED AND STAMPED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED.

SUBMIT "BIDDER-DESIGNED" DEFERRED SUBMITTALS TO THE ARCHITECT AND STRUCTURAL FOR REVIEW. THE DEFERRED SUBMITTALS SHALL ALSO BE SUBMITTED TO THE CITY FOR

DESIGN OF PREFABRICATED, "BIDDER-DESIGNED", MANUFACTURED, PRE-ENGINEERED, OR OTHER FABRICATED PRODUCTS SHALL COMPLY WITH THE FOLLOWING REQUIREMENTS. 1. DESIGN CONSIDERS TRIBUTARY DEAD, LIVE, WIND AND EARTHQUAKE LOADS IN

3. DESIGN SHALL CONFORM TO THE SPECIFICATIONS AND REFERNCE

a. CALCULATIONS PREPARED, STAMPED AND SIGNED BY THE SPECIALTY

c. PRODUCT DATA, TECHNICAL INFORMATION, AND MANUFACTURER'S WRITTEN REQUIREMENTS AND AGENCY APPROVALS AS APPLICABLE. d. S.S.E. MAY SUBMIT TO THE ARCHITECT / ENGINEER. A REQUEST TO UTILIZE RELEVANT ALTERNATE DESIGN CRITERIA OF SIMILAR NATURE AND GENERAL EQUIVALENCY WHICH IS RECOGNIZED BY THE CODE AND ACCEPTABLE TO THE AUTHORITY

DEFLECTION LIMITS FOR S.S.E. / BIDDER DESIGNED ELEMENTS

ICAL	LIMIT
AIN, SNOW, OAD (TL) DEFL.	L/240 WHERE (L IS SPAN LENGTH IN INCHES)
SNOW OR WIND	L/360
J.N.O.	L/360
۱.О.	L/480
RTING MASONRY	L/600 OR 1/4" MAX MASONRY (DL) ONLY
TION SUPPORT	L/600 OR 1/4" MAX
ONTAL	LIMIT
RTING BRITTLE	L/240 (NOTE 1)
RTING FLEXIBLE	L/180 (NOTE 1)
RTING MASONRY	L/600 @ 0.7 x CLADDING WIND OR 0.7E (NOTE 1)
Т	0.020 x STORY HEIGHT

*ROOF MEMBERS SHALL BE DESIGNED TO CONTROL PONDING SUCH THAT CHORD MEMBER STRESSES DO NOT EXCEED LIMITS

(NOTE 1) WIND LOAD IS REDUCIBLE TO 0.42 TIMES COMPONENT

THE S.E.R. WILL REVIEW THE SUBMITTAL FOR GENERAL CONFORMANCE WITH THE DESIGN OF THE BUILDING AND WILL STAMP THE SUBMITTAL ACCORDINGLY. REVIEW OF THE SPECIALTY STRUCTURAL ENGINEER'S (S.S.E.) SHOP DRAWINGS (COMPONENT DESIGN DRAWINGS) IS FOR COMPLIANCE WITH DESIGN CRITERIA AND COMPATIBILITY WITH THE DESIGN OF THE PRIMARY STRUCTURE AND DOES NOT RELIEVE THE S.S.E. OF RESPONSIBILITY FOR THAT DESIGN. ALL NECESSARY BRACING, TIES, ANCHORAGE, PROPRIETARY PRODUCTS SHALL BE FURNISHED AND INSTALLED PER MANUFACTURER'S INSTRUCTIONS OR THE S.S.E.'S DESIGN DRAWINGS AND

* HANDRAIL, GUARDRAIL, CATWALK FRAMING, BALCONY RAIL ANCHORAGES

* COLD-FORMED METAL STUDS - EXTERIOR WALL ASSEMBLIES

* COLD-FORMED METAL STUDS - FRAMING INTERIOR WALLS INCLUDING

* COLD-FORMED METAL STUDS & JOISTS FRAMING OVERHEAD SOFFITS * TILT-UP WALL PANELS INCLUDING BUT NOT LIMITED TO PANEL-TO-PANEL

CONNECTIONS, FOUNDATION CONNECTIONS, CONNECTIONS BETWEEN OTHER STRUCTURAL SOIL IMPROVMENTS (RAMMÉD-AGGREGATE PIER, RIGID INCLUSIONS, GROUT INJECTIONS, ETC.) TO MEET THE REQUIREMENTS OF THE GEOTECHNICAL REPORT

GENERAL MASONRY NOTES:

CODES: MASONRY DESIGN AND CONSTRUCTION IS BASED ON AND SHALL CONFORM TO THE PROVISIONS OF ACI A530-LATEST EDITION / ASCE 5-LATEST EDITION / TMS 402-LATEST EDITION INCLUDING SEISMIC REQUIREMENTS OF APPENDIX A. MASONRY: CONCRETE UNITS CONFORMING TO:

ASTM C90 FOR HOLLOW LOADBEARING UNITS ASTM C145 FOR SOLID LOADBEARING UNITS FOR CONCRETE BRICK ASTM C55 MINIMUM F'm = 2000 PSI

ALL MASONRY TO BE TYPE "N" AND MEET ASTM C90 GUIDELINES.

BRICK SHALL CONFORM TO ASTM C62.

8" CMU SHALL BE 7 5/8" X 7 5/8" X 15 5/8" W/ 3/8" JOINTS

MORTAR AND GROUT: USE MORTAR FOR MASONRY CONFORMING TO ASTM C270 TYPE S. GROUT SHALL CONFORM TO ASTM C476 MIN. Fc = 3000 PSI GROUT SHALL CONSIST OF 1 PART PORTLAND CEMENT MIXED TO DESIRED CONSISTENCY. MASONRY WALLS MADE OF CMU ARE TO BE REINFORCED USING 3/16" TRUSS TYPE

REINFORCEMENT (GALVANIZED AFTER FABRICATION) AT 16" O.C. MASONRY WALLS MADE OF BRICK AND CMU WITH INSULATED CAVITY ARE TO BE REINFORCED

USING 3/16" CAVITY-TRUSS TRIPOD TYPE REINFORCEMENT (GALVANIZED AFTER FABRICATION) AT 16" O.C. MASONRY COURSING TO BE A RUNNING BOND, PERPENDICULAR JOINTS SHALL BE

TOOTHED TOGETHER REINFORCING BARS SHOWN IN MASONRY WALLS SHALL BE LAPPED AS FOLLOWS,

EINFORGING BARS SHOWN IN MASONRY WALLS SHALL BE LAPPED
LL LAP SPLICES SHALL BE ACCORDING TO IBC SECTION 2107.2.3
#3 BARS-LAP 18" MIN
#4 BARS-LAP 24" MIN
#5 BARS-LAP 36" MIN
#6 BARS-LAP 56" MIN
#7 BARS-LAP 76" MIN

#8 BARS-LAP 107" MIN REINFORCING BARS SHALL CONFORM TO ASTM A-615 GRADE 60

CONDITIONS ONLY AFTER WALL HAS SET UP.

BACK-FILL AT THE BASEMENT AND RETAINING WALLS SHALL BE DONE UNDER CONTROLLED

ANY CMU MASONRY WITH COMPRESSIVE STRENGTH OF GREATER THAN 1500PSI SHALL BE VERIFIED BY TEST ACCORDING TO ASTM C1314 OR BY ASTM C140. THESE TEST RESULTS SHALL

BE AVAILABLE TO THE BUILDING INSPECTOR. ALL FILLED CELL MASONRY TO BE BUILT TO PRESERVE THE UNOBSTRUCTED VERTICAL

CONTINUITY OF THE CELLS TO BE FILLED. LAY UNITS WITH FULL FACE SHELL MORTAR BEDS AND CROSS WEBS ADJACENT TO VERTICAL CORES TO BE FILLED.

FILL CELLS WITH GROUT TO MAXIMUM OF 8 FEET. IF TOTAL POUR IS GREATER THAN 8 FEET, POUR IN FOUR (4) FOOT LIFTS STOPPING 1 1/2" BELOW UPPERMOST UNIT FOR CONSTRUCTION JOINTS **GENERAL STRUCTURAL STEEL NOTES:**

THE DESIGN, DETAILING, FABRICATION, AND ERECTION OF STRUCTURAL AND MISCELLANEOUS STEEL SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE AISC "SPECIFICATIONS FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS."

STEELS: STRUCTURAL STEEL IS TO CONFORM TO THE FOLLOWING SPECIFICATIONS: STRUCTURAL STEEL (WIDE FLANGE SHAPES): ASTM A992 ASTM A500, GRADE B, FY=46 KSI STRUCTURAL TUBING: STRUCTURAL STEEL (ALL OTHER SHAPES): ASTM A36 STRUCTURAL STEEL (PLATES):

ASTM A36

ROOF DECK TO BE 1 1/2" DEEP. 6" WIDE RIB. PRIME PAINTED. VULCRAFT 1.5B. 18GA OR EQUAL. FASTEN ROOF DECK TO SUPPORTS W/ #12 HEX-HEAD SCREWS USING 36/4 PATTERN MINIMUM WITH SIDELAP CONNECTION 12" oc TO ALLOW ROOF TO ACT AS A DIAPHRAGM.

WELDING: ALL WELDING IS TO CONFORM TO ANSI/AWS D1.1 STRUCTURAL WELDING CODE USING 370XX ELECTRODES. ALL WELDING IS TO BE DONE BY WELDERS QUALIFIED UNDER ANSI/AWS D1.1 PROVIDE EVIDENCE OF QUALIFICATION TO ARCHITECT

BOLTS: ALL BOLTED CONNECTIONS ARE TO BE MADE USING 3/4" A325 BOLTS IN BEARING TYPE CONNECTIONS UNLESS SHOWN OR NOTED OTHERWISE ON THE DRAWINGS. A307 BOLTS MAY BE USED FOR ERECTION BOLTS. ANCHOR RODS: ANCHOR RODS ARE TO BE OF A36 MATERIAL. ALL ANCHOR RODS ARE TO BE OF

A36 MATERIAL. ALL ANCHOR RODS ARE TO BE SET WITH A TEMPLATE PRIOR TO POURING CONCRETE. **GENERAL TIMBER NOTES:**

WOOD CONSTRUCTION SHALL BE IN ACCORDANCE WITH CHAPTER 23 OF THE SOUTH CAROLINA STATE BUILDING CODE.

ALL SHOP FABRICATED TRUSSES SHALL BE DESIGNED, FABRICATED, AND INSTALLED IN ACCORDANCE WITH THE NATIONAL DESIGN SPECTIFICATION. PUBLISHED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION. DESIGN AND FABRICATION DRAWINGS SHALL BE SEALED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN SOUTH CAROLINA. THESE DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER FOR REVIEW BEFORE FABRICATION AND INSTALLATION OF THE MEMEBERS.

ALL TRUSSES SHALL BE LATERALLY BRACED AS RECOMMENDED BY THE MANUFACTURER. ALL NAILING AND STAPLING FOR CONNECTION OF WOOD MEMBERS, NOT NOTED ON DRAWING SHALL BE IN ACCORDANCE WITH THE SOUTH CAROLINA STATE BLDG. CODE TABLE 2304.10.2 (FASTENING SCHEDULE). FOR NAILING AND STAPLING CONDITIONS NOT COVERED IN THIS TABLE, THE MANUFACTURER'S RECOMMENDATIONS UNLESS THE BUILDING CODE REQUIREMENTS ARE MORE STRINGENT.

WOOD CONNECTORS SHALL BE BY SIMPSON STRONG-TIE COMPANY, INC. SUBSTITUTIONS WILL BE ACCEPTED WITH APPROVAL OF THE ENGINEER. ALL PRODUCTS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS UNLESS THE BUILDING CODE REQUIREMENTS ARE MORE STRINGENT.

ALL ROOF TRUSSES SHALL BE ANCHORED AGAINST UPLIFT AT EACH BEARING POINT BY MEANS OF ANCHORS AS INDICATED ON THE DRAWINGS.

ALL LVL BEAMS SHALL HAVE A MODULUS OF ELASTICITY OF 3000KSI. LVL MANUFACTURERS: BOISE CASCADE; GEORGIA-PACIFIC; "TRUS JOIST" WEYERHAEUSER, OR EQUAL

GENERAL NOTES:

COMPLETE SHOP DRAWINGS FOR CONSTRUCTION OF ALL SPECIALTY ITEMS INCLUDING. BUT NOT LIMITED TO: PRECAST CONCRETE, HOLLOW CORE CONCRETE SLABS, CURTAIN WALL GLAZING SYSTEMS, LIGHT GAUGE STEEL ROOF AND WALL FRAMING, HEAVY TIMBER TRUSSES, GEOPIERS (FOUNDATIONS IMPROVEMENTS), GUARDRAILS AND STAIRS, SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF SOUTH CAROLINA AND SHALL BE MADE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.

RESULTS FOR ALL CONCRETE COMPRESSIVE STRENGTH TESTS SHALL BE AVAILABLE ON THE JOB SITE FOR REVIEW BY THE INSPECTOR.

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FOUNDATION NOTES:

AN ALLOWABLE SOIL BEARING PRESSURE OF 3000 PSF WAS USED FOR THE DESIGN OF SHALLOW FOUNDATIONS BASED ON THE REPORT OF SUBSURFACE EXPLORATION AND GEOTECHNICAL EVALUATION DATED APRIL 19, 2024 PREPARED BY ECS SOUTHEAST, LLC (ECS PROJECT No. 14-10819). IF EXISTING CONDITIONS OR TESTS INDICATE THAT THE SOIL IS SOFT OR OTHERWISE QUESTIONABLE, CONTACT THE ENGINEER BEFORE PLACING CONCRETE.

THE SOILS ENGINEER OR HIS REPRESENTATIVE SHALL INSPECT ALL SUBGRADE WORK PRIOR TO THE PLACEMENT OF ANY REINFORCING STEEL OR CONCRETE AND SHALL PERFORM TESTS TO VERIFY THAT SUCH WORK IS IN CONFORMANCE WITH THE PROCEDURES NOTED IN THE CONTRACT DOCUMENTS.

GENERAL CONCRETE NOTES:

ALL REINFORCING BARS TO CONFORM TO ASTM-A615, GRADE 60. ALL ACCESSORIES ARE TO BE INCLUDED. BARS ARE TO BE COLD BENT IN SHOP. BAR SUPPORTS ARE TO BE SPACED INACCORDANCE WITH ACI 318 AND CRSI. ALL BAR SPLICES ARE TO BE CONSIDERED CLASS "B". CLASS "B" SPLICES ARE TO BE LAPPED A DISTANCE OF 1.3 Ld. COMPRESSION REINFORCING SHALL BE LAPPED A MIN. OF 40 BAR DIAMETERS.

WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 GRADE 60 OR 70. PLACEMENT IN CONCRETE SHALL BE IN THE CENTER OF THE SLAB. UNLESS

NOTED OTHERWISE ON PLAN CONCRETE FOR FOOTINGS AND SLABS ON GRADE SHALL ATTAIN A 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI.

SEE FOUNDATION NOTES FOR SOIL REPORT AND DESIGN CRITERIA

CONCRETE PLACEMENT AND QUALITY: PER RECOMMENDATIONS IN ACI SP-15. DEBRIS: REMOVE ALL DEBRIS FROM FORMS BEFORE PLACING CONCRETE.

SEGREGATION OF AGGREGATES: DO NOT DROP CONCRETE THROUGH REINFORCING STEEL SO AS TO CAUSE SEGREGATION OF AGGREGATES.

INSERTS: SECURELY POSITION ALL ITEMS TO BE CAST IN CONCRETE SUCH AS REINFORCING DOWELS, BOLTS, ANCHORS, PIPES AND SLEEVES IN THE FORMS BEFORE PLACING CONCRETE. CONSTRUCTION JOINTS: OBTAIN THE ARCHITECTS APPROVAL OF JOINT LOCATIONS IN ALL

SLABS, BEAMS, AND SHEAR WALLS. REMOVE LAITANCE AND CLEAN SURFACE OF CONCRETE CONSTRUCTION JOINTS.

PIPES: DO NOT EMBED PIPES OTHER THAN ELECTRICAL CONDUITS IN STRUCTURAL CONCRETE EXCEPT WHERE SPECIFICALLY APPROVED BY THE ARCHITECT. MAXIMUM PIPE SIZE SHALL BE 1/3 OF THE SLAB THICKNESS AND LOCATED AT THE MID DEPTH. MINIMUM SPACING SHALL BE 3 TIMES THE PIPE DIAMETER. PIPES SHALL NOT IMPAIR THE STRENGTH OF THE MEMBER. ANY PIPES OF ALUMINUM ARE PROHIBITED IN STRUCTURAL CONCRETE.

REBAR COVER: ALL DIMENSIONS SHOWING THE LOCATION OF REINFORCING STEEL NOT NOTED AS "CLEAR" ARE TO CENTER OF STEEL MINIMUM REBAR COVER (CLEAR) FOR NON-PRESTRESSED CONCRETE SHALL BE AS FOLLOWS:

LOCATION MIN. COVERAGE (CLEAR) TOLERANCES

CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	3"	3/8"						
EXPC	SED TO EARTH OR WEATHEI	R:						
#5 AND SMALLER BARS	1 1/2"	3/8"						
#6 AND LARGER BARS	2"	3/4"						
NOT EXPOSED TO W	NOT EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND:							
STRUCTURAL SLABS & WALLS	3/4"	1/8"						
SLABS ON GRADE	1 1/2"	1/4"						
BEAMS AND COLUMNS PRIMARY REINFORCEMENT, FIES, STIRRUPS AND SPIRALS)	1 1/2"	3/8"						

TOLERANCES OF REBAR PLACEMENT: TOLERANCE FOR LONGITUDINAL LOCATION OF BENDS AND ENDS OF REINFORCEMENT SHALL BE PLUS OR MINUS 2 INCHES EXCEPT AT DISCONTINUING ENDS OF MEMBERS WHERE TOLERANCES SHALL BE PLUS OR MINUS 1/2 INCH. DO NOT ADD WATER TO CONCRETE MIX AT SITE UNLESS APPROVED BY ARCHITECT

CONCRETE: NORMAL WEIGHT CONCRETE SHALL HAVE A MINIMUM UNIT WEIGHT OF 145 POUNDS PER CUBIC FOOT.

AGGREGATE: NORMAL WEIGHT AGGREGATE CONFORMING TO ASTM C33. AGGREGATE SIZE CONFORMING TO:

3/4" ASTM C67 1" ASTM C67 1 1/2" ASTM C67

CONCRETE AGE: NO MORE THAN 90 MINUTES SHALL ELAPSE BETWEEN CONCRETE BATCHING AND CONCRETE PLACEMENT UNLESS APPROVED BY ARCHITECT.

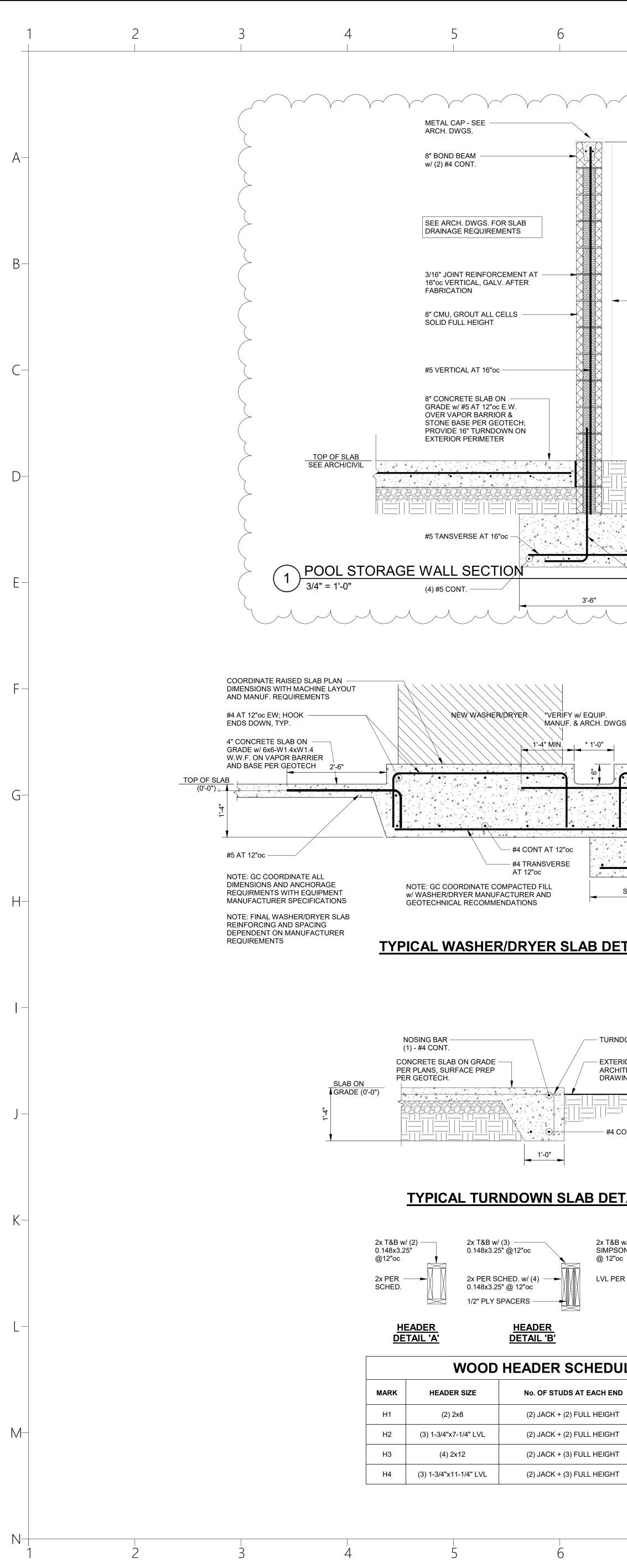
MINIMUM REQUIRED LAP SPLICE LENGTHS									
		TENSION	SPLICES						
BAR SIZE	C	ONCRETE	STRENGT	н	COMPRESSION SPLICES				
	2000 psi	3000 psi	4000 psi	5000 psi					
#3	27"	22"	20"	17"	15"				
#4	36"	29"	25"	22"	20"				
#5	44"	36"	31"	29"	25"				
#6	53"	43"	38"	34"	30"				
#7	77"	62"	55"	49"	35"				
#8	88"	72"	62"	56"	40"				
#9	99"	81"	70"	63"	45"				
#10	110"	90"	78"	70"	50"				

CONCRETE QUALITY

NCRETE USE	STRENGTH AT 28 DAYS	<u>SLUMP</u>	<u>AIR</u>	AGGREGATE SIZE	AGGREGATE TYPE
DUNDATIONS	3000 PSI	4"		1 1/2"	ASTM C33
FERIOR SLABS	4500 PSI	4"	4 - 6%	1"	ASTM C33
ICTURAL WALLS	4000 PSI	3"		1"	ASTM C33
ARD WALLS	4500 PSI	3"		1 1/2"	ASTM C33
AB ON GRADE	3000 PSI	4"		1"	ASTM C33
/ATED SLAB ON IETAL DECK	4000 PSI (LIGHT WEIGHT)	4"		1"	ASTM C33
TENSION SLAB	5000 PSI	4"		1"	ASTM C33
LEVATED SLABS	5000 PSI	4"		1"	ASTM C33

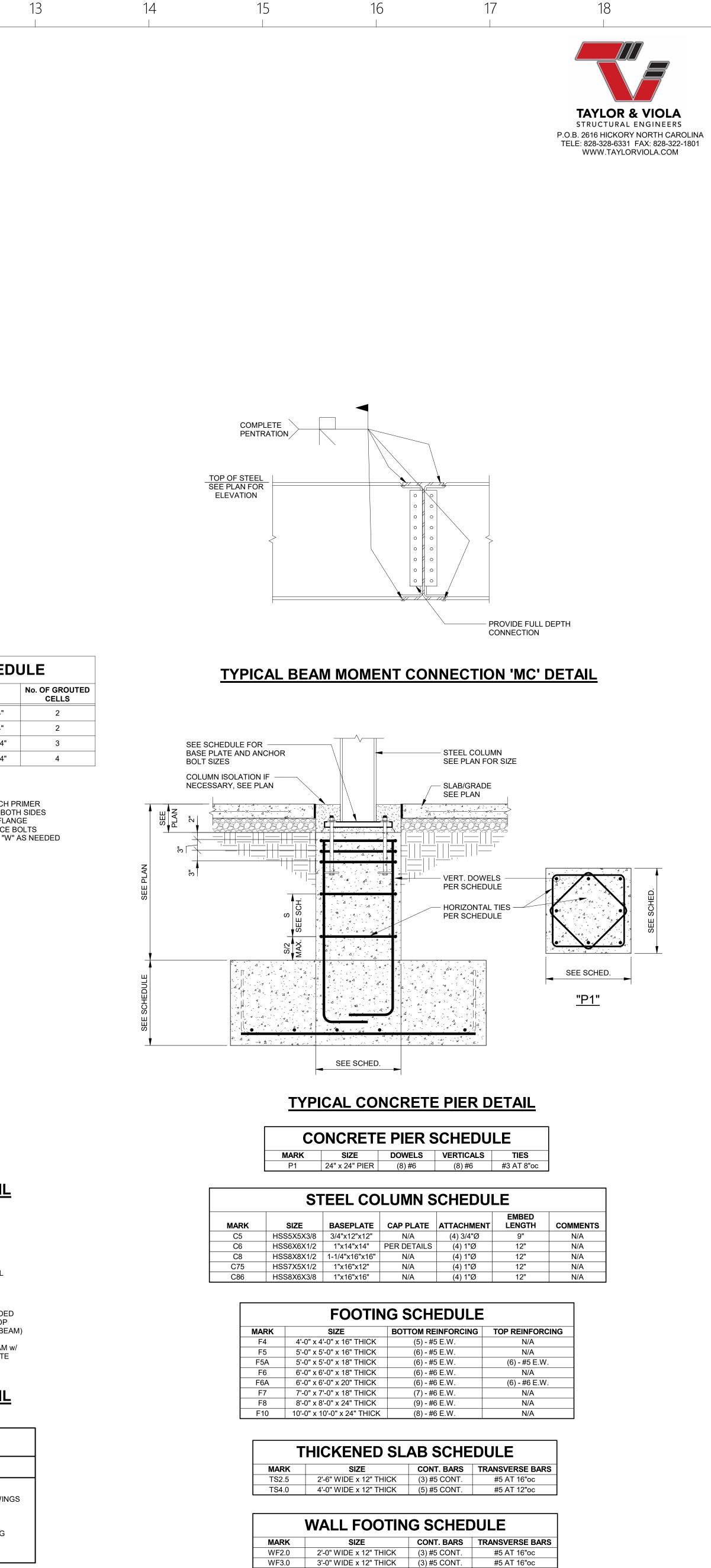


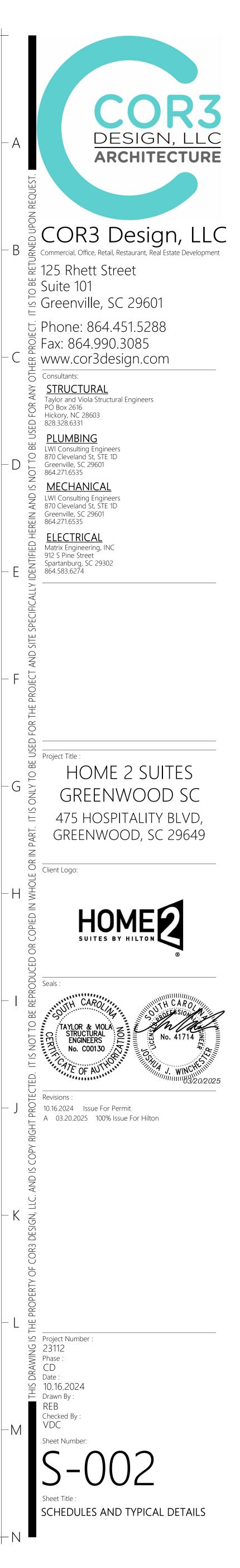


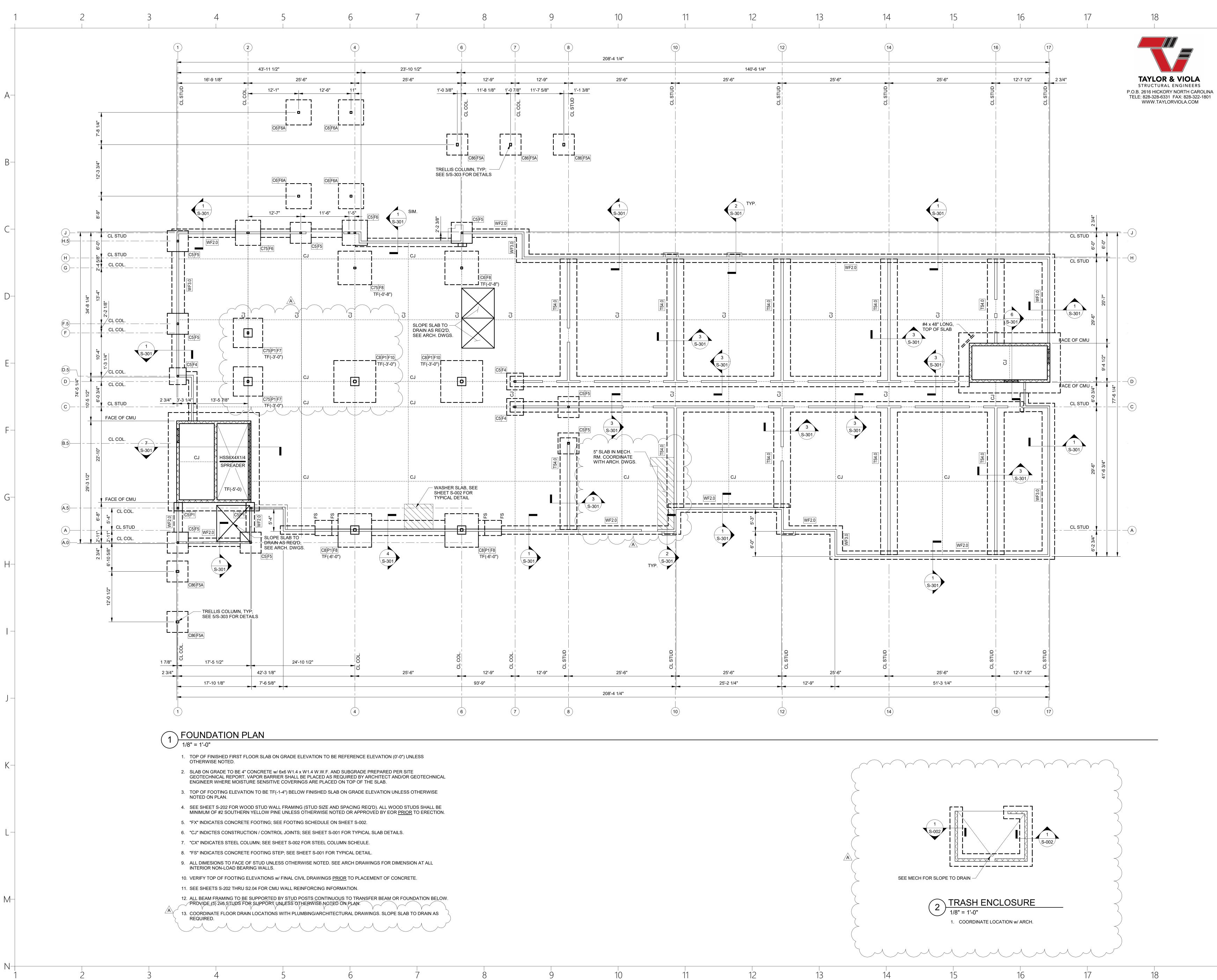


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Image: Set of the set of	SS.	EXTERIOR PLYWOOD OR OMATCH TYP. EXTERIOR PLYWOOD OR OSB WALL SHEATHING; NAILNG PER SHEET S-202 2x6 WOOD STUDS; SEE PLANS FOR SPACING, TYPICAL EXTERIOR SLAB/GRADE;		- ROOF or FLOOR SUPPORT BEAM BEAM BEARING PLATE (SEE SCHEDULE FOR SIZE) - (2) 3/4" DIA. x 6" LONG	MARKMAXBP1BP2BP3BP4NOTES:	KIPS) W I 20 8" 3/4" 30 12" 3/4" 50 16" 1 1/4" 60 16" 1 3/4"
LOOWN WWF INDOM SLABGRADE. SEE INDOM WWF INDOM SLABGRADE. SEE INDOM WWF INDOM SLABGRADE. SEE INDOM SLABGRADE. SEE INDOM SLABGRADE. SEE INDOM SLABGRADE. SEE INDOM WWF INDOM SLABGRADE. SEE INDOM WWF INDOM SLABGRADE. SEE			BEAM BEA	 GROUT WALL SOLID 24" WIDE BE BEAM BEARING WITH (3) #5 VERTICALS (1 EACH CELL) MININ NOT OTHERWISE NOTED IN BEA PLATE SCHEDULE RING SECTION 3/4" Ø x 6 STUDS A WELDED 	ELOW 2. PAINT 1'-0" I 3. WELD NOT OF WALL & RING 4. WHERE BEA OUTSIDE ON OUTSIDE ON T 16" oc (SHOP	LENGTH AT BEAM END w/ ZINC RICH PR REQ'D WHERE SLAB IS PRESENT BOTH RODS EXTEND THROUGH BEAM FLANG
JOOWN WWF INTEL BEARING PLATE SCHEDULE FOR SIZE) RIDE SLAGGADE: SEE INTEL BEARING PLATE INTEL BEARING PLATE SCHEDULE FOR SIZE) WINGS, TYPICAL INTEL SEARING PLATE INTEL BEARING PLATE SCHEDULE FOR SIZE) SCONT. TYPICAL BEAM AND PLATE LINTEL BEARING DET SC (WHERE NOTED) TAIL If WINDS TO NUL WALL TAIL If WINDS TO NUL WALL If WINDS TO NUL WALL <td< th=""><th><u>ETAIL</u></th><th></th><th></th><th></th><th></th><th>- BEAM AND PLATE LINTEL</th></td<>	<u>ETAIL</u>					- BEAM AND PLATE LINTEL
TAIL REINFORCEMENT AT 19°C vertical, Galv. AFTER FABRICATION VERT, REINF, PER SCHEDULE AND/OR ELEVATIONS, TYPICAL VERT, REINF, PER SCHEDULE AND/OR ELEVATIONS, TYPICAL Well be and be and wid (2) # 4 cont. Strubs AT 18°C Well be and Strubs AT 18°C Well be and Wid (2) # 4 cont. Strubs AT 18°C Well be and Wid (2) # 4 cont. MEADER DETAIL 'C' TYPICAL "BB8" DETAIL TYPICAL "LX" DE JLE Intel Size MINIMUM BEARING Remark Sele Strubs AT 18°C Well be and Wid (2) # 4 cont. JLE HEADER DETAIL 'F HEADER DETAIL 'F SEE STRUCTURAL 'S' BOND BEAM WITH (2) # 4 24° EACH END SEE STRUCTURAL 'S' EACH END Mark LINTEL SIZE MINIMUM BEARING SEE STRUCTURAL SEE STRUCTURAL LI'L W16x36 W1/4" BOTTOM PLATE SEE STRUCTURAL SEE ARCHITECTURE	RIOR SLAB/GRADE; SEE HITECTURAL AND CIVIL VINGS, TYPICAL		PL 3/8 TH LENGTH 1/2" Ø x 6 CENTER	IK x BEARING x 6 7/8" WIDE w/ (2) - "WELDED STUDS - ON CMU WALL		SCHEDULE FOR SIZE) - 3/8" STIFFNERS AT 16" O.C. (WHERE NOTED)
DETAIL 'C' Detail 'C' JLE LINTEL SCHEDULE D HEADER DETAIL 'A' MEADER DETAIL 'A' Mark LINTEL SIZE MINIMUM BEARING REMARK MEADER DETAIL 'A' HEADER DETAIL 'C' Mark see structural SEE STRUCTURAL HEADER DETAIL 'B' HEADER DETAIL 'B' SEE STRUCTURAL SEE ARCHITECTURE	3 w/ (2) SON SDWS22300 Doc			REINFORCEMENT AT 16"oc VERTICAL, GALV AFTER FABRICATION VERT. REINF. PER SCHEDULE AND/OR ELEVATIONS, TYPICAL	V.	VERT. REINF. PER SCHEDULE AND/OR ELEVATIONS, TYPICAL 8" BOND BEAM w/ (2) #4 CONT. 3/4" Ø x 6" LONG HEADED STUDS AT 16" oc (SHOP WELDED TO TOP OF BEAM) W-SHAPE LINTEL BEAM w/ 1/4" x 7" BOTTOM PLATE
D HEADER DETAIL D HEADER DETAIL 'A' MEADER DETAIL 'A' MARK LINTEL SIZE MINIMUM BEARING REMARK MARK BB8 8" BOND BEAM WITH (2) #4 24" EACH END SEE STRUCTURAL FOR LINTEL LOCATION PLATE HEADER DETAIL 'B' HEADER DETAIL 'B' SEE ARCHITECTURE SEE ARCHITECTURE	DETAIL 'C'		<u>TY</u>	PICAL "BB8" DET	AIL <u>TYF</u>	PICAL "LX" DETAIL
BB8 8" BOND BEAM WITH (2) #4 24" EACH END F HEADER DETAIL 'C' L1 W16x36 w/ 1/4" BOTTOM PLATE 8" EACH END F HEADER DETAIL 'B' SEE STRUCTURAL FOR LINTEL LOCATION PLATE SEE STRUCTURAL FOR LINTEL LOCATION PLATE			MARK			REMARKS
HEADER DETAIL 'C' L3 W24x55 w/ 1/4" BOTTOM PLATE 16" EACH END w/ BP4	THEADER DETAIL 'C		L1 L2	W16x36 w/ 1/4" BOTTOM PLATE W16x50 w/ 1/4" BOTTOM PLATE	8" EACH END 16" EACH END	SEE STRUCTURAL DRAWINGS FOR LINTEL LOCATIONS SEE ARCHITECTURAL DRAWINGS FOR OPENING WIDTHS, TYPICAL

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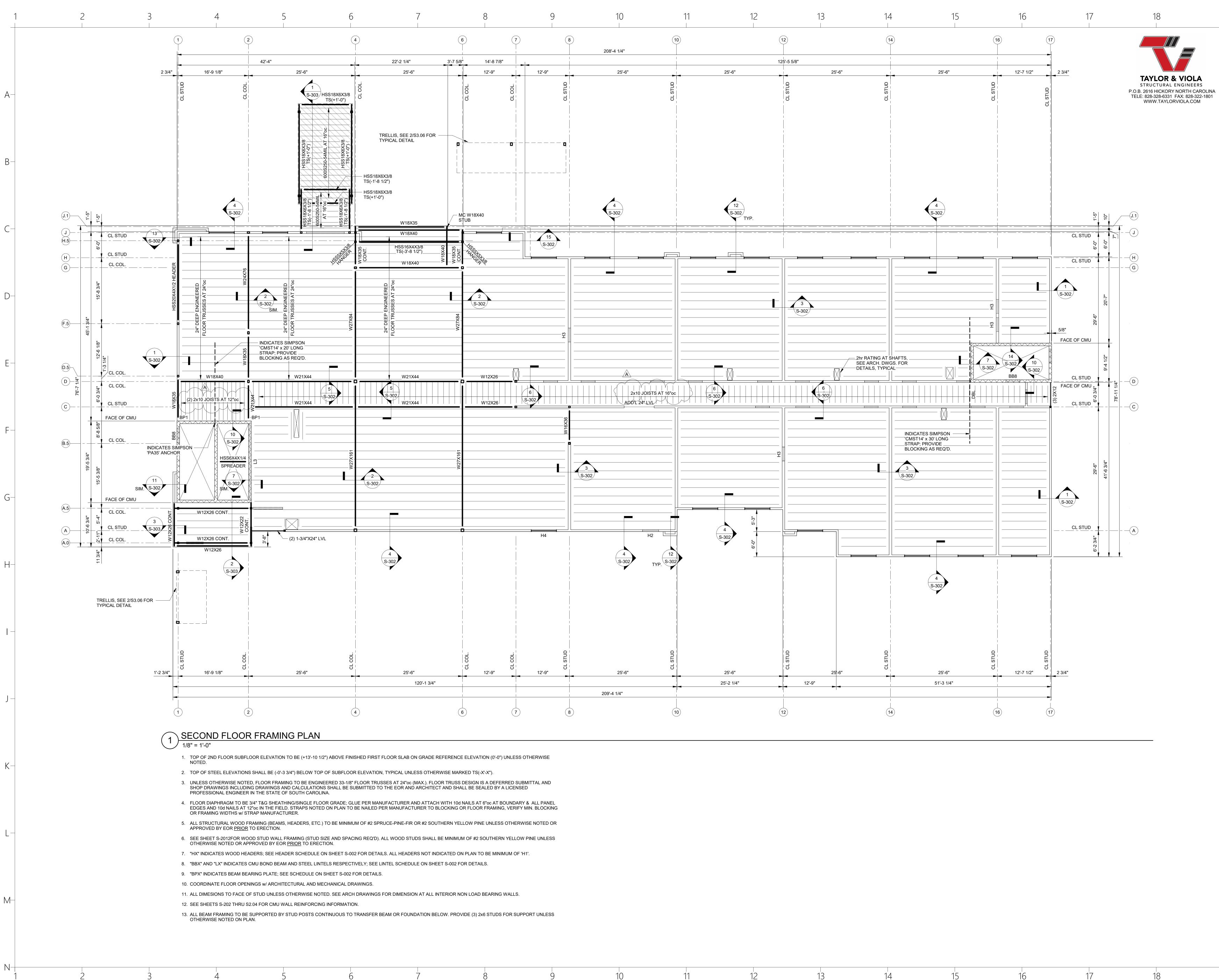




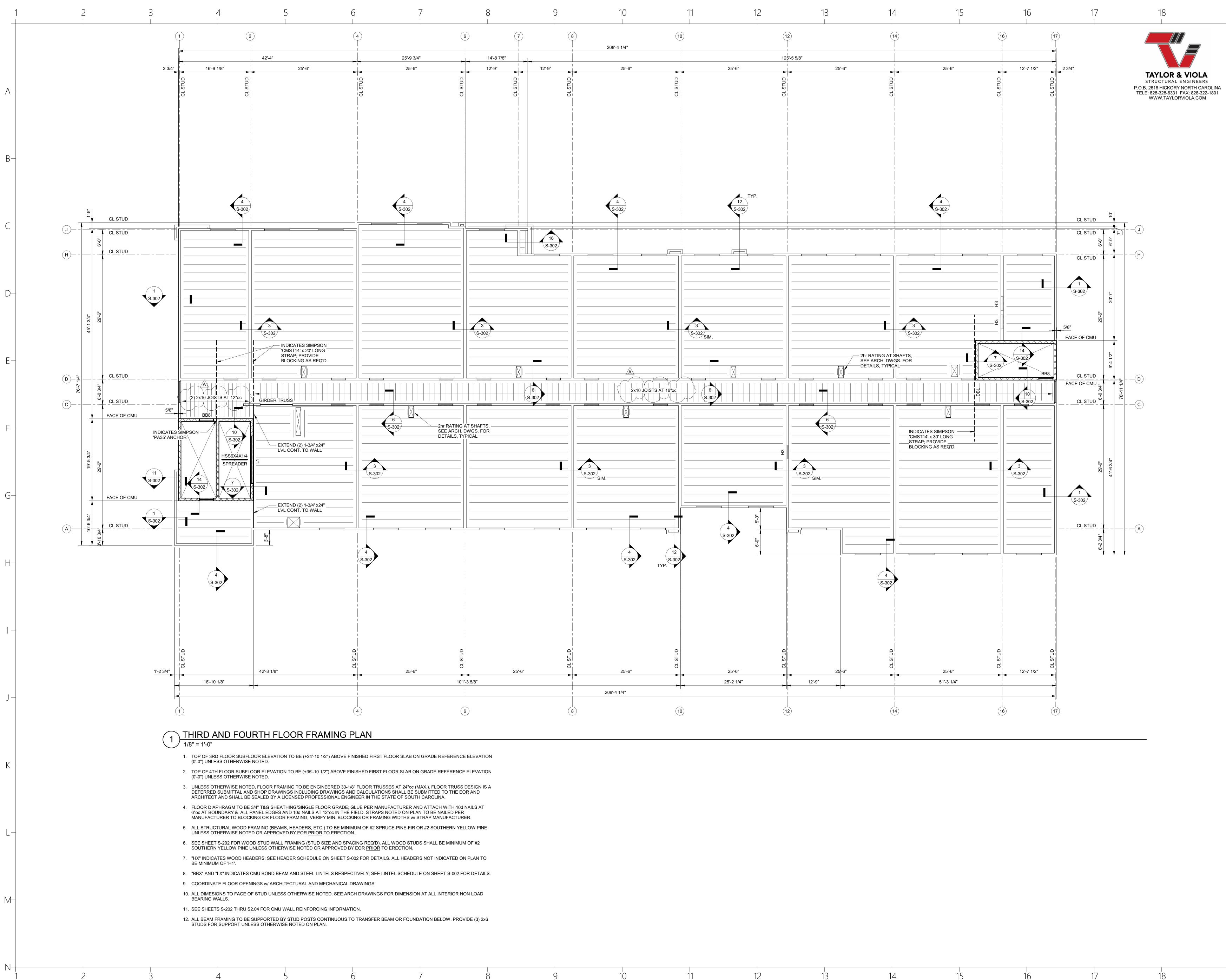


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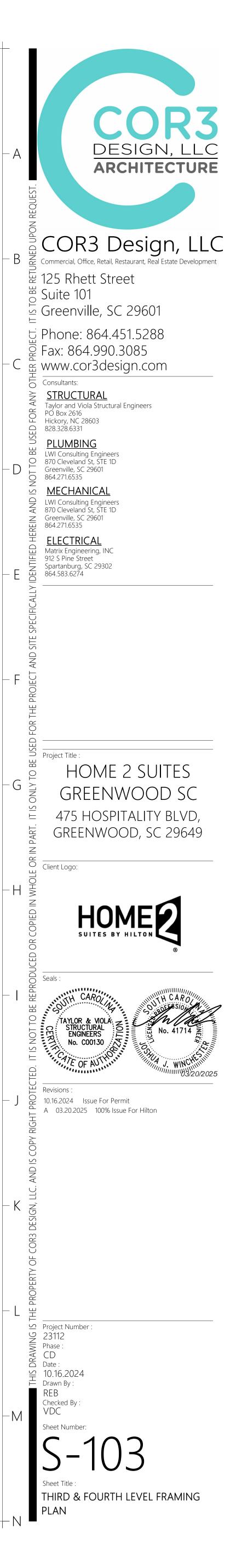


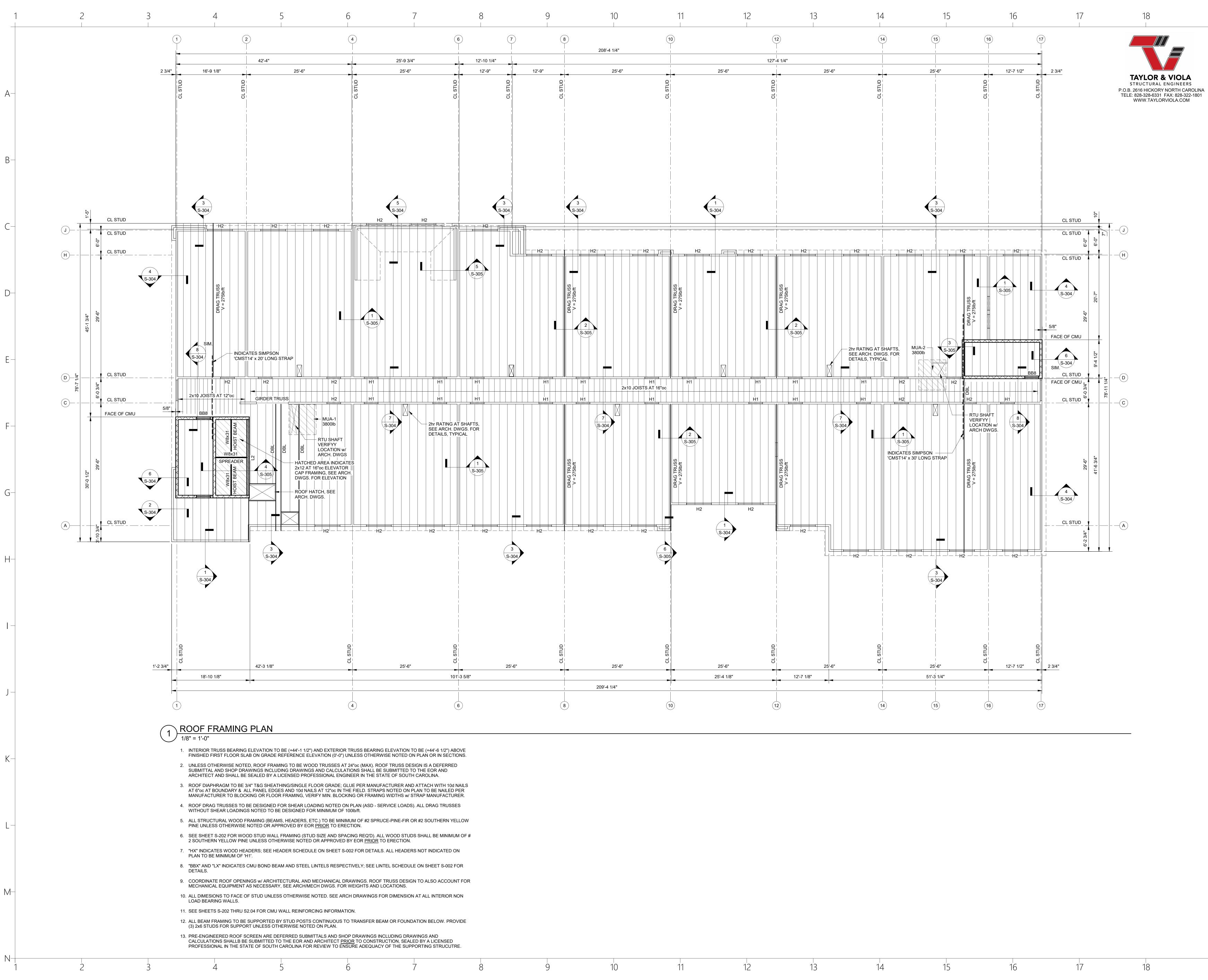


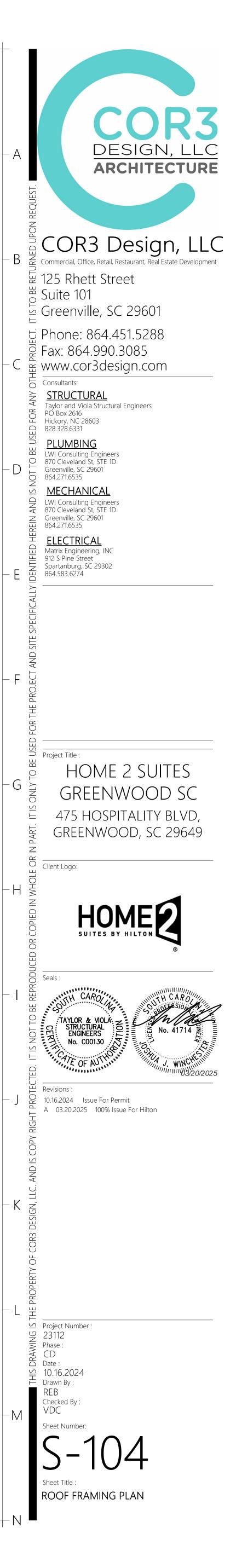


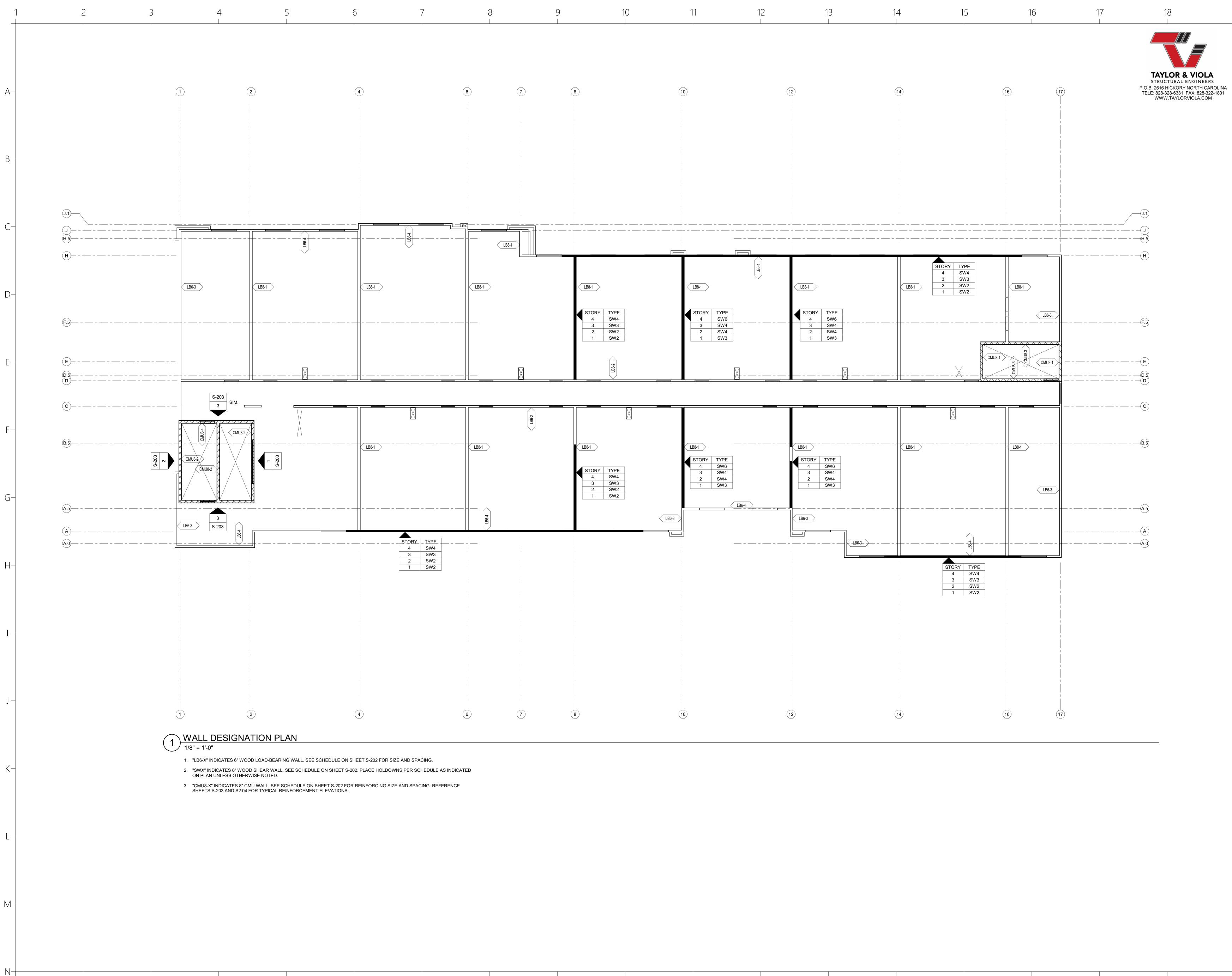


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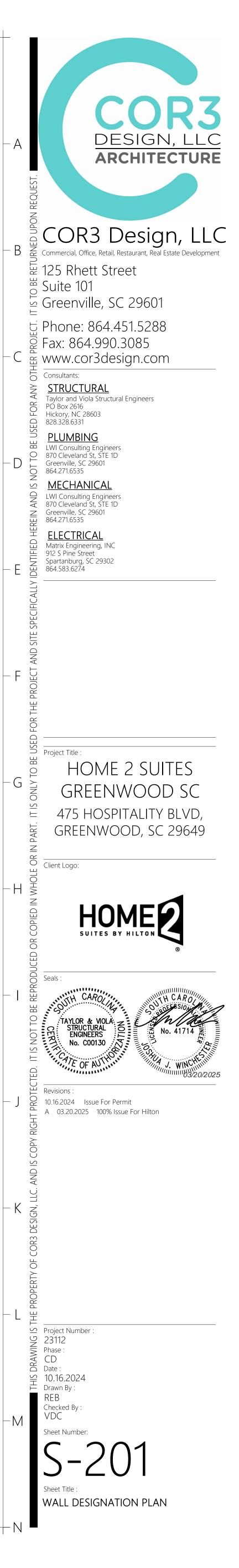


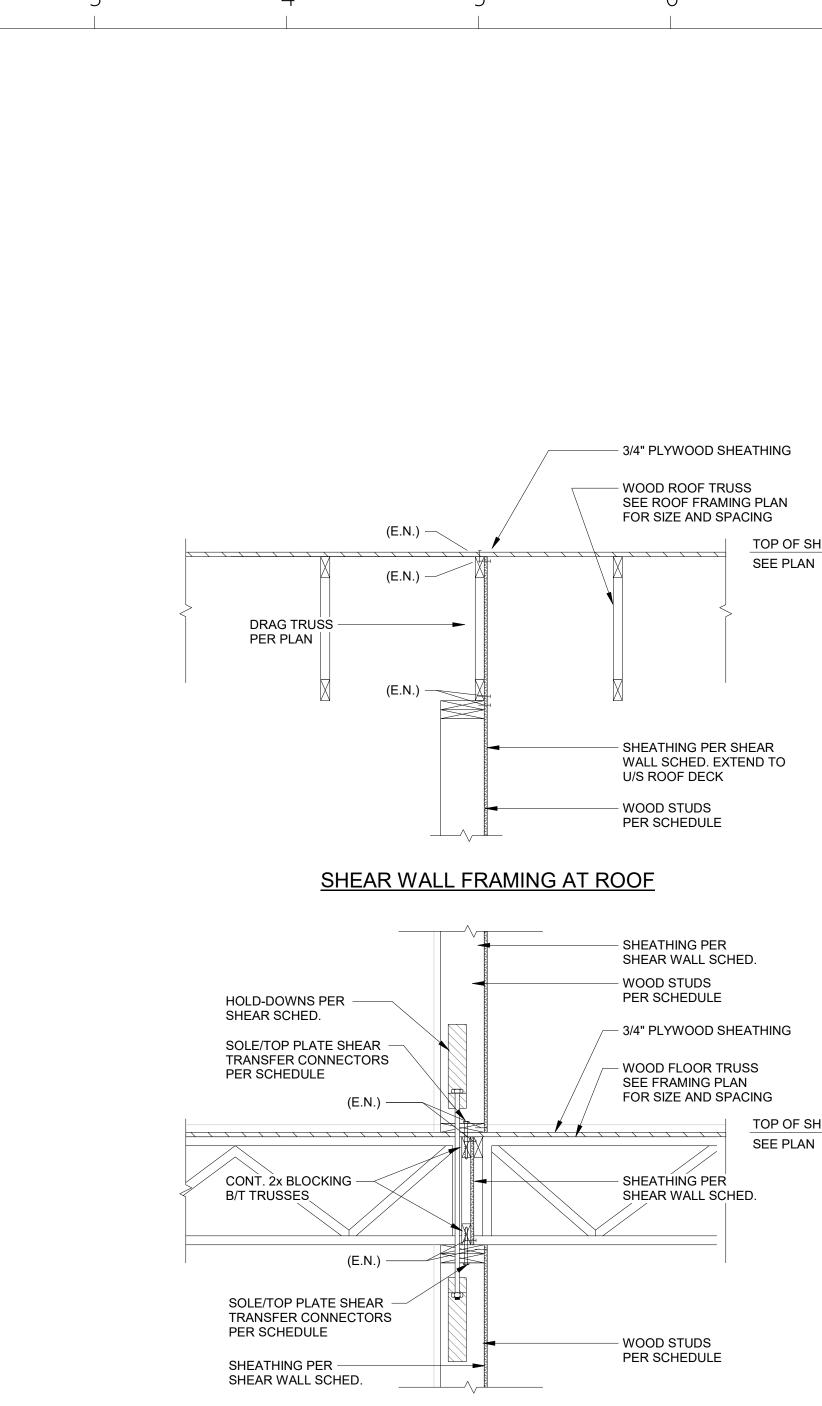






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SHEAR WALL FRAMING AT TYPICAL FLOOR LEVELS (HOLDOWN DETAIL)

WOOD SHEAR WALL FRAMING SCHEDULE

PLAN NOTATION	SHEATHING ²	EDGE NAILING	FIELD ⁶ NAILING	WIDTH OF NAILED FACE ⁷ FOR PANEL EDGE FRAMING/BLOCKING	BOUNDARY ELEMENT	BOUNDARY ELEMENT HOLD-DOWN ANCHOR AT GRADE	INTERMEDIATE ANCHORS AT GRADE	STRAPS/HOLDOWNS	SOLE/TOP PLATE SHEAR TRANSFER CONNECTORS	ROOF DRAG TRUSS TO TOP PLATE
SW6	1/2" (15/32") ONE FACE	10d @ 6"oc	10d @ 6"oc	2" NOMINAL	(3) 2x TO MATCH STUD	(1) SIMPSON HDU4-SDS2.5 w/ 5/8"Ø THREADED ROD OR SIMPSON PAB w/ 12" EMBED.	1/2"Ø ANCHOR BOLTS w/ 7" EMBED AT 48"oc	(1) SIMPSON HDU4-SDS2.5 w/ 5/8"Ø THREADED ROD	(1) SIMPSON 'SDWS22600DB' AT 16"oc	(1) SIMPSON A34 AT 16"oc
SW4	1/2" (15/32") ONE FACE	10d @ 4"oc	10d @ 6"oc	2" NOMINAL	(4) 2x TO MATCH STUD	(1) SIMPSON HDU8-SDS2.5 w/ 7/8"Ø THREADED ROD OR SIMPSON PAB w/ 20" EMBED.	1/2"Ø ANCHOR BOLTS w/ 7" EMBED AT 48"oc	(1) SIMPSON HDU5-SDS2.5 w/ 5/8"Ø THREADED ROD	(1) SIMPSON 'SDWS22600DB' AT 12"oc	(1) SIMPSON A35 AT 16"oc
SW3	1/2" (15/32") ONE FACE	10d @ 3"oc	10d @ 6"oc	3" NOM. OR (2) 2x CONNECTED W/ (2) 16d STAGGERED AT 3"oc	(5) 2x TO MATCH STUD	(1) SIMPSON HDU14-SDS2.5 w/ 1"Ø THREADED ROD OR SIMPSON PAB w/ 20" EMBED.	1/2"Ø ANCHOR BOLTS w/ 7" EMBED AT 32"oc	(1) SIMPSON HDU8-SDS2.5 w/ 7/8"Ø THREADED ROD	(1) SIMPSON 'SDWS22600DB' AT 8"oc	(1) SIMPSON HGA10 w/ 3" SDS AT 16"oc
SW2	1/2" (15/32") ONE FACE	10d @ 2"oc	10d @ 6"oc	3" NOM. OR (2) 2x CONNECTED W/ (2) 16d STAGGERED AT 3"oc	(6) 2x TO MATCH STUD	(1) SIMPSON HDU14-SDS2.5 w/ 1"Ø THREADED ROD OR SIMPSON PAB w/ 20" EMBED.	1/2"Ø ANCHOR BOLTS w/ 7" EMBED AT 16"oc	(1) SIMPSON HDU11-SDS2.5 w/ 1"Ø THREADED ROD	(1) SIMPSON 'SDWS22600DB' AT 6"oc	(1) SIMPSON HGA10 w/ 3" SDS AT 12"oc

1. ALL WOOD STUDS AND BLOCKING IN SHEAR WALLS SHALL BE A MINIMUM OF #2 SOUTHERN PINE UNLESS OTHERWISE APPROVED BY EOR PRIOR TO INSTALLATION.

2. ALL WOOD PANELS SHALL BE ZIP SYSTEM WALL SHEATHING (<u>STRUCTURAL 1 RATED OSB</u>). SEE ARCH. DWGS. FOR ADDITIONAL DETAILS.

3. WHERE SPECIFIED NAIL SPACING 3" OR LESS AT PANEL EDGES, EDGE NAILING FOR ADJACENT PANELS SHALL BE STAGGERED.

4. FOUNDATION ANCHOR BOLTS SHALL HAVE A STEEL PLATE WASHER UNDER EACH NUT NOT LESS THAN 0.229"x3"x3".

5. PROVIDE SOLID BLOCKING UNDER ALL SHEAR WALL BOUNDARY ELEMENTS FOR FULL BEARING OF BOUNDARY ELEMENT.

SEE WOOD STUD WALL FRAMING SCHEDULE THIS SHEET FOR STUD SIZE AND SPACING. 6. WHERE STUDS ARE SPACED LESS THAN 24"oc, THE MAXIMUM NAIL SPACING IN PANEL FIELD MAY BE 12"oc.

7. ALL SHEAR WALLS SHALL BE BLOCKED AT PANEL EDGES WHERE PANELS ARE INTERRUPTED BETWEEN FLOOR LEVELS. BLOCKING SHALL BE OF MINIMUM WIDTH PER SCHEDULE AND PANELS SHALL BE NAILED AT EDGES INTO BLOCKING PER SCHEDULE. SEE DETAILS GENERAL

NOTES FOR GENERIC SHEAR WALL COMPONENTS. 8. HOLD-DOWNS AT WALL SEGMENT ENDS SHALL BE AS LISTED IN SCHEDULE TYPICAL UNLESS NOTED OTHERWISE IN PLAN. HOLD-DOWNS SHALL BE PROVIDED AT EACH END OF EACH SHEAR WALL AND AT EACH SIDE OF EACH OPENING.

	WOOD STUD WALL FRAMING SCHEDULE								
LOCATION	INTERIOR LOAD-BEARING WALLS (LB8-1)	CORRIDOR WALLS (LB6-2)	EXTERIOR (END) LOAD-BEARING WALLS (LB6-3)	EXTERIOR (SIDE) LOAD BEARING WALLS (LB6-4)					
4th FL - ROOF	2x8 STUDS AT 16"oc	2x6 STUDS AT 16"oc	2x6 STUDS AT 16"oc	2x6 STUDS AT 16"oc					
3rd FL - 4th FL	2x8 STUDS AT 16"oc	2x6 STUDS AT 16"oc	2x6 STUDS AT 16"oc	2x6 STUDS AT 16"oc					
2nd FL - 3rd FL	(2) 2x8 STUDS AT 16"oc	(2) 2x6 STUDS AT 16"oc	(2) 2x6 STUDS AT 16"oc	2x6 STUDS AT 16"oc					
1st FL - 2nd FL	(2) 2x8 STUDS AT 16"oc	(2) 2x6 STUDS AT 16"oc	(2) 2x6 STUDS AT 16"oc	(2) 2x6 STUDS AT 16"oc					

1. ALL WOOD STUDS SHALL HAVE BLOCKING AT STUD MID-HEIGHT OR AT 4'-0"oc FOR WALLS TALLER THAN 10'-0"

2. ALL WOOD STUDS AND BLOCKING TO BE MIN. OF SOUTHERN YELLOW PINE #2 UNLESS OTHERWISE NOTED. ANY SUBSTITUTION MUST BE APPROVED BY EOR <u>PRIOR</u> TO ERECTION.

3. ALL EXTERIOR WOOD SHEATHING PANELS SHALL BE 1/2" (MIN.) STRUCTURAL SHEATHING GRADE APA PLYWOOD OR OSB. SEE ARCH. DWGS.

FOR ADDITIONAL DETAILS.

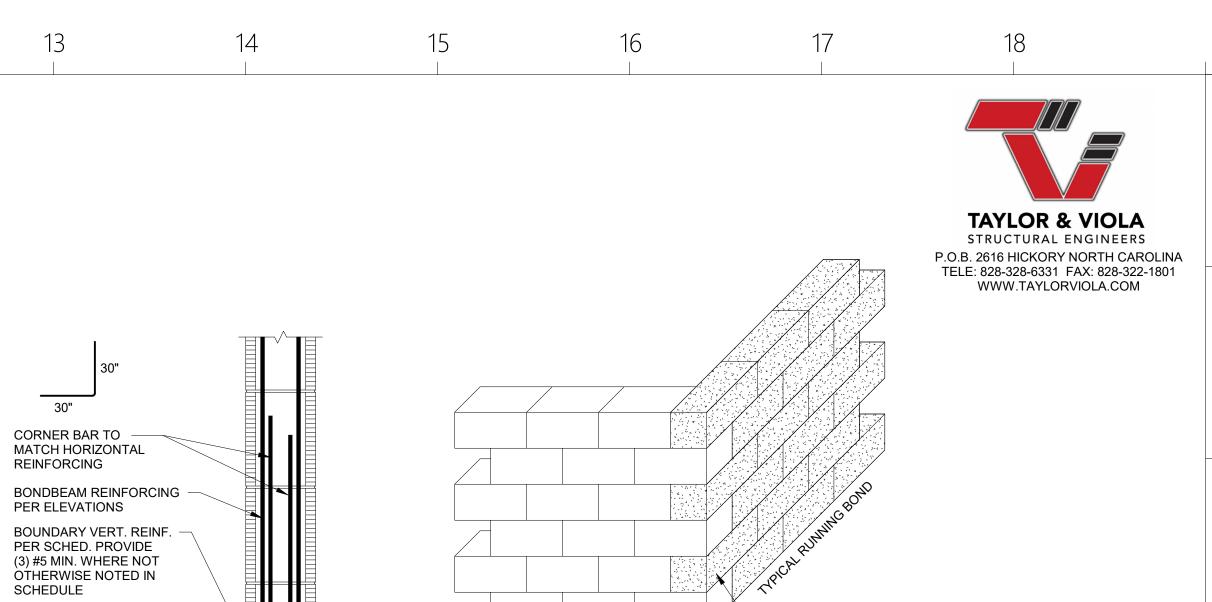
4. ALL EXTERIOR SHEATING SHALL BE ATTACHED USING 10d NAILS w/ 6"oc EDGE SPACING & 12"oc FIELD SPACING UNLESS OTHERWISE NOTED IN WOOD SHEAR WALL FRAMING SCHEDULE.

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TOP OF SHEATHING SEE PLAN

LEVEL 3 ╞<mark>╧╢╴╴╴┶╢╴╴┑┢╴╴</mark>╫┝╴╸┟┪╸╷╸┝╢╴╴╷┢╴╴╴┶╢╴ ╦╬╦╪╪╬┝┺╹┾╍┠╾╎╍┨╺╬╴╧╓╴┟╗╶╴╷╴╴┾╷╴╴ U U H H U U STRAPS / HOLD-DOWNS PER SHEAR SCHED. LEVEL 2 <mark>╵[╽]╞╕╸┙╸┝╪╵┍╕╶╢╴╵╵╵╵╵╵ ╲┙╞╕╸╵╾┝╪╵╾╕╶╢╴╵╵╵╵╵╵</mark> SOLE/TOP PLATE SHEAR TRANSFER CONNECTORS PER SCHEDULE TOP OF SHEATHING PANEL FIELD NAILING - KING STUDS SEE SCHEDULE PER SCHEDULE PANEL EDGE NAILING (E.N.) PER SCHEDULE ₽₩₽₽₽ - HEADER SEE SCHEDULE BOUNDARY ELEMENT -PER SCHED. - JACK STUDS SEE SCHEDULE PANEL EDGE FRAMING AND BLOCKING PER - WALL STUDS SCHED. SEE SCHEDULE BOUNDARY ELEMENT HOLD-DOWN PER SCHED. S.O.G.

TYPICAL WOOD SHEAR WALL



TYPICAL RUNNING BOND

WALL CORNER DETAIL AT BOND BEAM

TYPICAL RUNNING BOND

- CORNERS TO BE TOOTHED TOGETHER w/

ALTERNATING COURSES

TYPICAL

		(CMU8-1)				
LOCATION	HORIZONTAL REINFORCMENT	FIELD VERTICAL REINFORCMENT	BOUNDARY VERTICAL REINFORCMENT	REMARKS		
4TH - ROOF	8" BOND BEAM w/ (2) #4 AT 48"oc	#4 AT 24" oc	(2) #5			
3RD - 4TH	8" BOND BEAM w/ (2) #4 AT 48"oc	#4 AT 16" oc	(3) #5	SEE NOTES FOR CORNER REINF. INFORMATION		
2ND - 3RD	8" BOND BEAM w/ (2) #4 AT 48"oc	#5 AT 16" oc	(4) #6			
1ST - 2ND	8" BOND BEAM w/ (2) #4 AT 48"oc	#6 AT 8" oc	(4) #7			
		(CMU8-2)	· · · · ·			
LOCATION	HORIZONTAL REINFORCMENT	FIELD VERTICAL REINFORCMENT	BOUNDARY VERTICAL REINFORCMENT	REMARKS		
4TH - ROOF	8" BOND BEAM w/ (2) #4 AT 48"oc	#4 AT 48" oc	(2) #5	GROUT ELEVATOR		
3RD - 4TH	8" BOND BEAM w/ (2) #4 AT 48"oc	#5 AT 24" oc	(3) #5	WALLS SOLID		
2ND - 3RD	8" BOND BEAM w/ (2) #4 AT 48"oc	#5 AT 16" oc	(3) #6	SEE NOTES FOR CORNER REINF. INFORMATION		
1ST - 2ND	8" BOND BEAM w/ (2) #4 AT 48"oc	#6 AT 16" oc	(4) #6			
		(CMU8-3)				
LOCATION	HORIZONTAL REINFORCMENT	FIELD VERTICAL REINFORCMENT	BOUNDARY VERTICAL REINFORCMENT	REMARKS		
4TH - ROOF	8" BOND BEAM w/ (2) #4 AT 48"oc	#4 AT 48" oc	(2) #5			
3RD - 4TH	8" BOND BEAM w/ (2) #4 AT 48"oc	#4 AT 24" oc	(2) #5	SEE NOTES FOR		
2ND - 3RD	8" BOND BEAM w/ (2) #4 AT 48"oc	#5 AT 16" oc	(3) #6	CORNER REINF. INFORMATION		
1ST - 2ND	8" BOND BEAM w/ (2) #4 AT 48"oc	#6 AT 16" oc	(3) #6			
·		(CMU8-4)				
LOCATION	HORIZONTAL REINFORCMENT	FIELD VERTICAL REINFORCMENT	BOUNDARY VERTICAL REINFORCMENT	REMARKS		
4TH - ROOF	8" BOND BEAM w/ (2) #4 AT 48"oc	#4 AT 48" oc	(2) #5			
3RD - 4TH	8" BOND BEAM w/ (2) #4 AT 48"oc	#5 AT 24" oc	(2) #5	GROUT ELEVATOR WALLS SOLID		
2ND - 3RD	8" BOND BEAM w/ (2) #4 AT 48"oc	#5 AT 24" oc	(3) #6	SEE NOTES FOR CORNER REINF.		
1ST - 2ND	8" BOND BEAM w/ (2) #4 AT 48"oc	#6 AT 16" oc	(3) #6	INFORMATION		

BOUNDARY REINF.	GREA	ER REINF. TO MATCH TER BOUNDARY . SIZE (TYP)	GROUT BOUNDARY CELLS SOLID (TYP)		BOUNDARY REINF.
ER REINF.	BOUNDARY REINF.		VERT. REINF.	BOUNDARY REINF.	CORNER REINF.

1. WALLS SHALL BE REINFORCED AS INDICATED IN MASONRY WALL SCHEDULES. SEE ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATION OF ALL OPENINGS. GROUT FILL ALL CMU CELLS SOLID WHERE REINFORCING OCCURS.

2. BOUNDARY VERTICALS TO OCCUR AT FIRST UNINTERRUPTED CELL FULL HEIGHT OR END OF WALL. BEARING PLATES MAY NOT INTERUPT BOUNDARY VERTICALS; COORINDATE w/ BEARING PLATE SCHEDULE AND PROVIDE ADDITIONAL VERTICALS FOR EACH GROUTED CELL. 3. HORIZONTAL REINFORCING SHALL SPAN CONTINUOUS w/ BONDED CORNERS AND SHALL HOOK AROUND

VERTICAL REINFORCING AT ALL TERMINATIONS. SPACE REINFORCING SO THAT MAXIMUM C/C SPACING DOES NOT EXCEED VALUES SHOWN IN SCHEDULE AND SO THAT REINFORCING SPANS CONTINUOUS BETWEEN LAP SPLICES.

4. UNLESSS OTHERWISE NOTED IN REINF. SCHEDULE, INSTALL (2) #4 HORIZONTAL BAR IN BONDS AT THE TOP AND BOTTOM OF WALL OPENINGS, EXTEND BEYOND THE VERTICAL REINFORCING ADJACENT TOTHE OPENING 48 BAR DIAMETERS OR 24" MINIMUM.

5. INSTALL (1) #4 (MIN.) AT THE FIRST OPEN CELL ADJACENT TO AN OPENING AND FIRST CELL BEYOND END OF LINTEL; DOWEL AT TOP AND BOTTOM INTO ADJACENT LEVEL BOND BEAMS AND/OR FOUNDATION.

6. UNLESS MORE STRINGENT REINFORCING IS REQUIRED PER MASONRY REINFORCING SCHEDULE, HORIZONTAL REINFORCING SHALL CONSIST OF A BOND BEAM w/ (2) #4 CONTINOUS UNDER EACH FLOOR AND ROOF PLANK w/ 3/16" HORIZONTAL JOINT REINFORCING AT 16" oc (GALV. AFTER FABRICATION) BETWEEN FLOORS.

7. VERTICAL REINFORCEMENT SHALL BE PROVIDED AT THE ENDS OF ALL WALLS AND IN THE FIRST CELL EACH SIDE OF ALL MASONRY CONTROL JOINTS. PROVIDE A MINIMUM OF (1) #5 VERTICAL BAR UNLESS OTHERWISE NOTED IN THE MASONRY REINFORCMENT SCHEDULE.

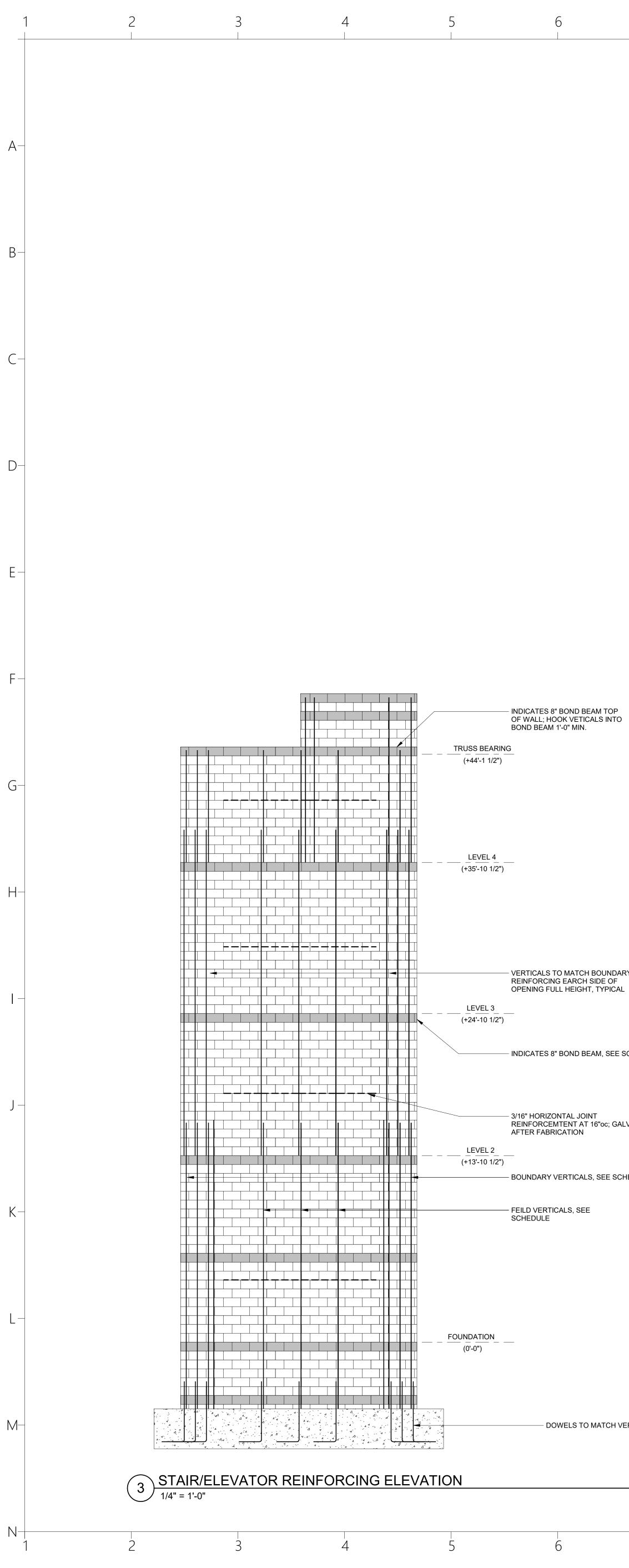
8. INSTALL ONE VERTICAL CORNER BAR AT ENDS OF ALL WALLS, TYPICAL. CORNER REINFORCMENT SHALL MATCH THE GREATER BAR DIAMETER IF TWO SEPERATE VERTICAL BOUNDARY BAR SIZES OCCUR IN

15

CORNE

ADJACENT WALLS.





				LEVEL 4 (+35'-10 1/2")		
				(+33-10 1/2)		
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DARY DF					ALS TO MATCH BOUNDARY	
CAL				OPENI	RCING EARCH SIDE OF IG FULL HEIGHT, TYPICAL	
				<u>LEVEL 3</u> (+24'-10 1/2")		
EE SCHEDULE					TES 8" BOND BEAM, SEE SCHEDULE	
GALV.				~		
GALV.				REINFO	ORIZONTAL JOINT ORCEMTENT AT 16"oc; GALV. FABRICATION	
				<u>LEVEL 2</u> (+13'-10 1/2")		
SCHEDULE					ARY VERTICALS, SEE SCHEDULE	
				BOUND	ART VERTICALS, SEE SCHEDULE	
		╷╫╫╧╷╷╶╴╴╴		FIELD \ SCHED	/ERTICALS, SEE ULE	
				BOUND	ALS TO MATCH ARY REINFORCING LINTEL BEARING, TYP	
				UNDER	LINTEL DEARING, TTP	
				FOUNDATION		
				(0'-0")		
				DOWEL	S TO MATCH VERTICALS, TYPICAL	
H VERTICALS, TYPICAL						
	2 STAIR/ELEV 1/4" = 1'-0"	ATOR REINF	ORCING ELEVATION	JN		
7	8	9	10	11	12	13

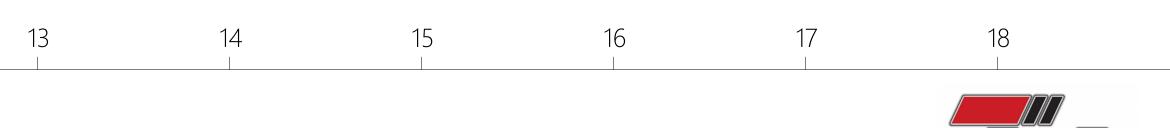
INDICATES 8" BOND BEAM TOP
 OF WALL; HOOK VETICALS INTO

BOND BEAM 1'-0" MIN.

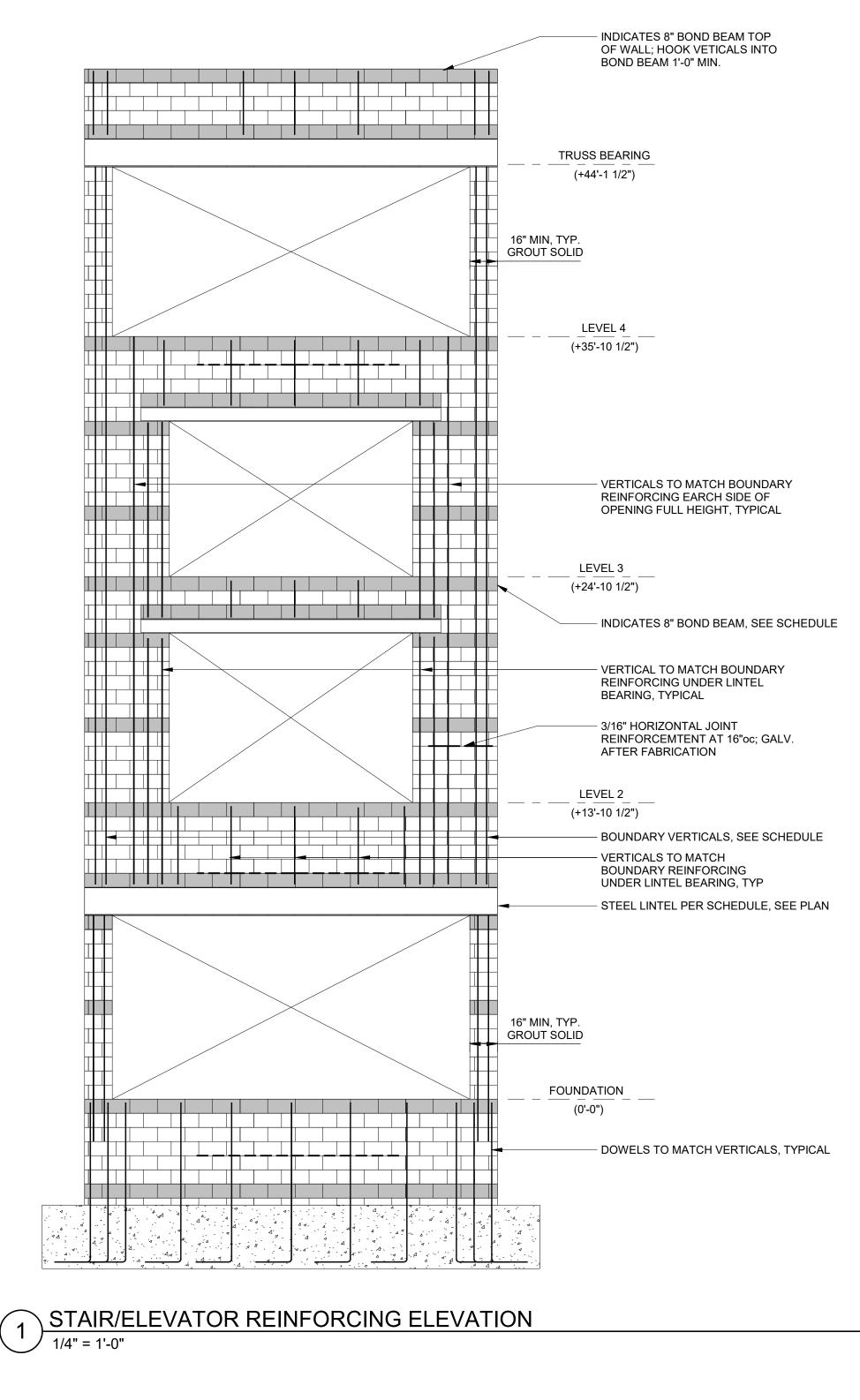
TRUSS BEARING

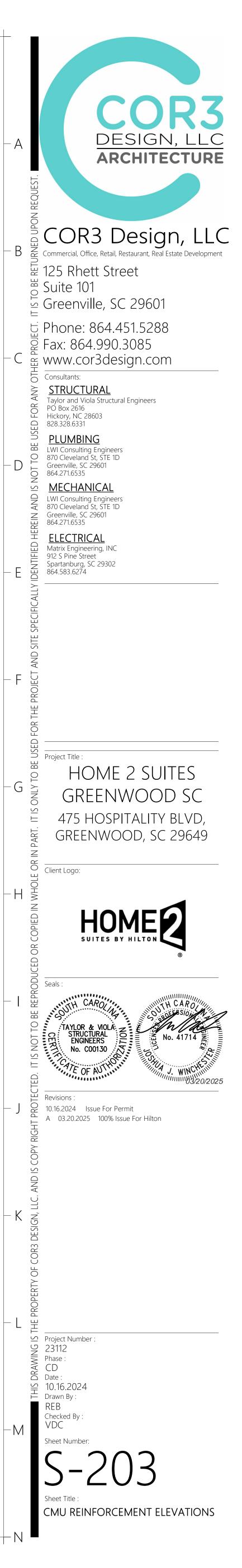
(+44'-1 1/2")

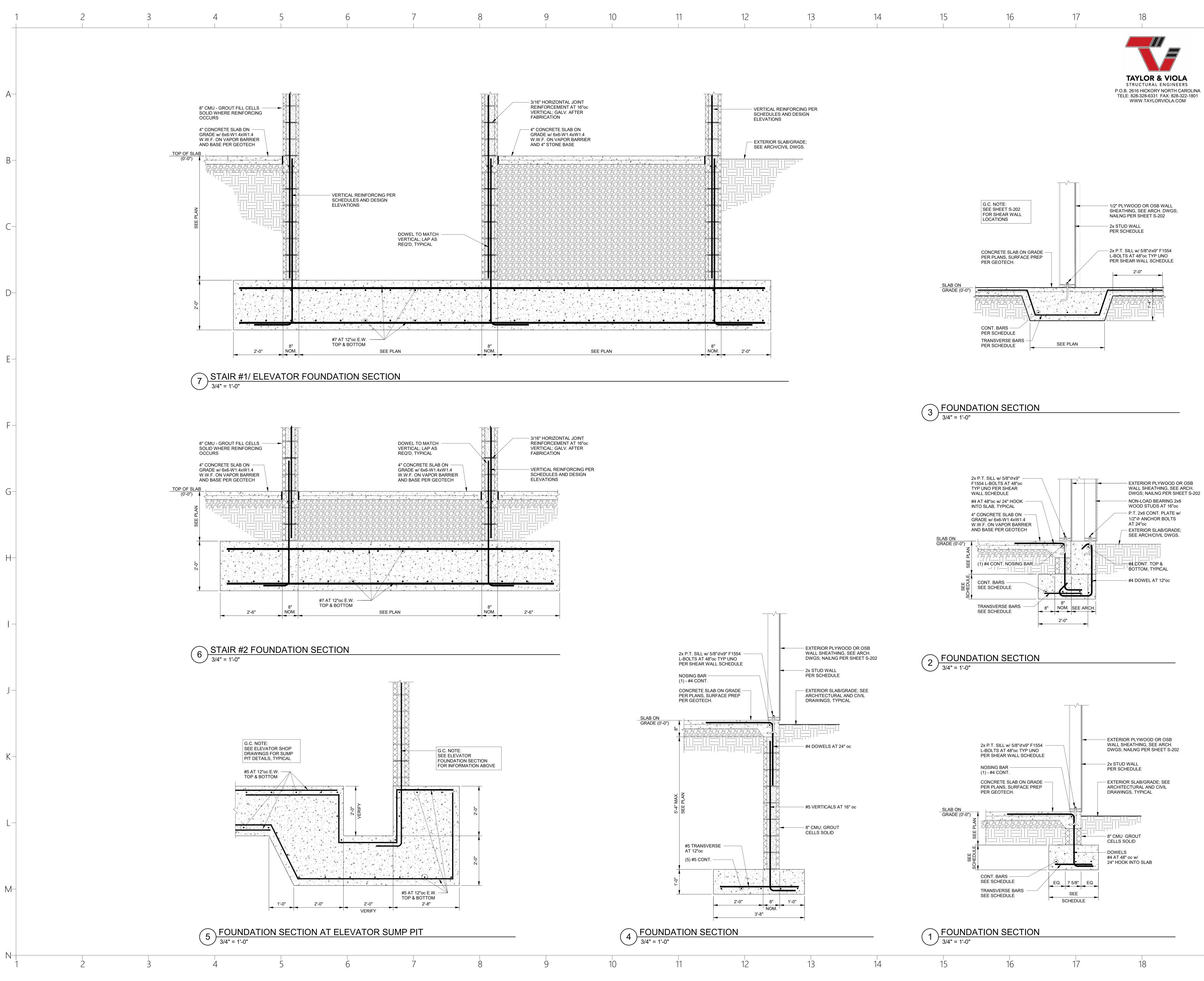
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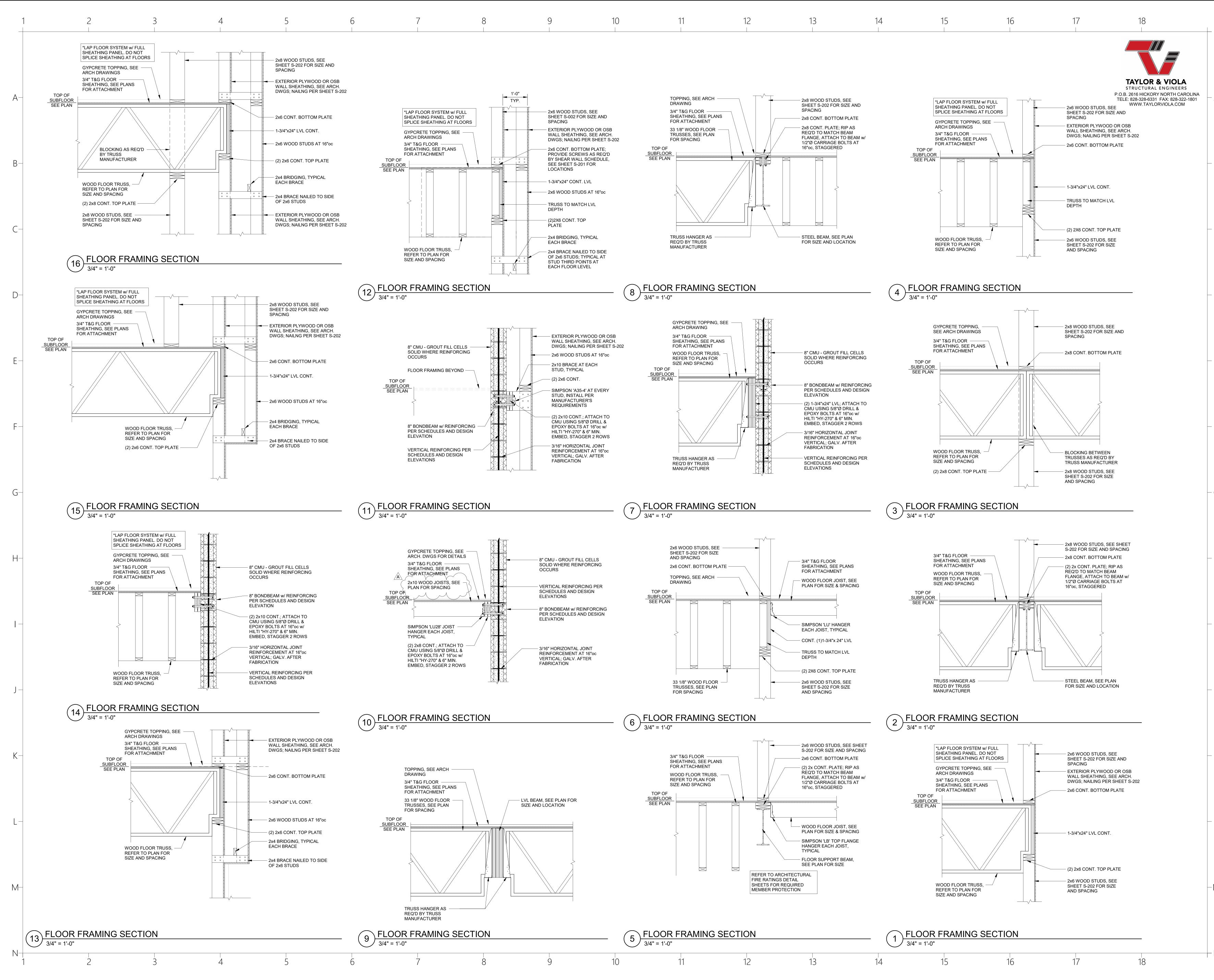


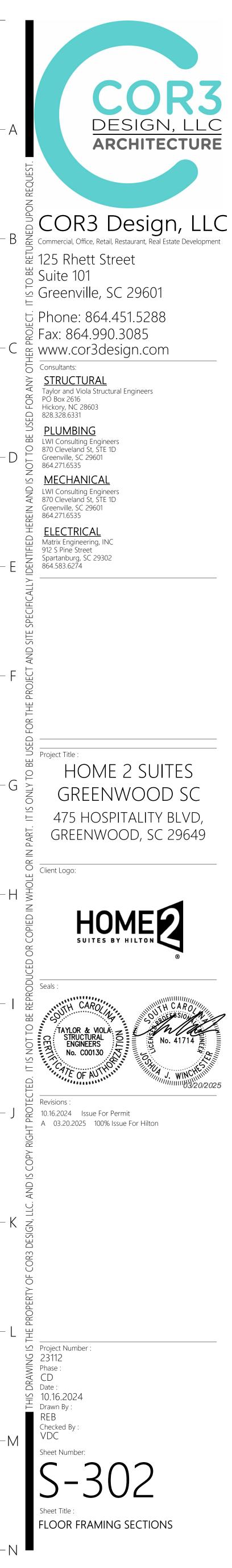






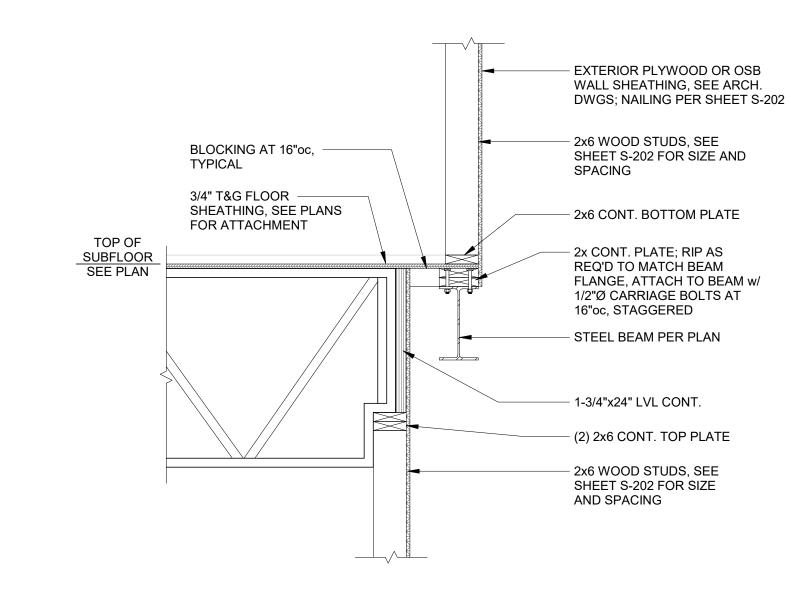




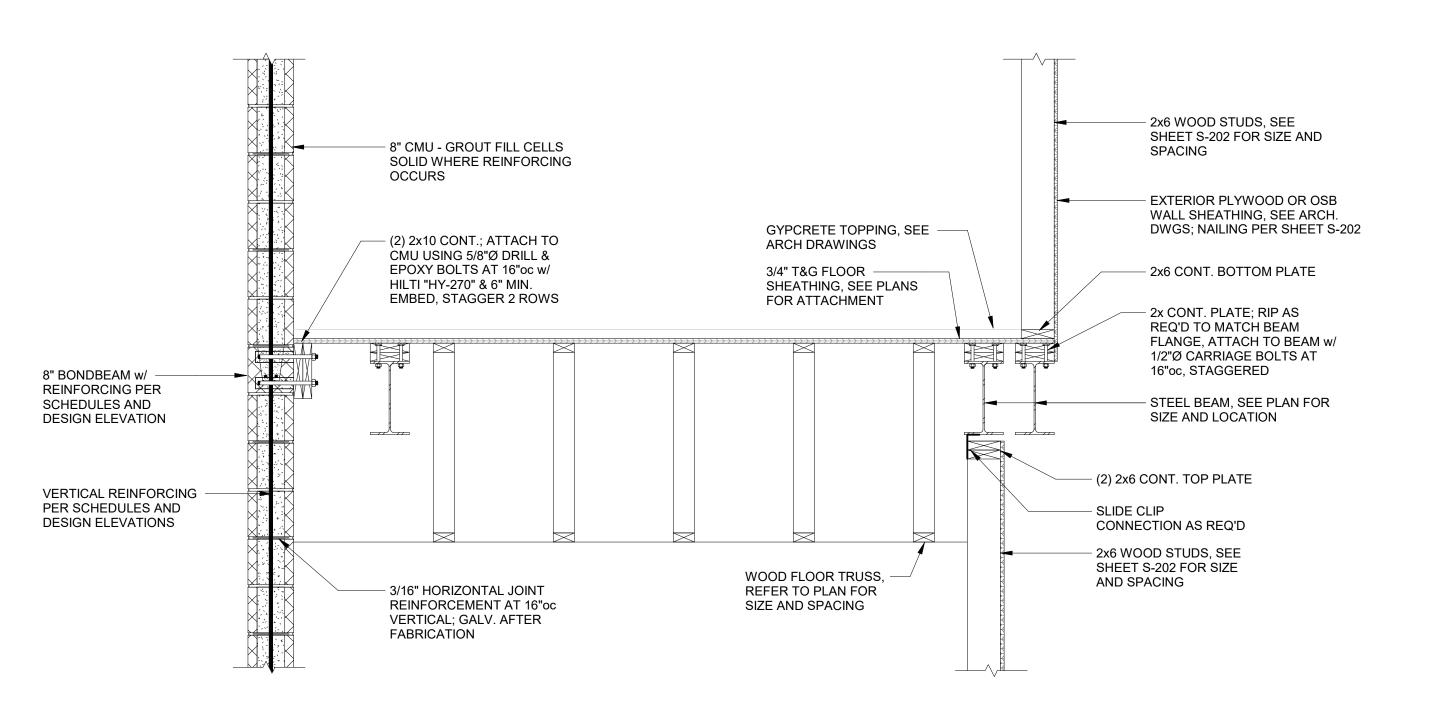


-	1	2	3	4	5	6
Α-						
B-						
C-						
D-						FRAME IS DESIGNED
						OPEN AIR STRUCTUR SLATS TO REMAIN UNCOVERED HSS8X4X3/8" BEAM BETWEEN COLUMNS 6x6x1/4" CONT. ANGL BETWEEN COLUMNS NOTCH 2x FOR LIGH AS REQ'D, SEE ARCH
E-						1/4"x2"x2" WELDED T. w/ 1/2"Ø THRU-BOLT, SEE ARCH. F.R.T. 2x4 SLATS AT & TOENAIL OR PROVID
F-						'A34' CLIPS AT BASE REQ'D BY ARCH.
G-						HSS8X6X3/8" COLUMN; MAX. SPACING BETWEI COLUMNS SHALL NOT EXCEED 13'-0" CL TO C 6x6x1/4" CONT. ANGLE BETWEEN COLUMNS
H-						EXTERIOR SLAB/PATIO BY OTHERS
] -						5 <u>TYPICAI</u> 3/4" = 1'-0"
J —						•
K-						THE STERIOR
L-						EXTERIOR SEE ARCH. FOR DETAI
M-						4) #5 CONT. E
						4 <u>TRELLIS</u> 3/4" = 1'-0"
N-	1	2	3	4	5	6

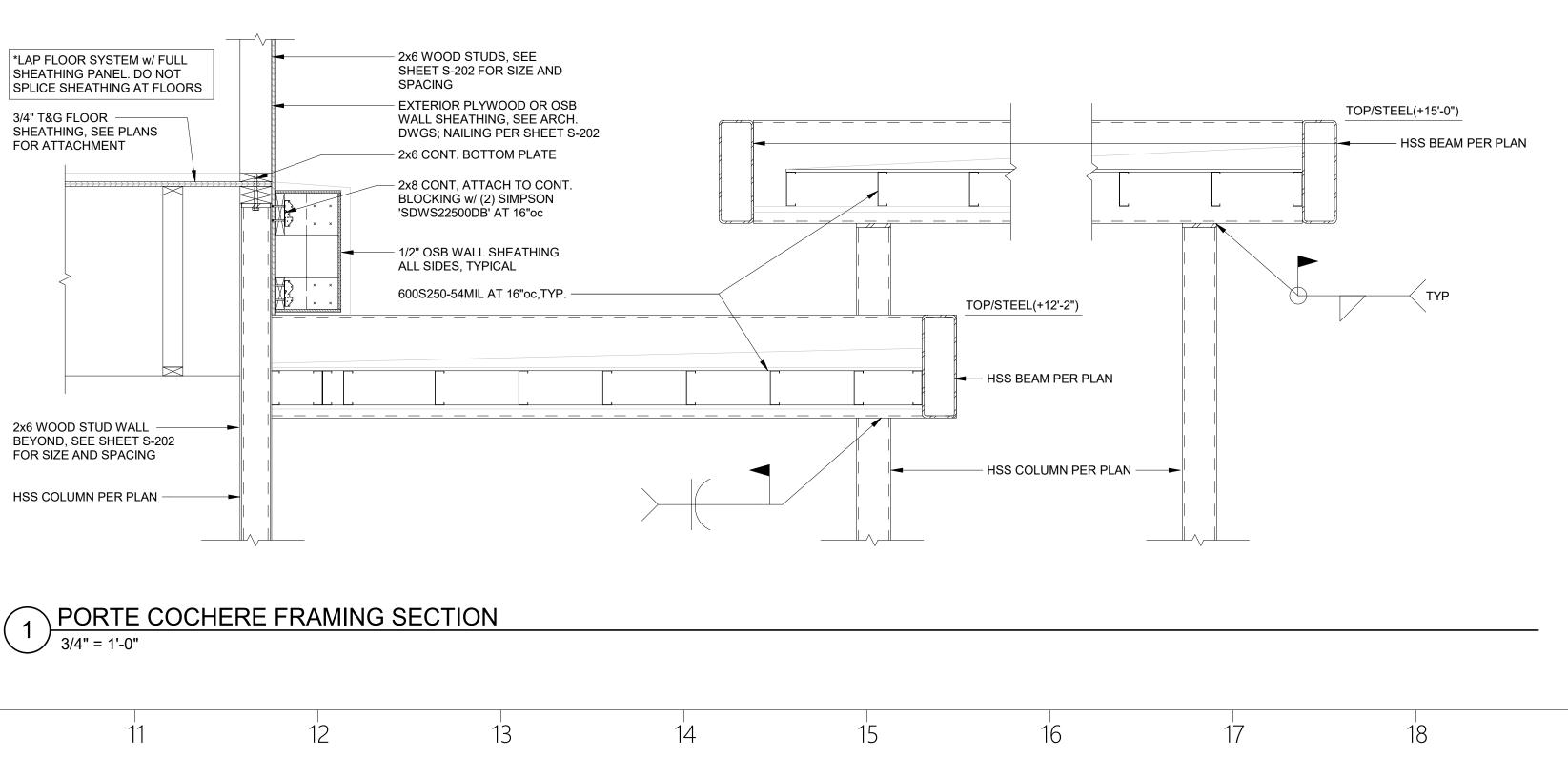


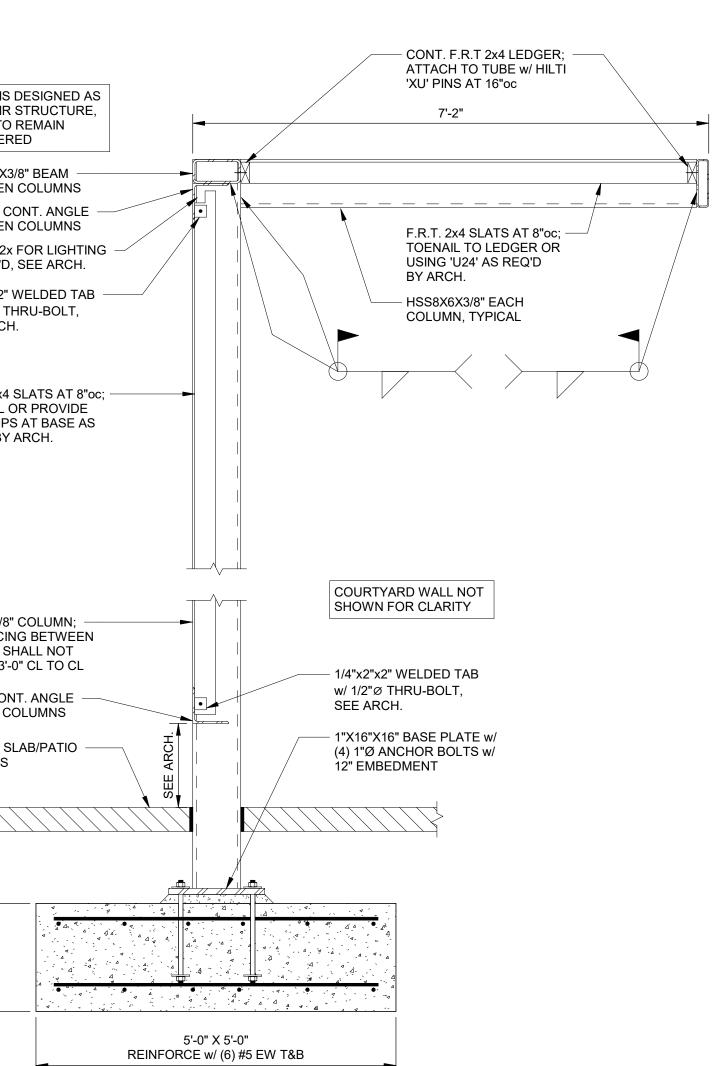




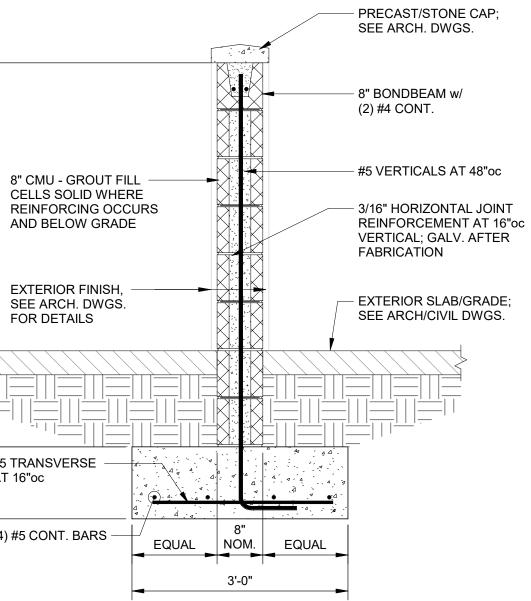








YPICAL TRELLIS FRAMING SECTION /4" = 1'-0"



RELLIS COURTYARD SITE WALL SECTION /4" = 1'-0"

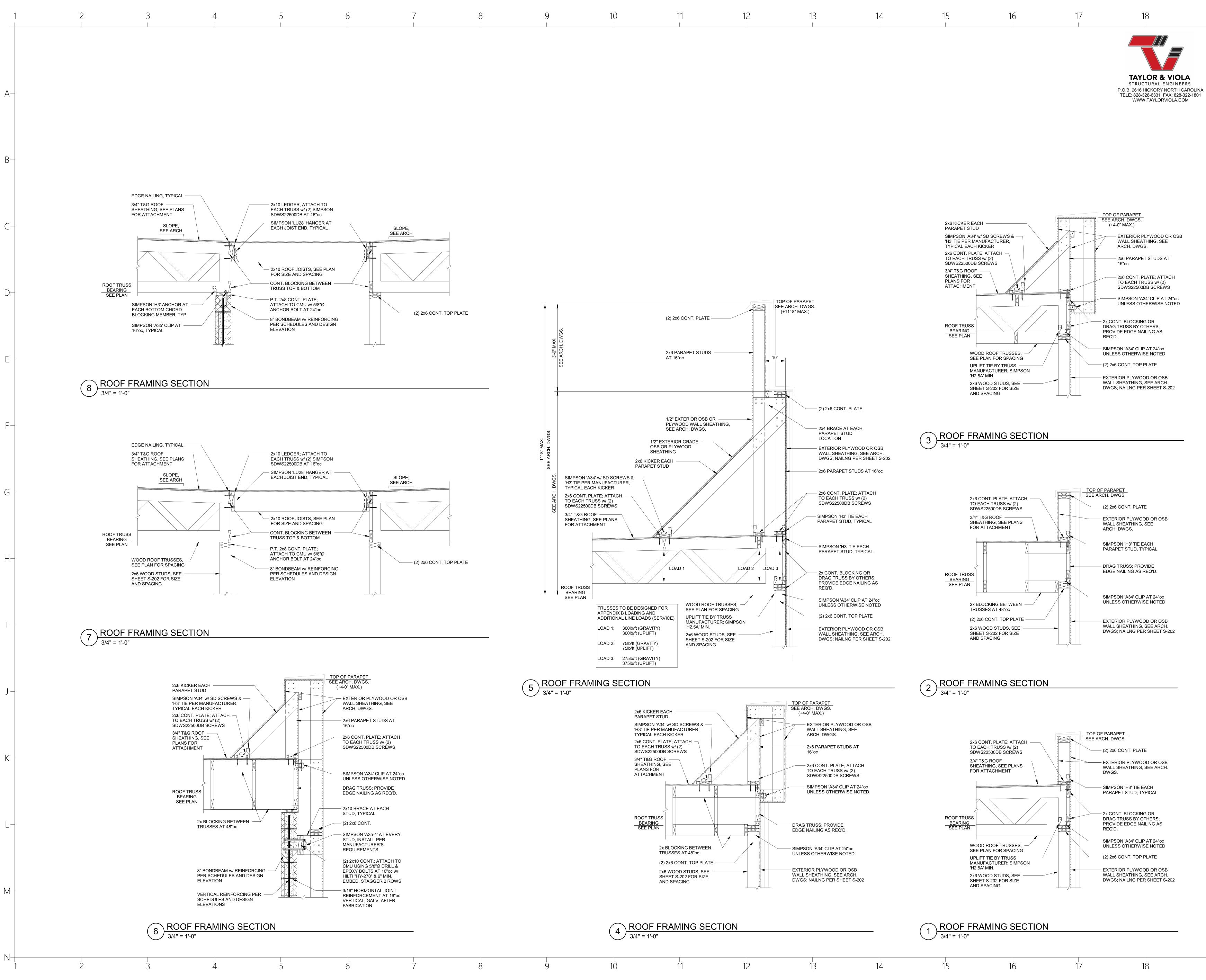
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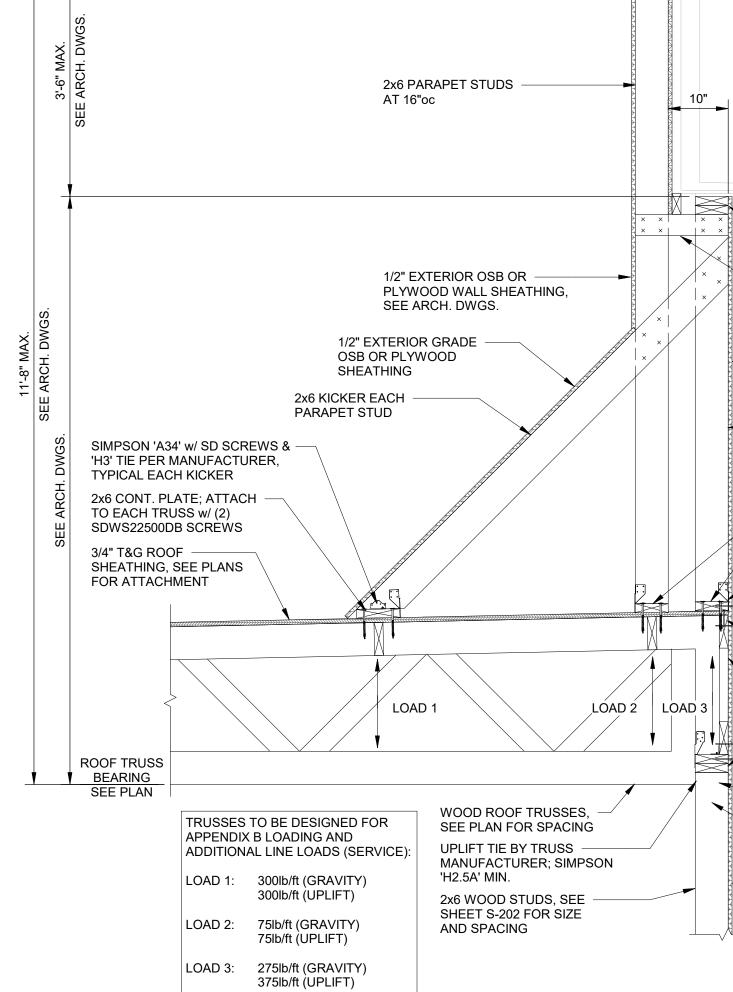
13	14	15	16	17	18	

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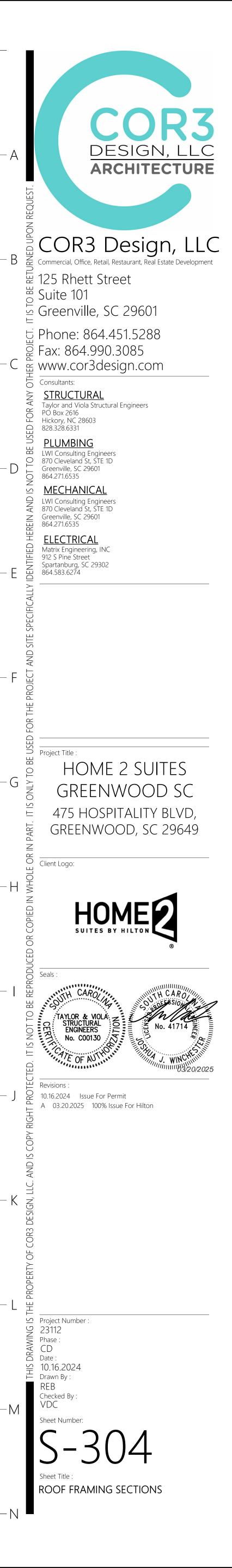


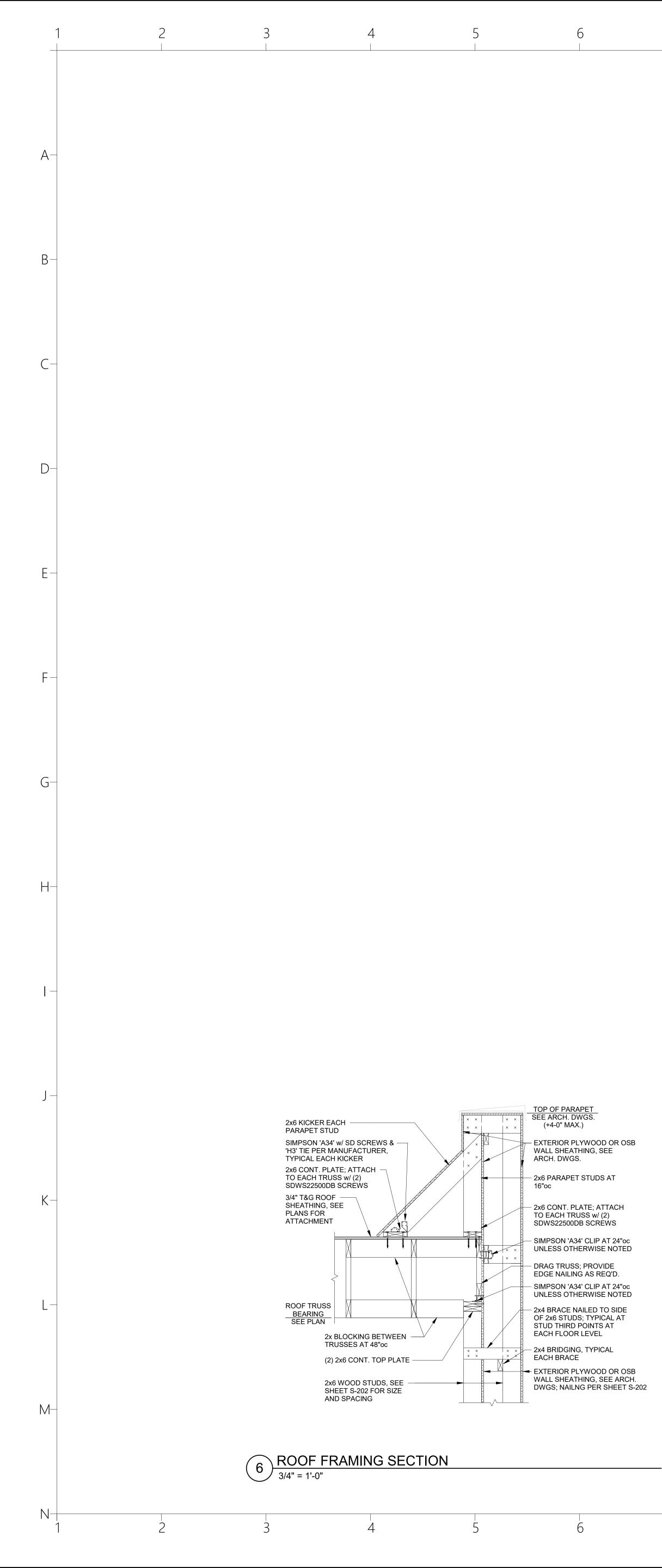


7	8	9	10	11	12



-	7 8	3	9 1	10 1	1 12	2





8	9	10	11	12

