

Sat Jun 22, 2026 - 12:05pm - K:\DRAW-2626038 - Bennett's Creek, Suffolk VA\26038_S0.1-S0.2.dwg - Layout: S0.1

STRUCTURAL GENERAL NOTES

CODE: 2021 USBC (VIRGINIA UNIFORM STATEWIDE BUILDING CODE)
BASE CODES: 2021 IBC & ASCE 7-22
RISK CATEGORY = III

LIVE LOADS

FLOOR ----- 100 PSF

ROOF SNOW LOAD

GROUND SNOW LOAD (Pg) = 46 PSF
FLAT ROOF SNOW LOAD (Pf) = 35 PSF + DRIFT
SLOPED ROOF SNOW LOAD (Ps) = 28 PSF
SNOW EXPOSURE FACTOR (Ce) = 0.8
THERMAL FACTOR (Ct) = 1.1

WIND LOADS
BASIC WIND SPEED: V3S = 128 MPH ULT.
TOPOGRAPHIC FACTOR Kzt = 1.0
WIND DIRECTIONALITY FACTOR KJ = 0.85
WIND EXPOSURE CATEGORY = D
INTERNAL PRESSURE COEFFICIENT (GCpi) = ±0.18

EARTHQUAKE DESIGN DATA

SEISMIC IMPORTANCE FACTOR (Ie) = 1.25
MAPPED SPECTRAL RESPONSE ACCELERATIONS Ss = 0.12 S1 = 0.042
SITE CLASS = E (REPORT)
SPECTRAL RESPONSE COEFFICIENTS Sds = 0.099 Sd1 = 0.059
SEISMIC DESIGN CATEGORY = A
BASIC SEISMIC-FORCE-RESISTING SYSTEM = LIGHT-FRAME WOOD WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE
DESIGN BASE SHEAR: V = 0.019W
SEISMIC RESPONSE COEFFICIENT(S) Cs = 0.019
RESPONSE MODIFICATION FACTOR(S) R = 6.5

SEISMIC LOADS CALCULATED IN ACCORDANCE WITH THE EQUIVALENT LATERAL FORCE PROCEDURE.

SPECIAL LOADS

INTERIOR WALLS - LATERAL LOAD - 5 PSF

GENERAL

THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE AND INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, TEMPORARY BRACING, ETC. THAT MAY BE NECESSARY. SUCH MATERIAL SHALL REMAIN THE CONTRACTOR'S PROPERTY AFTER COMPLETION OF THE PROJECT.

WORK THESE DRAWINGS WITH ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS.

BUILDING PAD PREPARATION

ALL TREES, BRUSH, ROOTS, TOPSOIL, RUBBLE, ORGANICALLY CONTAMINATED OR OTHERWISE OBJECTIONABLE MATERIALS ENCOUNTERED ARE TO BE REMOVED FROM STRUCTURAL AREAS OF THE SITE.

SUBGRADE SECTORS WHICH WILL EXIST IN CUT AND THOSE WHICH ARE TO SUPPORT FILL STRUCTURES ARE TO BE PROOF ROLLED. AREAS EXHIBITING INSTABILITY ARE TO BE UNDERCUT AND BACKFILLED ON A LIFT-BY-LIFT BASIS WITH EACH LIFT CAREFULLY COMPACTED.

IF UNSTABLE SUBGRADE SECTORS CANNOT BE STABILIZED BY EXCAVATION AND RECOMPACTION, THEN CRUSHED STONE OR SIMILAR COARSE AGGREGATE MATERIALS SHALL BE ROLLED INTO THE SUBGRADE UNTIL A FIRM SUBGRADE REACTION IS ACHIEVED.

THE PROPOSED ENGINEERED FILL MATERIALS ARE TO BE PLACED IN LIFTS NOT EXCEEDING EIGHT (8) INCHES IN LOOSE MEASURED THICKNESS. EACH LIFT IS TO BE COMPACTED AS FOLLOWS:

SLAB ON GRADE: MINIMUM OF 95% MAXIMUM DENSITY BY ASTM D-698.

FOOTINGS BEARING ON FILL: A MINIMUM OF 98% MAXIMUM DENSITY BY ASTM D-698.

THE EARTHWORK PROGRAM SHOULD BE CONDUCTED UNDER THE SUPERVISION OF A SOILS LABORATORY. THE IN-PLACE DENSITIES ACHIEVED ARE TO BE VERIFIED BY TESTS.

FOUNDATIONS

THE CONTRACTOR SHALL FAMILIARIZE HIMSELF WITH THE SURVEY AND THE SUB-SURFACE INVESTIGATION REPORT BEFORE STARTING CONSTRUCTION. ALL FOUNDATION WORK SHALL BE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE SOILS REPORT BY STANTEC CONSULTING SERVICES INC., DATED JULY 30, 2025 EXCEPT WHERE NOTED OTHERWISE ON DRAWINGS OR SPECIFICATIONS.

A SOILS TESTING LABORATORY SHALL BE RETAINED BY THE CONTRACTOR TO PROVIDE CONSTRUCTION REVIEW TO INSURE CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS DURING THE EXCAVATION, BACKFILL, AND FOUNDATION PHASES OF THE PROJECT.

THE SOILS TESTING LABORATORY SHALL: DISCUSS WITH THE ENGINEER THE DESIGN INTENT OF THE CONSTRUCTION DOCUMENTS AND THE TESTING PROCEDURES USED TO INSURE CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS BEFORE CONSTRUCTION BEGINS. INFORM THE ENGINEER OF ANY VARIANCE IN THESE PROCEDURES.

IT SHALL BE THE RESPONSIBILITY OF THE SOILS TESTING LABORATORY TO DETERMINE TOPSOIL AND EXCAVATION STRIPPING DEPTH; INSPECT ALL SUBSOIL EXPOSED DURING STRIPPING, SITE GRADING, AND EXCAVATION OPERATIONS; APPROVE FILL MATERIALS, PERFORM DENSITY TESTS OF FILLS TO INSURE PLACEMENT PER SPECIFICATION REQUIREMENTS; INSPECT FOUNDATION BEARING SURFACES.

FOUNDATION DESIGN IS BASED ON ASSUMED BEARING PRESSURE OF 1500 PSF BEARING PRESSURE ON SOIL CONDITIONS DEEMED ACCEPTABLE BY THE GEOTECHNICAL REPORT. MINIMUM FROST DEPTH SHALL BE 18 INCHES.

STEP FOOTINGS, WHERE REQUIRED, AT A RATIO OF ONE (1) VERTICAL TO TWO (2) HORIZONTAL WITH A MAXIMUM VERTICAL STEP OF 2'-0" UNLESS NOTED OTHERWISE.

INUNDATION AND LONG TERM EXPOSURE OF BEARING SURFACES, WHICH WILL RESULT IN DETERIORATION OF BEARING FORMATIONS, SHALL BE PREVENTED. FOOTINGS SHALL BE PLACED IMMEDIATELY FOLLOWING FOOTING EXCAVATIONS AND BEARING SURFACE INSPECTION.

ALL FILL MATERIALS SHALL BE FREE OF ORGANIC CONTAMINATIONS AND OTHER DELETERIOUS MATTER.

FOR BACKFILL AGAINST FOOTINGS, ETC., PLACE IN 8" THICK LAYERS, WITH EACH LIFT COMPACTED AT NEAR OPTIMUM MOISTURE CONTENT, UNTIL A MINIMUM IN PLACE DENSITY OF 95% OF THE MAXIMUM DENSITY AS DETERMINED BY ASTM TEST PROCEDURE D-1557 IS ACHIEVED.

NOTIFY STRUCTURAL ENGINEER OF ANY UNUSUAL SOIL CONDITIONS THAT ARE IN VARIANCE WITH THE SOILS REPORT.

CONCRETE

ALL CONCRETE SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH IN 28 DAYS
SHALL BE: 3000 PSI FOR SLAB ON GRADE
4000 PSI FOR ALL OTHER CONCRETE

REINFORCING BARS SHALL BE NEW BILLET STEEL BARS CONFORMING TO ASTM A-615. GRADE 60 (60,000 PSI YIELD).

NO TACK WELDING OF REINFORCING IN THE FIELD WILL BE PERMITTED.

WELDED WIRE MESH REINFORCING SHALL CONFORM TO ASTM A-185.

ALL CONCRETE CONSTRUCTION SHALL CONFORM TO ACI 301-10, "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" UNLESS NOTED OTHERWISE.

ALL DETAILING, FABRICATION AND PLACING OF REINFORCING BARS, UNLESS OTHERWISE NOTED, SHALL CONFORM TO ACI 318-08, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" AND THE LATEST ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES."

PROVIDE DOVETAIL ANCHORS AT 2'-0" O.C. FOR ALL MASONRY FACED WALLS AND PROVIDE AT COLUMNS WHERE WALLS ABUT COLUMNS.

CONCRETE MINIMUM LAP SPLICE, LAP REINF BARS: #5 BARS 36"; #6 BARS 43"

DO NOT BEGIN CONCRETE CONSTRUCTION UNTIL ALL CONCRETE MIX DESIGNS HAVE BEEN REVIEWED AND APPROVED BY THE ENGINEER OF RECORD. ALL CEMENT SHALL BE TYPE I OR II. A MAXIMUM OF 25% FLY ASH MAY BE USED, UNO. MAXIMUM W/C RATIOS BY WEIGHT FOR NORMAL CONCRETE SHALL BE:

0.45 FOR AIR-ENTRAINED CONCRETE
0.50 FOR SLAB-ON GRADE AND ALL OTHER CONCRETE

SLAG IS NOT PERMITTED IN ANY CONCRETE MIX DESIGNS.

MAXIMUM SLUMP WITH HIGH RANGE WATER REDUCING AGENT SHALL BE 8". MAXIMUM SLUMPS WITHOUT ADMIXTURES SHALL BE 5" FOR COLUMNS, PIERS, AND WALLS. ALL OTHER CONCRETE SLUMPS WITHOUT ADMIXTURES SHALL BE 4".

ALL AGGREGATES SHALL COMPLY WITH ASTM C33. MAXIMUM AGGREGATE SIZE SHALL BE 1" (#57) UNLESS NOTED OTHERWISE.

THE OWNER SHALL EMPLOY A TESTING LAB TO MOLD AND TEST CONCRETE CYLINDERS, PERFORM SLUMP TESTS, AND TO PERFORM TESTS FOR AIR CONTENT. A THREE CYLINDER MINIMUM FOR EACH 50 CUBIC YARDS FOR EACH TYPE OF CONCRETE MIX SHALL BE PERFORMED EACH DAY. STRENGTH TESTS SHALL BE IN ACCORDANCE WITH ASTM C39.

SLUMP EXCEEDING THE SPECIFIED MAXIMUM, WHEN OCCURRING IN TWO CONSECUTIVE TESTS MADE ON DIFFERENT PORTIONS OF THE SAME SAMPLE, WILL BE CAUSE FOR REJECTION OF THAT TRUCKLOAD AND SHALL BE REPORTED TO THE OWNER AND CONTRACTOR IMMEDIATELY. THE REPLACEMENT OF SUCH CONCRETE WITH THE SPECIFIED CONCRETE SHALL BE PROVIDED AT NO ADDITIONAL EXPENSE TO THE OWNER.

SLAB ON GRADE REINFORCING SHALL BE "FIBERSTRAND 100" BY EUCLID CHEMICAL CO. OR EQUAL. DOSAGE RATE AT 1-1/2 POUNDS PER CUBIC YARD, MINIMUM.

MASONRY

ALL MASONRY MATERIALS AND CONSTRUCTION SHALL COMPLY WITH THE RECOMMENDATIONS OF "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530-08)" AND "SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530.1-08)," AND MINIMUM REQUIREMENTS ESTABLISHED BY LOCAL BUILDING CODES.

CONCRETE MASONRY UNITS SHALL BE ASTM C90, TYPE I WITH A MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI (AVG. NET AREA) AND f'm = 1500 PSI.

TYPE "S" MORTAR WITH A MINIMUM COMPRESSIVE STRENGTH OF 1,800 PSI SHALL BE USED FOR ALL REINFORCED MASONRY WALLS IN ACCORDANCE WITH ASTM C270.

GROUT TO FILL CORES SHALL BE ASTM C-476, COARSE GROUT (3/8" MAXIMUM AGGREGATE) WITH A MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI IN 28 DAYS.

REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60.

ALL MASONRY UNITS SHALL HAVE GALVANIZED HORIZONTAL REINFORCEMENT AS FOLLOWS:

LADDER TYPE, 3/16" SIDE RODS AND #9 GAGE CROSS RODS SPACED 16" O.C. VERTICALLY.

LAP ALL SPLICES 48 BAR DIAMETERS UNLESS SHOWN OTHERWISE ON DRAWINGS.

VERTICAL BARS SHALL BE HELD IN POSITION AT 48" MAX (UNO) WITH VERTICAL BAR POSITIONERS AT TOP OF BOTTOM COURSE AND BOTTOM OF TOP COURSE AND AT INTERVALS NOT EXCEEDING 200 DIAMETERS OF THE REINFORCING, NOR 10 FEET. BARS SHALL BE IN SECURED PLACE PRIOR TO GROUTING.

VERTICAL REINFORCING BARS SHALL HAVE A MINIMUM CLEARANCE OF 3/4 OF AN INCH FROM THE MASONRY AND NOT LESS THAN ONE BAR DIAMETER BETWEEN BARS.

ALL CORES WITH REINFORCEMENT SHALL BE FILLED SOLID WITH GROUT. ALL GROUT SHALL BE CONSOLIDATED IN PLACE BY VIBRATION TO INSURE COMPLETE FILLING OF CELLS.

MORTAR PROTRUSIONS, EXTENDING INTO CELLS OR CAVITIES TO BE REINFORCED AND FILLED, SHALL BE REMOVED.

LAY MASONRY UNITS WITH FULL MORTAR COVERAGE ON HORIZONTAL AND VERTICAL FACE SHELLS. BED WEBS IN MORTAR IN STARTING COURSE ON FOOTINGS AND IN ALL COURSES OF COLUMNS AND PILASTERS, AND WHERE ADJACENT TO CELLS OR CAVITIES TO BE REINFORCED OR FILLED WITH CONCRETE OR GROUT.

GROUT ONE (1) COURSE OF MASONRY SOLID UNDER ALL WALL BEARING SLABS.

PROVIDE 16" OF SOLID MASONRY UNDER WALL BEARING BEAMS UNLESS NOTED OTHERWISE.

PROVIDE 8" OF SOLID MASONRY 24" WIDE MINIMUM UNDER WALL BEARING JOISTS.

ALL CORNERS SHALL BE TIED BY MASONRY BOND. PROVIDE 3-#5 VERT BARS FULLY GROUTED, ONE IN THE CORNER CELL AND ONE IN EA ADJACENT CELL.

GROUT CORES SOLID A MINIMUM OF ONE COURSE BELOW ANY CHANGE IN WALL THICKNESS.

THE COLLAR JOINT IN MULTI-WYTHE WALLS SHALL BE FULLY GROUTED AS THE WALL IS CONSTRUCTED.

ALL MASONRY WALLS SHALL HAVE VERTICAL CONTROL JOINTS AT APPROXIMATELY 25'-0" O.C.; COORDINATE EXACT LOCATIONS WITH ARCHITECTURAL DRAWINGS.

MASONRY MINIMUM LAP SPLICE, LAP REINF BARS: #5 BARS 45"; #6 BARS 54"

STRUCTURAL STEEL

STRUCTURAL STEEL SHAPES SHALL CONFORM TO ASTM A-992 GRADE 50 (Fy = 50 KSI). SQUARE AND RECTANGULAR HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO ASTM A-500 GRADE B (Fy = 46 KSI). ROUND HOLLOW STRUCTURAL SECTIONS SHALL CONFORM TO ASTM A500 GRADE B (Fy = 42 KSI). STEEL PIPE SHALL CONFORM TO ASTM A-53 GRADE B (Fy = 35 KSI MIN). ANGLES, CHANNELS, PLATES, AND BARS SHALL CONFORM TO ASTM A-36 (Fy = 36 KSI) AS A MINIMUM UNLESS NOTED OTHERWISE ON THE DRAWINGS.

DETAILING, FABRICATION, AND ERECTION SHALL CONFORM TO THE LATEST AISC SPECIFICATIONS.

ALL STRUCTURAL STEEL SHALL BE SHOP PRIMED.

FIELD CONNECTIONS SHALL BE BOLTED, BEARING TYPE UNLESS NOTED OTHERWISE, USING 3/4" HIGH STRENGTH BOLTS CONFORMING TO ASTM A-325. ONE SIDED CONNECTIONS ARE NOT PERMITTED UNLESS DETAILED ON DRAWINGS.

ALL CONNECTIONS TO TUBES AND PIPES SHALL USE THRU PLATES UNLESS NOTED OTHERWISE.

ALL WELDING SHALL BE DONE USING E-70XX ELECTRODES IN ACCORDANCE WITH THE LATEST AWS SPECIFICATIONS.

WORK THESE DRAWINGS WITH ARCHITECTURAL DRAWINGS FOR NAILER HOLES AND ARCHITECTURAL CLEARANCES.

GENERAL CONTRACTOR SHALL VERIFY ALL STRUCTURAL BEAM LOCATIONS, MECHANICAL UNIT WEIGHTS AND OPENING SIZES AND LOCATIONS WITH MECHANICAL CONTRACTOR AND VENDOR'S DRAWINGS FOR ACTUAL MECHANICAL UNIT PURCHASED.

ALL 4 x 4 x 5/16 ANGLES REQUIRED FOR ROOF UNITS AND ROOF OPENINGS OVER 12" x 12" TO BE SUPPLIED BY STRUCTURAL STEEL FABRICATOR AND BE COORDINATED BY GENERAL CONTRACTOR WITH THE JOIST FABRICATOR, MECHANICAL DRAWINGS AND MECHANICAL EQUIPMENT SUPPLIER.

ALL STRUCTURAL STEEL BEAMS AND COLUMNS ADJACENT TO MASONRY SHALL HAVE ADJUSTABLE MASONRY ANCHORS AT 2'-8" C/C.

NON-METALLIC, NON-SHRINK, NON-STAINING GROUT UNDER ALL COLUMN BASEPLATES AND BEAM BEARING PLATES SHALL CONSIST OF A PREMIXED PRODUCT COMPLYING WITH ALL REQUIREMENTS OF CRD-C621, ASTM C827, AND C109.

MISCELLANEOUS STEEL LINTEL SCHEDULE

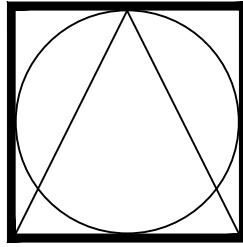
FOR WALLS 12" OR THICKER

FOR OPENINGS UP TO 4'-0" USE 3-1/2 x 3-1/2 x 5/16 ANGLE.
FOR OPENINGS FROM 4'-0" TO 5'-0" USE 4 x 3-1/2 x 5/16 LLV.
FOR OPENINGS FROM 5'-0" TO 6'-0" USE 5 x 3-1/2 x 5/16 LLV.
FOR OPENINGS FROM 6'-0" TO 7'-0" USE 6 x 3-1/2 x 5/16 LLV.
USE ONE ANGLE FOR EACH 4" WYTHE OF MASONRY.

ALL LINTELS SHALL HAVE A BEARING AT EACH END OF 1 INCH PER FOOT OF OPENING WITH A MINIMUM OF 6".

ALL LINTELS SHALL BEAR ON 16" SOLID MASONRY EXTENDING 16" BEYOND END OF LINTEL.

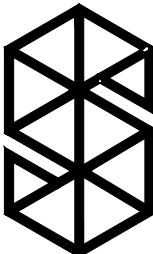
ALL EXTERIOR LINTELS AND EXPOSED EXTERIOR BOTTOM LINTEL PLATES SHALL BE GALVANIZED; LINTELS SHALL BE GALVANIZED AFTER FABRICATION.



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06/19/26

BENNETT'S CREEK
HEALTH & REHABILITATION CENTER
1931/1915 BRIDGE ROAD (HIGHWAY 11)
SUFFOLK, VIRGINIA



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DRAWING NAME
GENERAL
NOTES

DSCA PROJECT NO.
24.061

S0.1

NOTE:
SCALES INDICATED ARE BASED ON FULL SIZE 24"x36" DRAWING. DIMENSIONS SHALL NOT BE DETERMINED BY COMPUTER SCALING OF DRAWINGS.

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STRUCTURAL GENERAL NOTES CONT.

WOOD ROOF TRUSS DESIGN CRITERIA

DESIGN SUPERIMPOSED LOADING:

TOP CHORD:	
LIVE LOAD (SNOW):	SEE DESIGN LOADS @ BEGINNING OF GENERAL NOTES
	UNBALANCED SNOW LOADS SHALL BE TAKEN INTO
	ACCOUNT. REDUCTION FOR SLOPED ROOF SHALL BE
	PERMITTED AT GABLED ROOF AREAS ONLY.
DEAD LOAD:	10 PSF
BOTTOM CHORD:	
LIVE LOAD:	20 PSF @ ATTIC AREAS (SEE ARCH FOR LOCATIONS)
DEAD LOAD:	10 PSF
WIND UPLIFT:	15 PSF OR ROOF NET AREA

TRUSSES SHALL BE DESIGNED TO CARRY ALL DRIFTING SNOW LOADS, AND ROOFTOP MECHANICAL EQUIPMENT LOADS AND ATTIC-LOCATED HVAC UNITS AS SHOWN ON THE MECHANICAL AND ARCHITECTURAL DRAWINGS.

THE TRUSS MANUFACTURER SHALL ENSURE CONSIDERATION TO THE ADDITIONAL LOADS REQUIRED BY ALL CEILING HUNG SOFFITS AND SOFFITS WITH CABINETS AS SHOWN ON THE ARCHITECTURAL DRAWINGS.

ALLOWABLE STRESS DESIGN USED IN THE DESIGN OF THE ROOF TRUSSES SHALL BE BASED ON THE LUMBER CONNECTIONS USED WITH A MAXIMUM 15% INCREASE FOR SNOW AND A MAXIMUM INCREASE OF 33% FOR WIND.

TRUSS DESIGN DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER OF RECORD FOR REVIEW PRIOR TO FABRICATION.

EACH TRUSS SHALL BE FASTENED TO THE TOP WALL PLATE WITH STANDARD SEISMIC & HURRICANE TIES ("SIMPSON" OR EQUAL) EACH END WITH THE MINIMUM NAILING REQUIRED BY THE TIE MANUFACTURER TO DEVELOP FULL CAPACITY.

BRACE WOOD TRUSSES AS PER THE TRUSS PLATE INSTITUTE PUBLICATION "BRACING WOOD TRUSSES: COMMENTARY AND RECOMMENDATIONS" BWT-76.

ALL WOOD TRUSS CHORDS SHALL BE:
#1 DOUGLAS FIR, LARCH OR #1 SOUTHERN PINE OR
#2 DOUGLAS FIR #2 MG. SOUTHERN PINE

ALL WOOD TRUSS WEBS SHALL BE:
CONSTRUCTION DOUGLAS FIR, LARCH OR SOUTHERN PINE

PROVIDE CAMBER FOR DEAD LOAD DEFLECTION

THE CONTRACTOR SHALL FURNISH ALL TEMPORARY BRACING AS NEEDED DURING THE INSTALLATION OF THE TRUSSES AND IT SHALL BE LEFT IN PLACE DURING CONSTRUCTION AS REQUIRED. THE CONTRACTOR IS ALSO RESPONSIBLE FOR THE INSTALLATION OF ANY AN ALL BRACING SHOWN ON THE TRUSS MANUFACTURER SHOP DRAWINGS. ALL CONTINUOUS LATERAL BRACING TO BE PERMANENTLY FASTENED TO THE END WALLS.

GIRDER TRUSSES SHALL HAVE FULL BEARING ON WOOD COLUMNS WHERE SHOWN.

THE ROOF PLAN PROPOSED TRUSS LAYOUTS DO NOT SERVE AS A TRUSS DESIGN. ALL TRUSSES SHALL BE DESIGNED AND CERTIFIED BY THE TRUSS MANUFACTURER. THE TRUSS MANUFACTURER SHALL VERIFY ALL SPANS, SLOPES, BEARING POINTS, AND DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS BEFORE FABRICATION. THE TRUSS SPACING SHALL BE NO GREATER THAN 24" C/C. THE TRUSS MANUFACTURER SHALL PROVIDE ALL GABLE END TRUSSES WITH INTERMEDIATE STUDS MEMBERS AT 16" C/C.

ALL OPENINGS OVER 8'-0" WIDE TO HAVE HEADERS AND LINTELS DESIGNED BY THE TRUSS MANUFACTURER (UNO).

WOOD CONSTRUCTION

DETAIL, FABRICATE, AND ERECT STRUCTURAL WOOD IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION STANDARDS AND SPECIFICATIONS AND THESE DRAWINGS.

ALL LUMBER GRADES SHALL BE DOUGLAS FIR-LARCH (DF), UNLESS NOTED OTHERWISE: BEAMS, HEADERS, PURLINS, 4x JOISTS DF NO. 1
Fb = 1,200 PSI MINIMUM; E = 1,800,000 PSI MINIMUM

FRAMING HANGERS, HANGER FASTENERS, ANCHORS, CONNECTIONS ETC., SHALL BE AS MANUFACTURED BY "SIMPSON-STRONG TIE" OR APPROVED EQUAL AND APPROVED BY THE BUILDING DEPARTMENT.

PLYWOOD TO BE STAMPED BY THE DOUGLAS FIR PLYWOOD ASSOCIATION AND SHALL CONFORM TO U.S. PRODUCT STANDARDS PS 1, WITH EXTERIOR GLUE. PLYWOOD AT EXPOSED EAVES SHALL BE EXTERIOR GRADE PLYWOOD. ALL WOOD SHEATHING SHALL BE PLACED WITH 1/8" GAP BETWEEN ADJACENT SHEETS.

NAILING AT EDGE OF ALL OPENINGS TO BE THE SAME AS BOUNDARY NAILING. ALL NAILS SHALL BE COMMON NAILS EXCEPT WHERE OTHERWISE STATED. NAIL SPACING IS TO BE AS SHOWN ON THE DRAWINGS.

ALL BOLT HEADS AND NUTS THAT BEAR AGAINST THE FACE OF WOOD MEMBERS SHALL BE PROVIDED WITH METAL WASHERS. ALL BOLT HOLES SHALL BE BORED 1/32" TO 1/16" LARGER THAN BOLT DIAMETER.

ALL STUD WALLS SHALL HAVE FIRE BLOCKING AT 8'-0" O.C. MAXIMUM.

WHERE WOOD IS IN CONTACT WITH CONCRETE FOR MASONRY, USE FOUNDATION REDWOOD, PRESSURE TREATED DOUGLAS FIR OR SHEET METAL CONNECTORS.

PROVIDE MULTIPLE STUDS FOR SOLID BEARING AT ENDS OF MISCELLANEOUS BEAMS WHERE POSTS ARE NOT SHOWN, CARRY STUDS DOWN TO SILL PLATE. AT A MINIMUM INTERIOR WALL STUD COLUMNS SHALL BE (2)2X4 STUDS GLUED AND NAILED. AT A MINIMUM EXTERIOR WALL STUD COLUMNS SHALL BE (2)2X6 STUDS GLUED AND NAILED.

HOLES IN STRUCTURAL MEMBERS FOR PIPES AND CONDUIT SHALL COMPLY WITH THE BUILDING CODE AND SHALL BE VERIFIED BY THE ENGINEER.

ALL JOISTS, RAFTERS AND HEADERS SHALL HAVE A MINIMUM OF 3 1/2" BEARING OVER SUPPORTS, UNLESS NOTED OTHERWISE.

NO STRUCTURAL MEMBER SHALL BE CUT OR NOTCHED UNLESS SPECIFICALLY SHOWN OR APPROVED BY THE ENGINEER OF RECORD.

ALL SILL PLATES IN CONTACT WITH CONCRETE OR MASONRY SHALL BE TREATED WOOD AND SHALL CONFORM TO AWPA C2.

ALL STUDDING SHALL BE CONTINUOUS BETWEEN POINTS OF LATERAL SUPPORT.

THE MAXIMUM MOISTURE CONTENT SHALL NOT EXCEED 19% (UNO)

SPACE ALL NAILS 3/8" MINIMUM FROM PLYWOOD EDGES OR SUPPORTING MEMBER EDGES.

WHEN LAMINATING VERTICALLY 2-2X MEMBERS TOGETHER, USE 16d NAILS AT 9" O.C.

SEE O.B.C TABLE 2304.10.1 FOR CODE REQUIRED WOOD FASTENING SCHEDULE. USE CODE SCHEDULE AS A MINIMUM REQUIREMENT UNLESS STRICTER REQUIREMENTS ARE STATED ELSEWHERE ON THESE PLANS.

SHEARWALL SHEATHING(ALL EXTERIOR WALLS, PLUS INTERIOR LOCATIONS ON PLAN: APPLY TO BOTH SIDES OF 2x6 WALLS, 7/16" ORIENTED STRAND BOARD (OSB) STRUCTURAL 1 GRADE SHEATHING VERTICALLY AND NAIL @ 4"C/C MINIMUM WITH 8d COMMON CORROSION RESISTANT NAILS OR EQUAL, AND AT 12"C/C MAXIMUM TO SUPPORTS AT INTERMEDIATE SUPPORTS WITH 8d ANNULAR OR SPIRAL THREAD NAILS. INSTALLATION OF ALL SHEATHING SHALL BE INSPECTED PRIOR TO COVERING.

ROOF SHEATHING:
5/8" MINIMUM (UNO)EXTERIOR APA ORIENTED STRAND BOARD (OSB)THAT SHALL BE STRUCTURAL I GRADE WITH EXTERIOR GLUE. ROOF SHEATHING INSTALLED OVER TWO OR MORE SPANS WITH WITH STAGGERED JOINTS AND FACE GRAIN PERPENDICULAR TO TRUSSES UNLESS NOTED OTHERWISE. SUITABLE EDGE SUPPORT SHALL BE PROVIDED BY THE USE OF PLYCLIPS AT MIDSPAN. NAIL TO SUPPORTS AT 4" C/C ALONG PANEL EDGES AND 8" C/C AT INTERMEDIATE SUPPORTS WITH 2 1/2" 8d COMMON OR 2" 6d DEFORMED SHANK NAILS. FLAT ROOF DECK SHALL BE 3/4" MINIMUM EXTERIOR APA ORIENTED STRAND BOARD (OSB) EXTERIOR APA STRUCTURAL I GRADE, GLUED WITH THE SAME FASTENING PATTERN AS THE TYPICAL ROOF SHEATHING. INSTALLATION OF ALL SHEATHING SHALL BE INSPECTED PRIOR TO COVERING.

DEFERRED STRUCTURAL SUBMITTALS

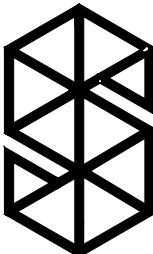
- DRAWINGS AND CALCULATIONS FOR DEFERRED SUBMITTALS SHALL BE STAMPED BY AN ENGINEER REGISTERED IN THE STATE WHERE THE STRUCTURE IS LOCATED AND SUBMITTED TO THE ENGINEER OF RECORD FOR APPROVAL PRIOR TO SUBMISSION TO THE BUILDING DEPARTMENT FOR REVIEW. DEFERRED SUBMITTALS SHALL INCLUDE THE FOLLOWING STRUCTURAL COMPONENTS:
-PREFABRICATED WOOD TRUSSES, BRACING, AND CONNECTIONS

SPECIAL INSPECTIONS

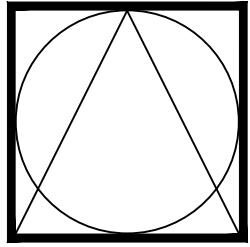
TESTING AND FIELD INSPECTIONS SHALL BE PERFORMED IN ACCORDANCE WITH THE STRUCTURAL SPECIFICATION REQUIREMENTS TO VERIFY THAT THE STRUCTURE CONFORMS TO THE DRAWINGS AND SPECIFICATIONS.

ALL SPECIAL INSPECTIONS SHALL FOLLOW THE CITY REQUIREMENTS.

NOTE:
SCALES INDICATED ARE BASED ON FULL SIZE 24"x36" DRAWING.
DIMENSIONS SHALL NOT BE DETERMINED BY COMPUTER SCALING OF DRAWINGS.



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DSC
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mail@dscarchitects.com

DATE:
PERMIT SUBMISSION:
06/19/26

BENNETT'S CREEK
HEALTH & REHABILITATION CENTER
1931/1915 BRIDGE ROAD (HIGHWAY 11)
SUFFOLK, VIRGINIA



DRAWING NAME
GENERAL NOTES

DSCA PROJECT NO.
24.061

S0.2

SOUTH WING FOUNDATION PLAN

1"=10'-0"

NOTES:

1. FLOOR CONSTRUCTION: 4" CONCRETE SLAB ON GRADE WITH POLYPROPYLENE MICRO-FIBER REINF. PROVIDE MINIMUM 10-MIL VAPOR BARRIER AND MINIMUM 4" LAYER OF GRAVEL BELOW SLAB UNLESS EXISTING SUBGRADE IS OF ACCEPTABLE MATERIAL TO THE GEOTECHNICAL ENGINEER.

2. TOP OF SLAB ELEVATION = 0'-0"
TOP OF FOOTING (T/F) = (-0'-8") INTERIOR FOOTINGS UNO.
TOP OF FOOTING (T/F) = (-1'-4") EXTERIOR FOOTINGS UNO.
REINFORCING AT CONTINUOUS WALL FOOTINGS SHALL BE CONTINUOUS THROUGH COLUMN SPREAD FOOTINGS.

3. PROVIDE 2 - #4 X 3'-0" LONG AT ALL RE-ENTRANT CORNERS.

4. SEE ARCHITECTURAL PLANS FOR ADDITIONAL PLAN DIMENSIONS, SLAB EXTENTS, AND FLOOR SLOPE AND DEPRESSIONS.

5. SEE SHEETS S0.1 AND S0.2 FOR GENERAL NOTES.

6. SEE SHEET S3.1 FOR TYPICAL FOUNDATION DETAILS.

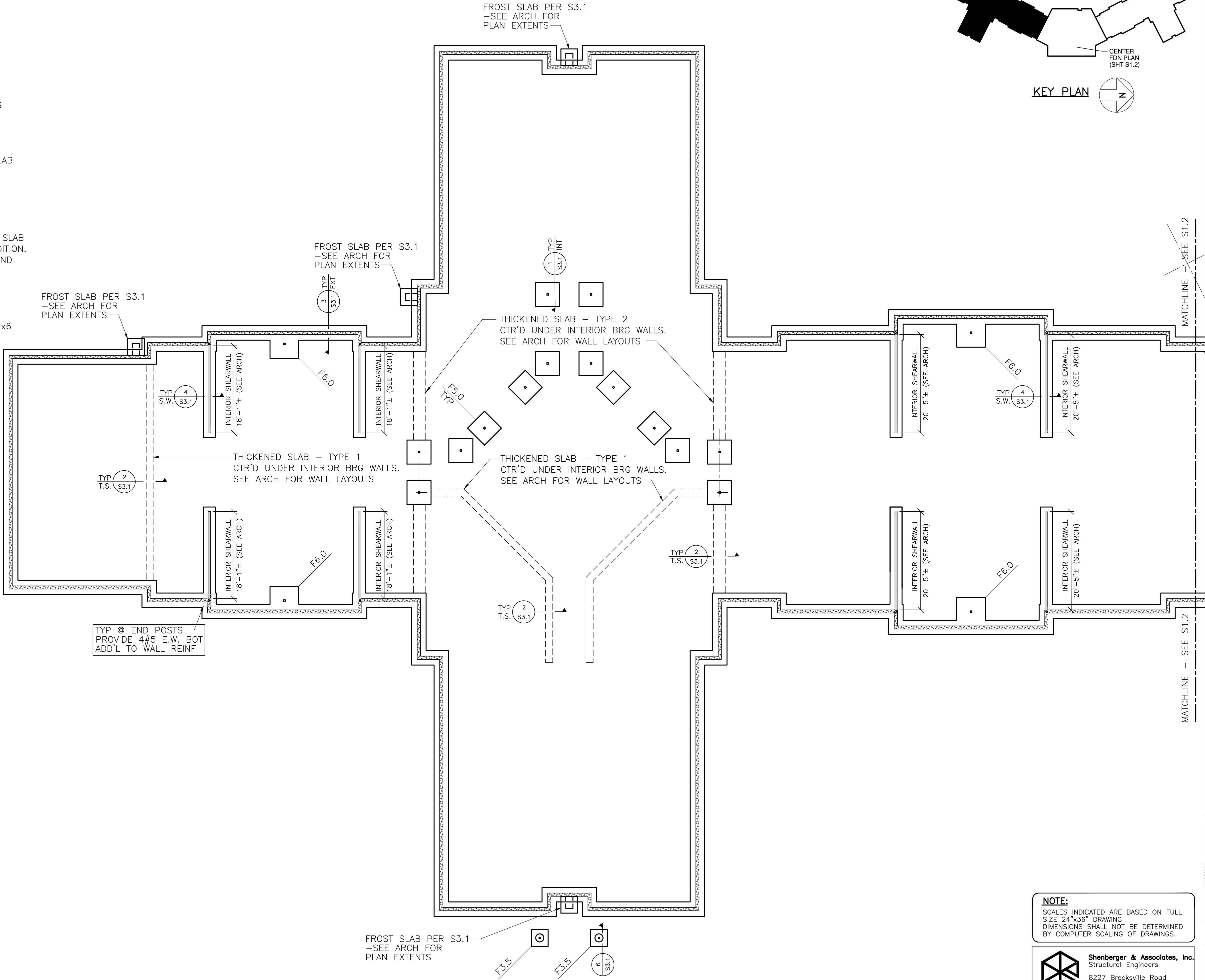
7. SEE ARCHITECTURAL DRAWINGS FOR FINISHED FLOOR ELEVATIONS, SLAB SLOPED, DRAINS, TRENCHES, RECESSES, AND ANY OTHER SLAB CONDITION. SEE MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS FOR SIZE AND LOCATION OF CONCRETE EQUIPMENT PADS.

8. SEE THIS SHEET FOR FOOTING SCHEDULE.

9. SEE SHEET S3.1 FOR TYPICAL TENSION ANCHOR REQUIRED TO BE AT EACH DOOR/WINDOW JAMB AND WALL END PANEL OF EXTERIOR 2x6 STUD FRAMING

10. COORDINATE ALL WALL & COLUMN LOCATION DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

11. FOR FOOTING SCHEDULE SEE S1.2.



NOTE:
SCALES INDICATED ARE BASED ON FULL SIZE 24"x36" DRAWING.
DIMENSIONS SHALL NOT BE DETERMINED BY COMPUTER SCALING OF DRAWINGS.

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HEALTH & REHABILITATION CENTER
1931/1915 BRIDGE ROAD (HIGHWAY 11)
SUFFOLK, VIRGINIA

COMMONWEALTH OF VIRGINIA
CHERYL GALLAGHER
Lic. No. 0402065165
6.18.26
PROFESSIONAL ENGINEER

DRAWING NAME
PARTIAL
FOUNDATION
PLAN

DSCA PROJECT NO.
24.061

S1.1

CENTER FOUNDATION PLAN

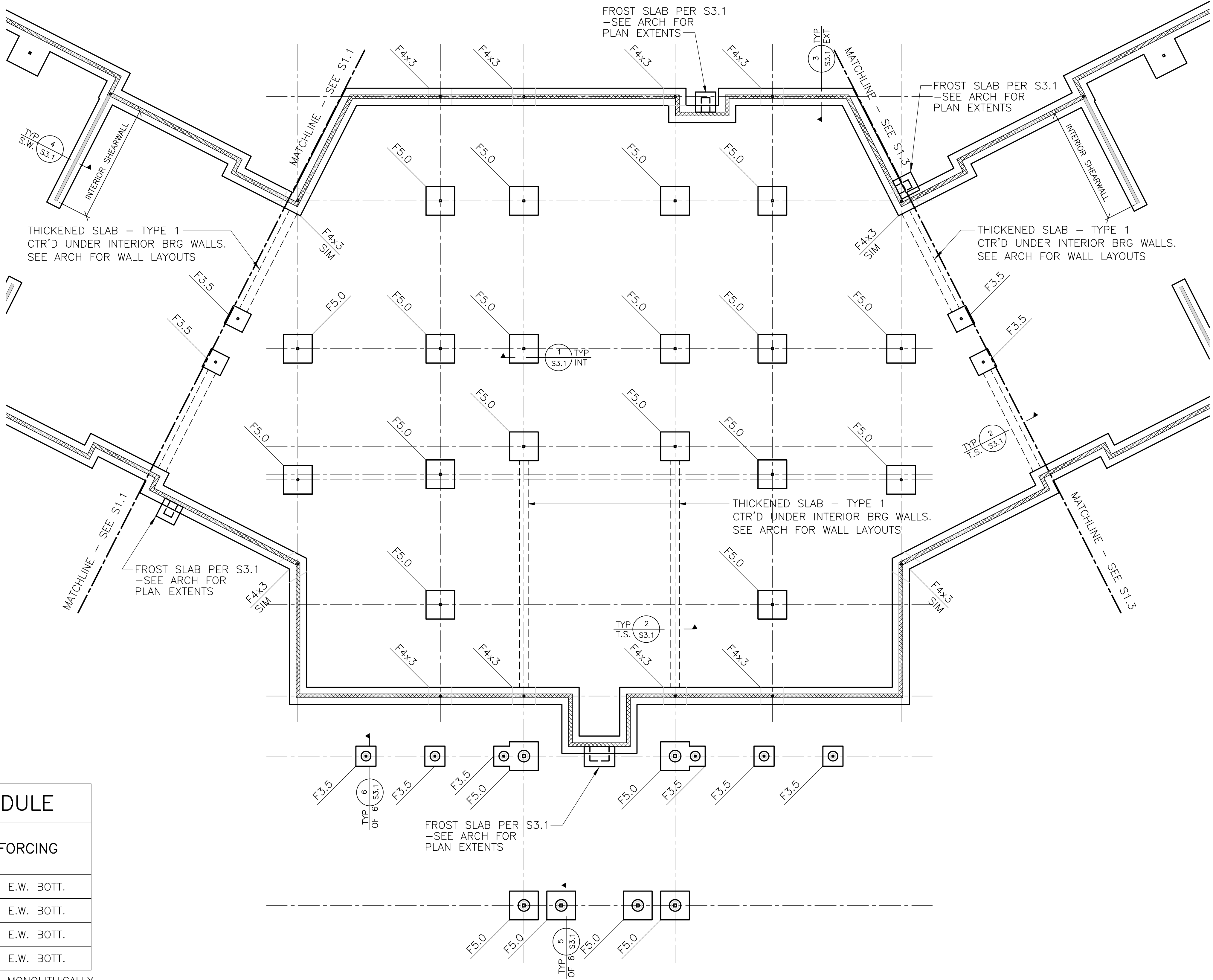
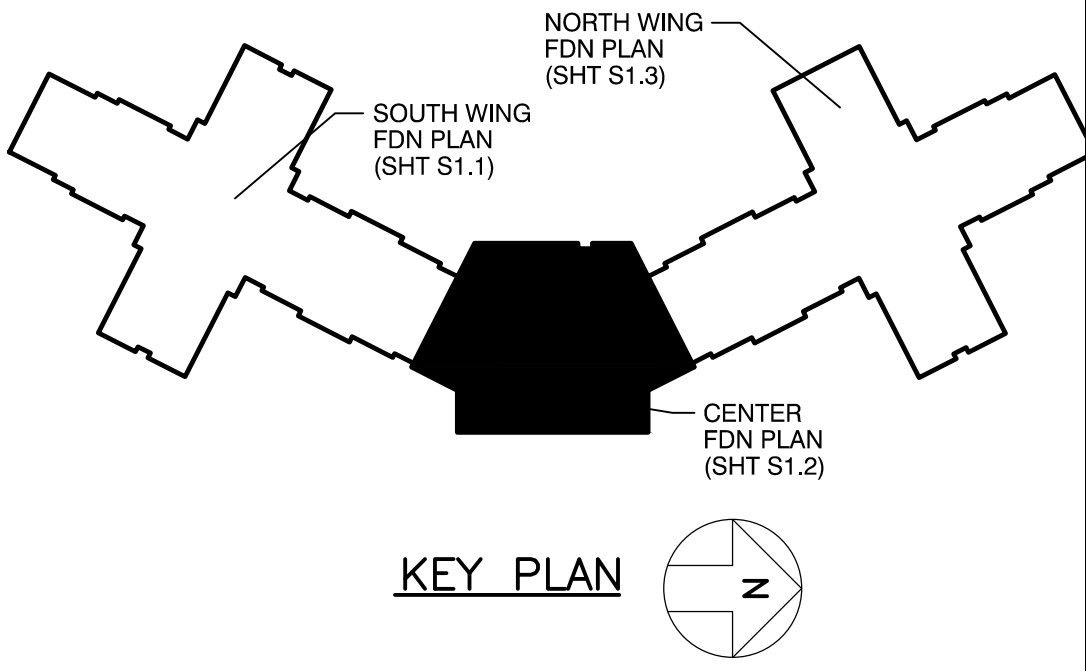
1"=10'-0"

NOTES:

1. FLOOR CONSTRUCTION: 4" CONCRETE SLAB ON GRADE WITH POLYPROPYLENE MICRO-FIBER REINF. PROVIDE MINIMUM 10-MIL VAPOR BARRIER AND MINIMUM 4" LAYER OF GRAVEL BELOW SLAB UNLESS EXISTING SUBGRADE IS OF ACCEPTABLE MATERIAL TO THE GEOTECHNICAL ENGINEER.

2. FOR ADDITIONAL PLAN NOTES SEE SHEET S1.1.

3. COORDINATE ALL WALL & COLUMN LOCATION DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

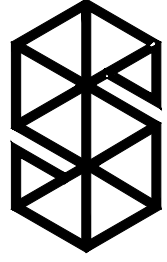


COLUMN FOOTING SCHEDULE		
MARK	FOOTING SIZE	REINFORCING
	LENGTH x WIDTH x DEPTH	
F3.5	3'-6"x3'-6"x1'-0"	4 - #5 E.W. BOTT.
F4x3**	4'-0"x3'-0"x1'-0"	4 - #5 E.W. BOTT.
F5.0	5'-0"x5'-0"x1'-0"	5 - #5 E.W. BOTT.
F6.0**	6'-0"x6'-0"x1'-0"	6 - #5 E.W. BOTT.

* COL FTGS SHALL BE CTR'D ON COL AND POURED MONOLITHICALLY WALL FTGS AND THICKENED SLABS. WALL FTG AND THICKENED SLAB CONT REINF SHALL BE CONT THROUGH COL FTGS.

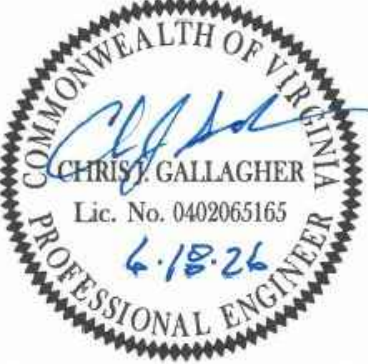
** POUR INTEGRAL WITH CONTINUOUS WALL FOOTING CENTER REQUIRED SCHEDULE REINFORCING UNDER POST.

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1931/1915 BRIDGE ROAD (HIGHWAY 11)
SUFFOLK, VIRGINIA



DRAWING NAME
PARTIAL
FOUNDATION
PLAN

DSCA PROJECT NO.
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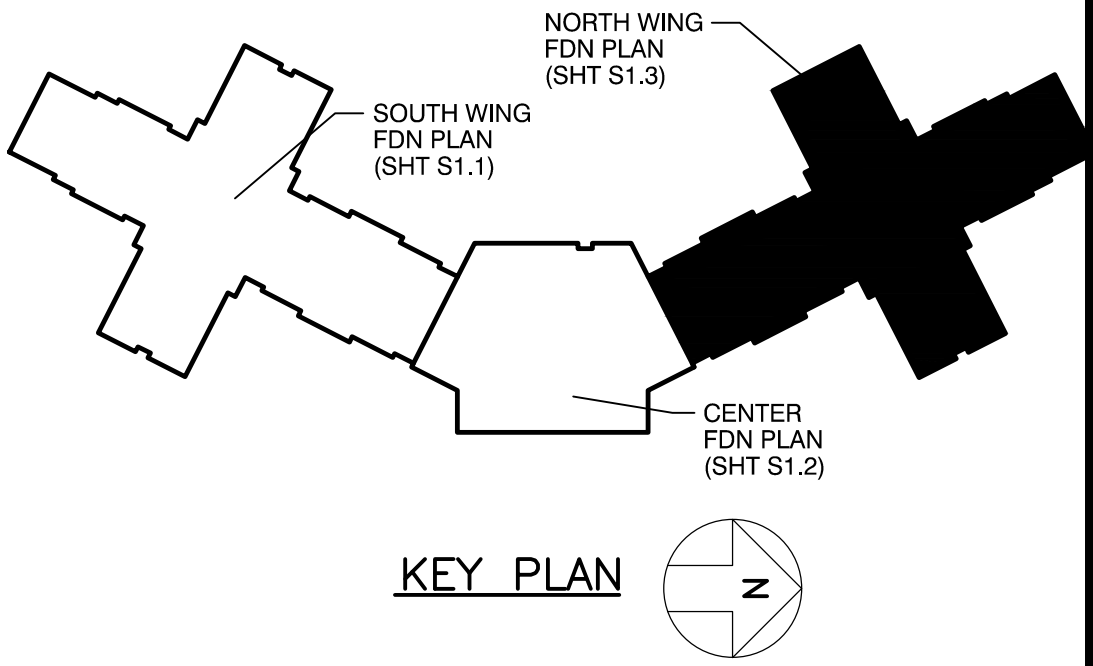
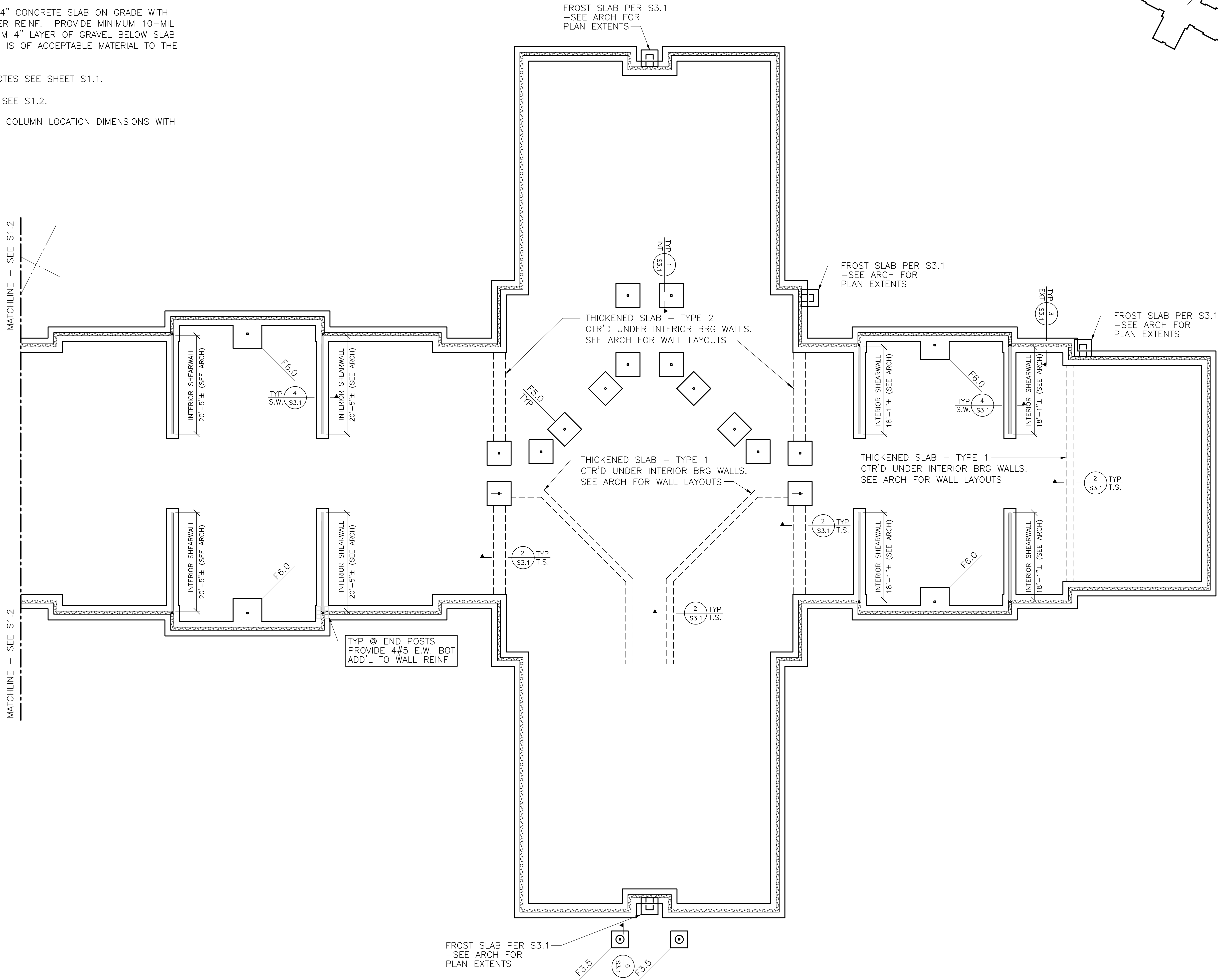
S1.2

NORTH WING FOUNDATION PLAN

1"=10'-0"

NOTES:

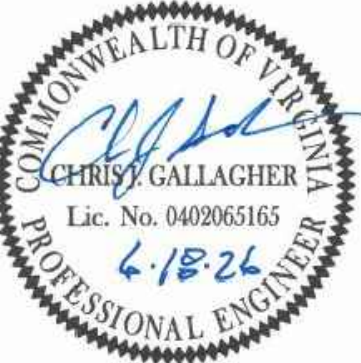
1. FLOOR CONSTRUCTION: 4" CONCRETE SLAB ON GRADE WITH POLYPROPYLENE MICRO-FIBER REINF. PROVIDE MINIMUM 10-MIL VAPOR BARRIER AND MINIMUM 4" LAYER OF GRAVEL BELOW SLAB UNLESS EXISTING SUBGRADE IS OF ACCEPTABLE MATERIAL TO THE GEOTECHNICAL ENGINEER.
2. FOR ADDITIONAL PLAN NOTES SEE SHEET S1.1.
3. FOR FOOTING SCHEDULE SEE S1.2.
4. COORDINATE ALL WALL & COLUMN LOCATION DIMENSIONS WITH ARCHITECTURAL DRAWINGS.



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DRAWING NAME
**PARTIAL
FOUNDATION
PLAN**

DSCA PROJECT NO.
24.061

S1.3

SOUTH WING ROOF FRAMING PLAN

1"=10'-0"

NOTES:

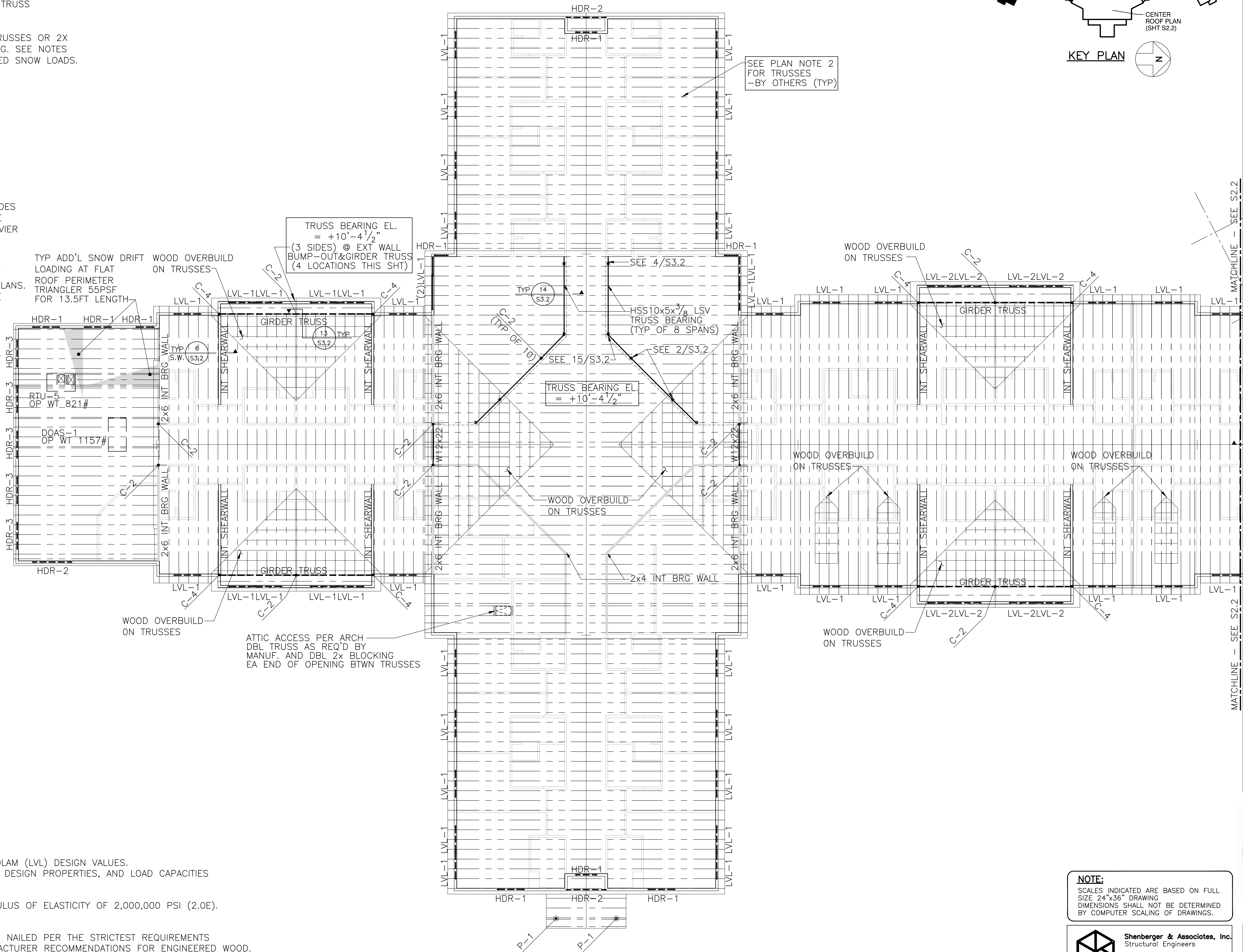
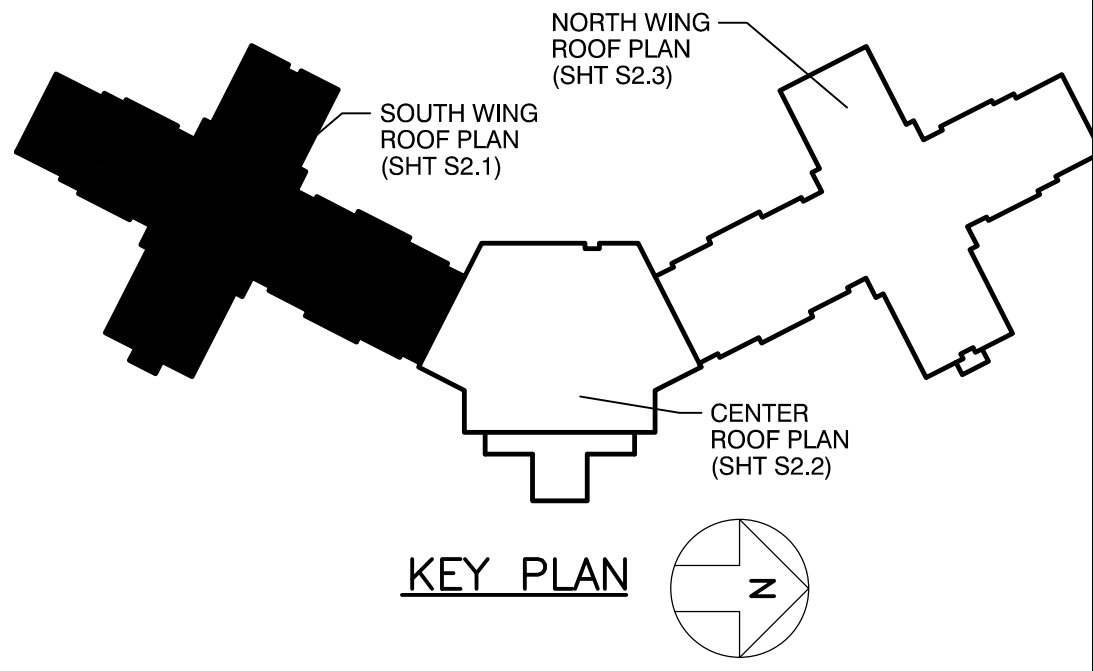
1. ROOF TRUSS BEARING ELEVATION SOUTH WING= +9'-1 1/8" (UNO)
2. TYPICAL TRUSS SPACING 2'-0" MAX. SEE TRUSS PROFILES NOTED ON ARCH DRAWINGS AND ASSOCIATED FILES ARCH ARCH SHEET TT. THE CONTRACTOR SHALL COORDINATE ALL DUCT LOCATIONS WITH THE TRUSS DESIGNER AND THE MECHANICAL CONTRACTOR.
3. AREAS NOTED AS OVERBUILD FRAMING MAY USE OVERBUILD TRUSSES OR 2X WOOD MEMBERS. OVERBUILDS REQUIRE ADDITIONAL 10PSF LOADING. SEE NOTES ON PLAN FOR TYPICAL SNOW DRIFT LOADINGS, ADD'L TO BALANCED SNOW LOADS.
4. SEE SHEETS S0.1 AND S0.2 FOR GENERAL NOTES.
5. SEE SHEET S3.2 FOR TYPICAL DETAILS.
6. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATION OF ALL ITEMS TO BE HUNG FROM TRUSSES WITH THE TRUSS DESIGNER. NO ALTERATION OF THE TRUSSES IN THE FIELD SHALL OCCUR WITHOUT THE PERMISSION OF THE TRUSS DESIGNER.
7. MECHANICAL SYSTEMS (I.E. DUCTS, UNITS, ORIENTATION, ETC.) ARE SHOWN ON THE MECHANICAL DRAWINGS. COORDINATION OF MECHANICAL UNIT SIZES, LOCATIONS, AND WEIGHTS WITH ALL TRADES IS THE RESPONSIBILITY OF THE INSTALLING CONTRACTOR. PROVIDE SUPPORTS TO ALL EDGES OF THE UNITS. ONLY MECH ITEMS HEAVIER THAN 300# ARE SHOWN ON PLAN. GC TO FIELD VERIFY WEIGHTS PRIOR TO INSTALLATION WITH MECH SUPPLIER, AND SHALL ALERT TRUSS DESIGNER TO ANY DISCREPANCIES PRIOR TO INSTALLATION.
8. ALL ROOF OPENINGS MAY NOT BE SHOWN ON THE FRAMING PLANS. FOR EXACT SIZE, NUMBER, AND LOCATION OF OPENINGS SEE THE ARCH, MECH, PLUMB, AND ELEC DRAWINGS. THE ROOF FRAMING SUPPLIER SHALL PROVIDE FRAMING TO ALL ROOF OPENINGS TO SUPPORT EDGES AND EQUIPMENT. FINAL DIMENSIONS SHALL BE COORDINATED WITH THE INSTALLING CONTRACTOR PRIOR TO COMMENCING THE WORK. SEE 12/S3.2 FOR TYP SUGGESTED DET.
9. SEE ARCHITECTURAL DRAWINGS FOR ALL WALL DIMENSIONS.
10. SEE S2.2 FOR COLUMN SCHEDULE
11. ALL SIMPSON STRONG-TIE CONNECTORS/HANGERS SHALL USE THE MAXIMUM ANCHORAGE CONDITION FOR EACH PIECE AS REQUIRED BY THE MANUF., UNLESS NOTED OTHERWISE.
12. SEE ARCH FOR LOCATIONS OF ATTIC LOADING. SEE S0.2 FOR TRUSS BOTTOM CHORD LOADING AT ATTIC AREAS.

HEADER SCHEDULE

- LVL-1 (3)1.75"x9.25" MICROLAM LVL 2.0E-2600Fb w/(2)2x6 MINIMUM END BEARING & (3)2x6 MINIMUM CENTER BEARING
- LVL-2 (3)1.75"x9.5" MICROLAM LVL 2.0E-2600Fb w/(2)2x6 MINIMUM END BEARING & (3)2x6 MINIMUM CENTER BEARING
- LVL-3 (3)1.75"x16" MICROLAM LVL 2.0E-2600Fb w/(4)2x6 MINIMUM END BEARING
- LVL-4 (2)1.75"x9.25" MICROLAM LVL 2.0E-2600Fb w/WOOD POSTS PER PLAN
- HDR-1 (2)2x8 w/(2)2x6 MINIMUM END BEARING
- HDR-2 (3)2x8 w/(2)2x6 MINIMUM END BEARING
- HDR-3 (3)2x10 w/(2)2x6 MINIMUM END BEARING &(3)2x6 MINIMUM CENTER BEARING

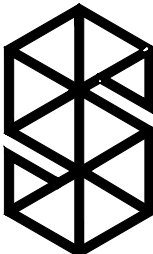
NOTES:

1. SEE S0.02 FOR WOOD GENERAL NOTES
2. ALL LAMINATED VENEER LUMBER (LVL) ARE BASED ON MICROLAM (LVL) DESIGN VALUES. ACCEPTABLE EQUALS SHALL MEET OR EXCEED THE DIMENSIONS, DESIGN PROPERTIES, AND LOAD CAPACITIES OF THE ENGINEERED WOOD MANUFACTURER AND THESE PLANS.
3. ALL LVL BEAMS AND HEADERS SHALL HAVE A MINIMUM MODULUS OF ELASTICITY OF 2,000,000 PSI (2.0E). AND AN Fb EQUAL TO PR GREATER THAN 2600 PSI (UNO).
4. ALL LINTEL MEMBERS SHALL BE FULL LENGTH AND GLUED & NAILED PER THE STRICTEST REQUIREMENTS BETWEEN THE GENERAL NOTES, LOCAL CODES, AND THE MANUFACTURER RECOMMENDATIONS FOR ENGINEERED WOOD.
5. PROVIDE MINIMUM OF (1)2X ABOVE AND BELOW HEADERS. MAINTAIN THE DOUBLE TOP PLATES OF HEADER OCCURS JUST BELOW OF THE WOOD WALL ASSEMBLIES. SEE S3.2 AND ARCH FOR ADDITIONAL WOOD CONSTRUCTION ADDITIONAL DETAILS.

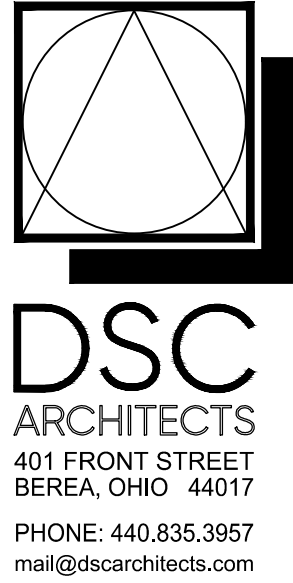


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SUFFOLK, VIRGINIA



DRAWING NAME
PARTIAL
ROOF
FRAMING
PLAN
DSCA PROJECT NO.
24.061

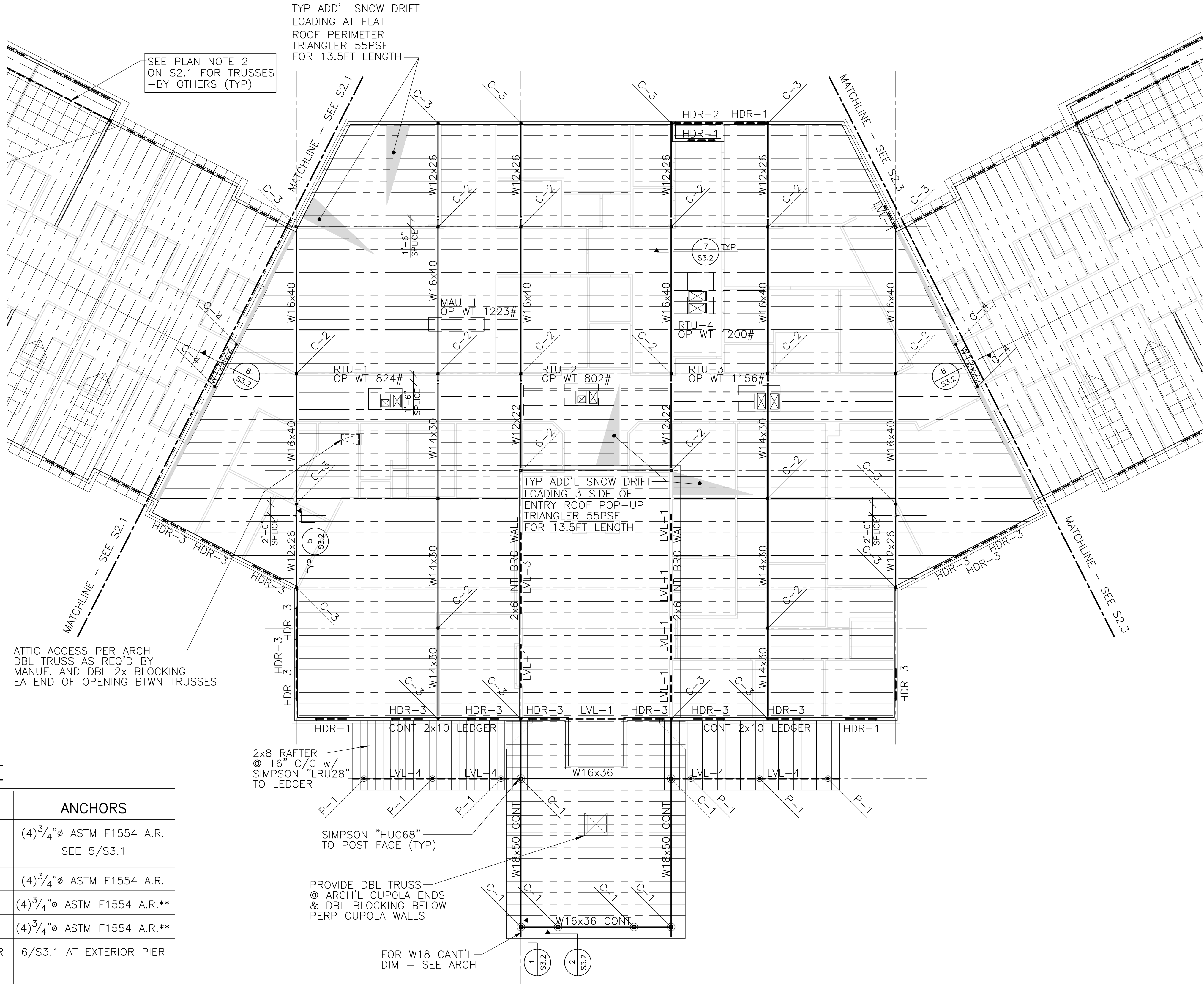
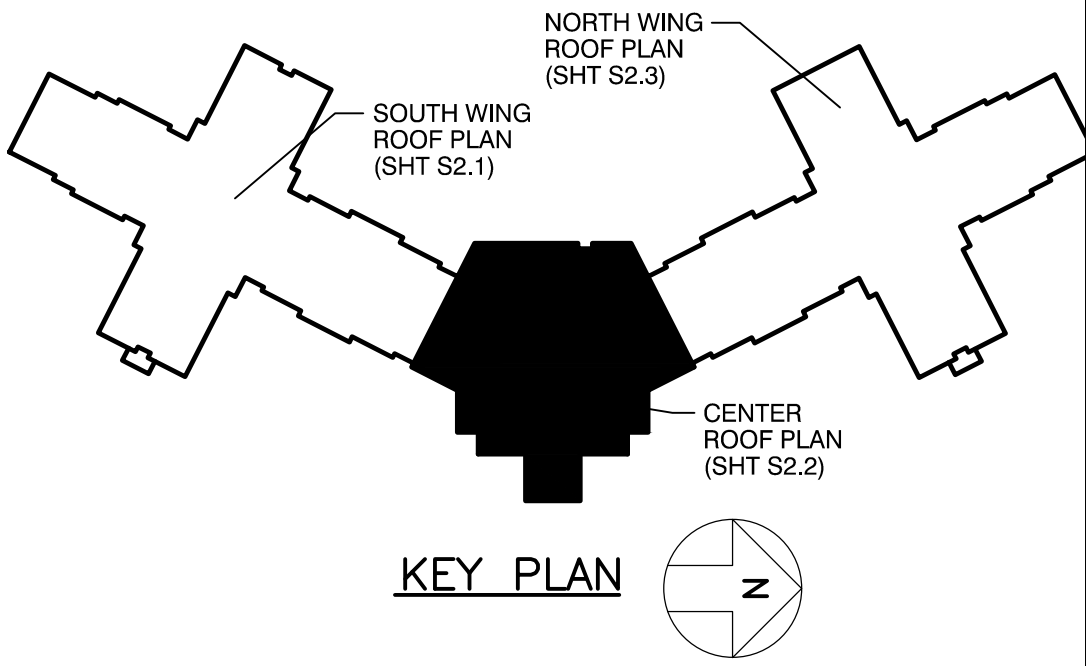
S2.1

CENTER ROOF FRAMING PLAN

1"=10'-0"

NOTES:

1. ROOF TRUSS BEARING ELEVATION CENTER = +10'-4 1/2"
ROOF TRUSS BEARING ELEVATION ENTRY = +14'-4 1/2"
SEE S2.1 & S2.3 FOR TRUSS BEARING AT WINGS.
2. FOR ADDITIONAL PLAN NOTES AND HEADER SCHEDULE SEE SHEET S2.1.
3. ALL SIMPSON STRONG-TIE CONNECTORS/HANGERS SHALL USE THE MAXIMUM ANCHORAGE CONDITION FOR EACH PIECE AS REQUIRED BY THE MANUFACTURER, UNLESS NOTED OTHERWISE.
4. SEE ARCH FOR LOCATIONS OF ATTIC LOADING. SEE S0.2 FOR TRUSS BOTTOM CHORD LOADING AT ATTIC AREAS.



COLUMN SCHEDULE

MARK	SIZE	BASEPLATE	ANCHORS
C-1	HSS6x6x3/8	3/4"x12"x1'-0" SEE 5/S3.1	(4)3/4"Ø ASTM F1554 A.R. SEE 5/S3.1
C-2	HSS4x4x5/16	3/4"x10"x0'-10"	(4)3/4"Ø ASTM F1554 A.R.
C-3	4"Ø STD STL PIPE	3/4"x10"x0'-10"**	(4)3/4"Ø ASTM F1554 A.R.**
C-4	3 1/2"Ø STD STL PIPE	3/4"x9"x0'-9"**	(4)3/4"Ø ASTM F1554 A.R.**
P-1	4x4 SOLID WOOD POST STRUCTURAL GRADE (7000 LBS MAX LOAD)	6/S3.1 AT EXTERIOR PIER	6/S3.1 AT EXTERIOR PIER
P-2	6x6 SOLID WOOD POST STRUCTURAL GRADE (15000 LBS MAX LOAD)	6/S3.1 AT EXTERIOR PIER	6/S3.1 AT EXTERIOR PIER

* COLUMNS SHALL BE CENTERED IN THE ARCHITECTURALLY DIMENSIONED WALL SPACES AND/OR UNDER THE TRUSS GIRDERS. SEE ARCH DWGS FOR WALL DIMENSIONS.
** SCHEDULE BASEPLATES ARE FOR INTERIOR SPREAD FOOTINGS OR PIERS ON SPREAD FOOTINGS. FOR POSTS IN EXTERIOR WALLS SEE S3.1 FOR SPECIAL BASEPLATES.



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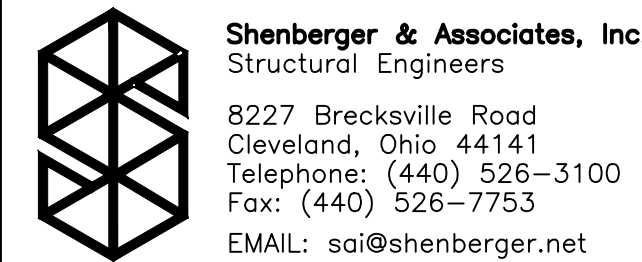
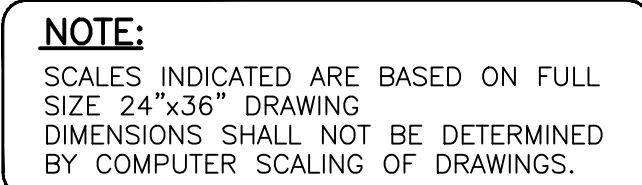
S2.2

$$1'' = 10' - 0''$$

1. ROOF TRUSS BEARING ELEVATION NORTH WING = $+9'-1\frac{1}{8}"$ (UNO)
2. FOR ADDITIONAL PLAN NOTES AND COLUMN SCHEDULE SEE SHEET S2.1.
3. SEE S2.2 FOR COLUMN SCHEDULE
4. SEE S2.1 FOR HEADER SCHEDULE
5. ALL SIMPSON STRONG-TIE CONNECTORS/HANGERS SHALL USE THE MAXIMUM ANCHORAGE CONDITION FOR EACH PIECE AS REQUIRED BY THE MANUFACTURER, UNLESS NOTED OTHERWISE.
6. SEE ARCH FOR LOCATIONS OF ATTIC LOADING. SEE S0.2 FOR TRUSS BOTTOM CHORD LOADING AT ATTIC AREAS.



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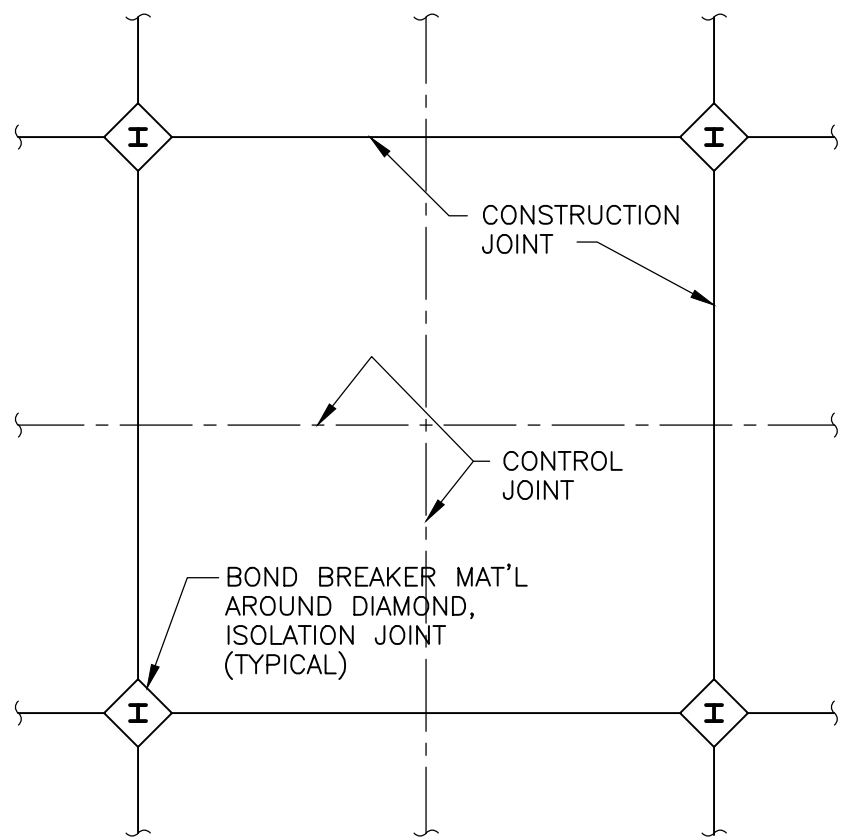


HEALTH & REHABILITATION CENTER
1931/1915 BRIDGE ROAD (HIGHWAY 17)
SUFFOLK, VIRGINIA

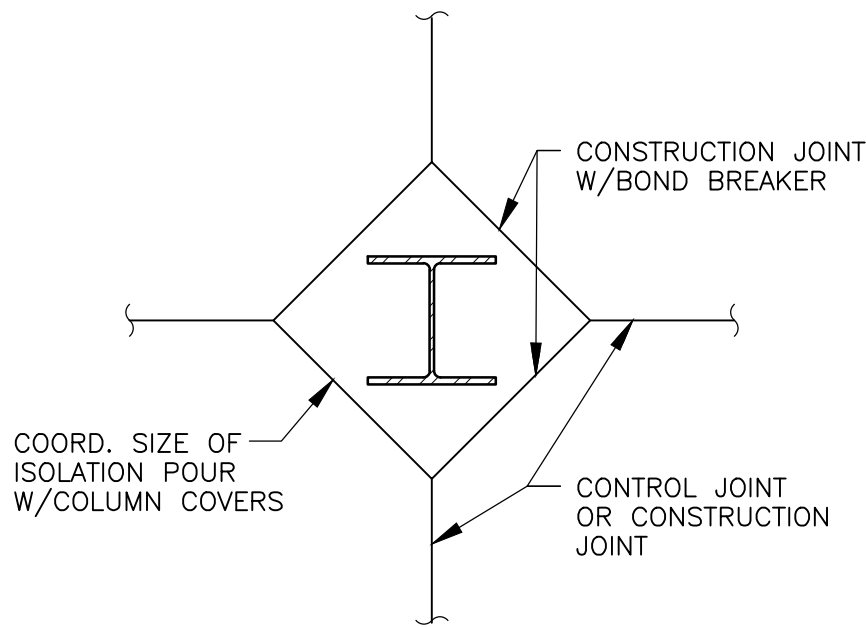


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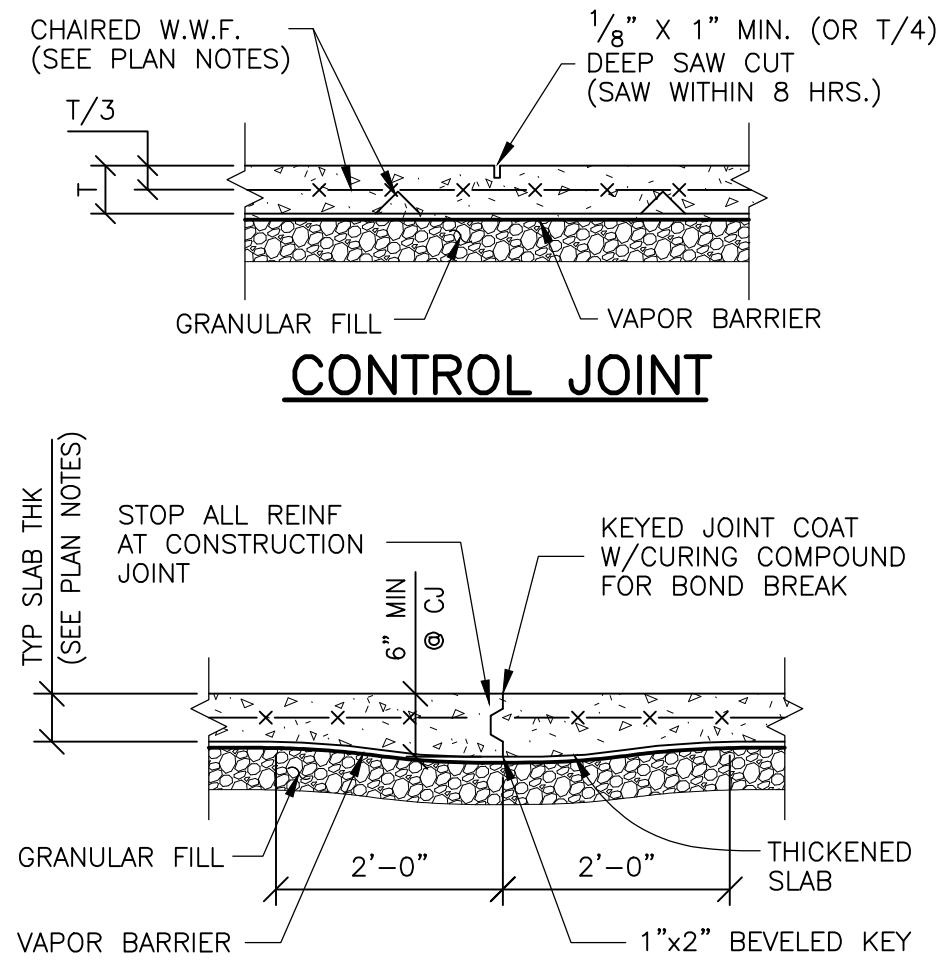
S2.3



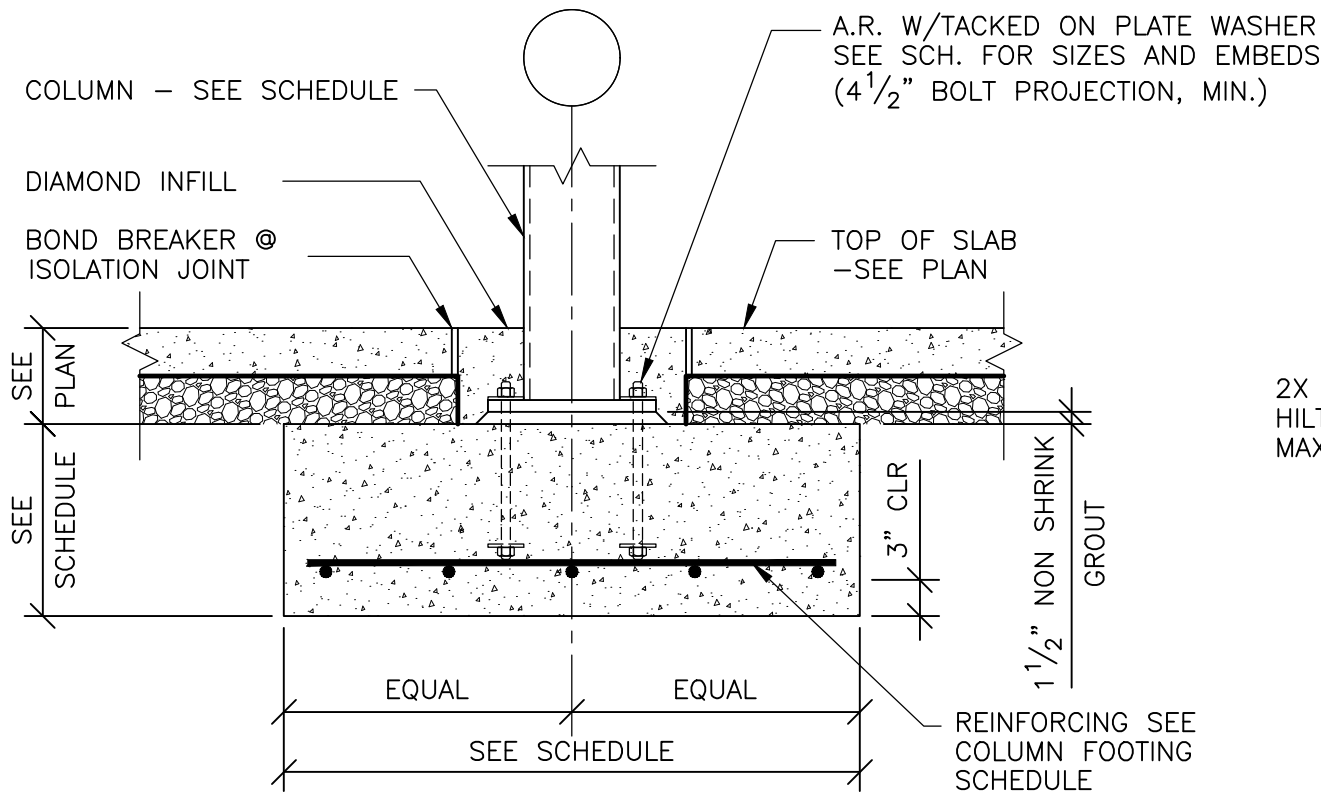
TYPICAL BAY
SLAB ON GRADE JOINT DETAILS



TYPICAL SLAB JOINT
AT COLUMN

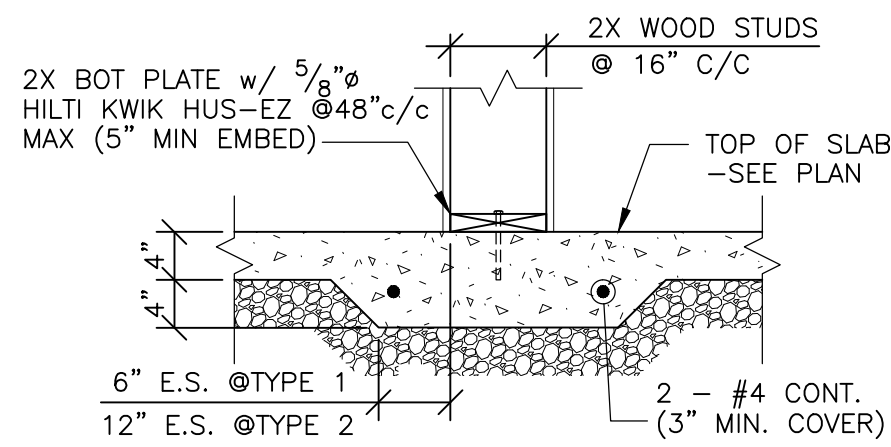


CONSTRUCTION JOINT



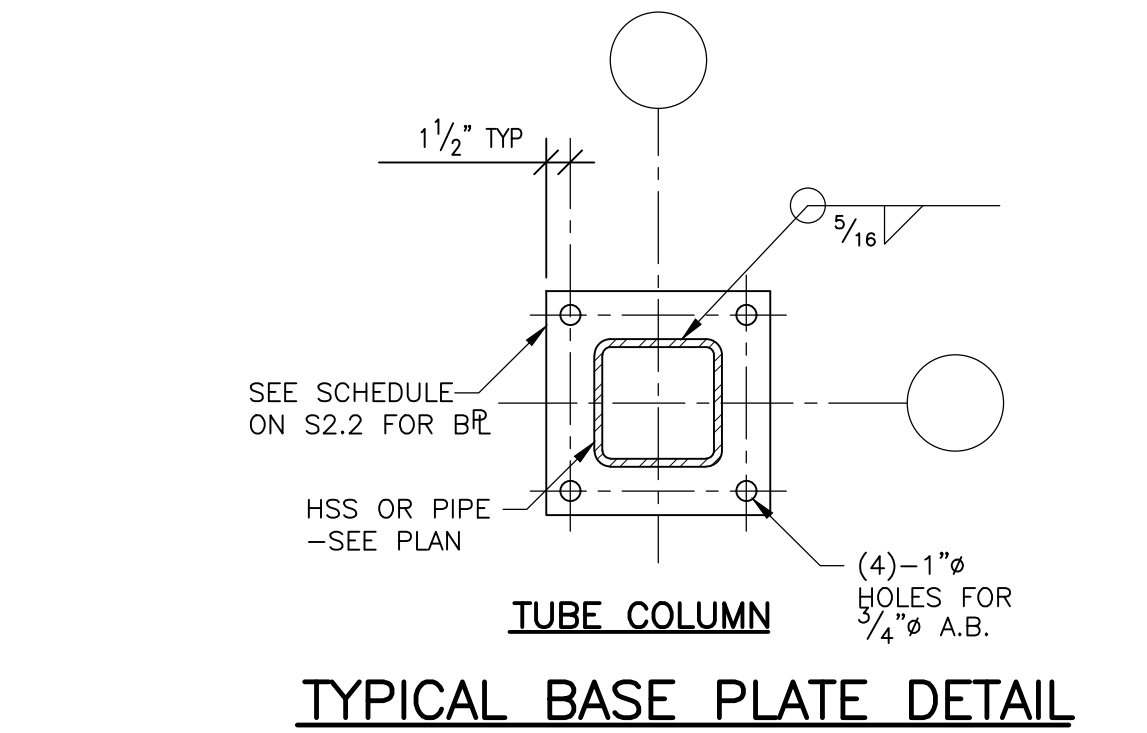
TYPICAL INTERIOR COLUMN FOOTING DETAIL

SECTION 1
3/4"=1'-0"

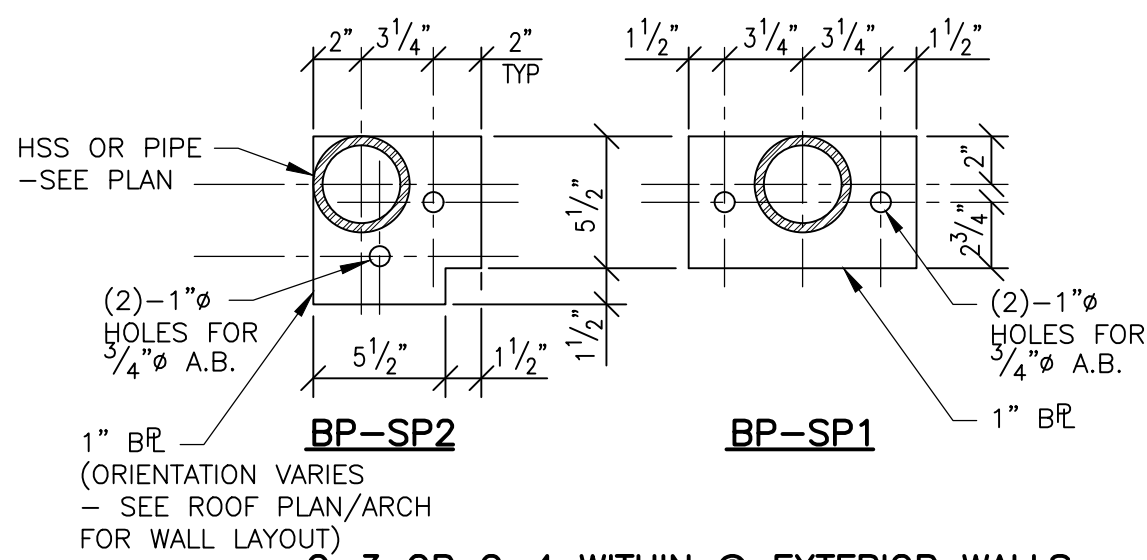


TYPICAL THICKENED SLAB
AT INTERIOR LOAD-BEARING WALLS

SECTION 2
3/4"=1'-0"

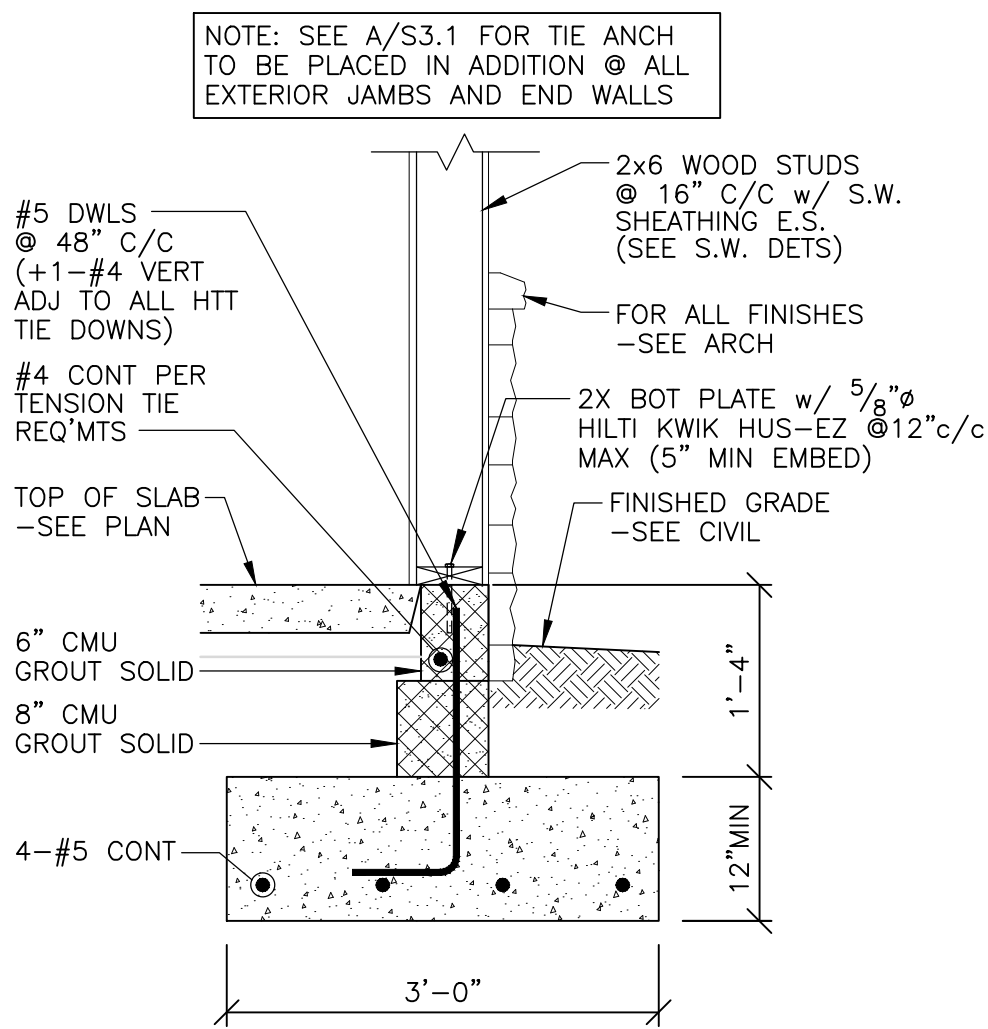


TYPICAL BASE PLATE DETAIL



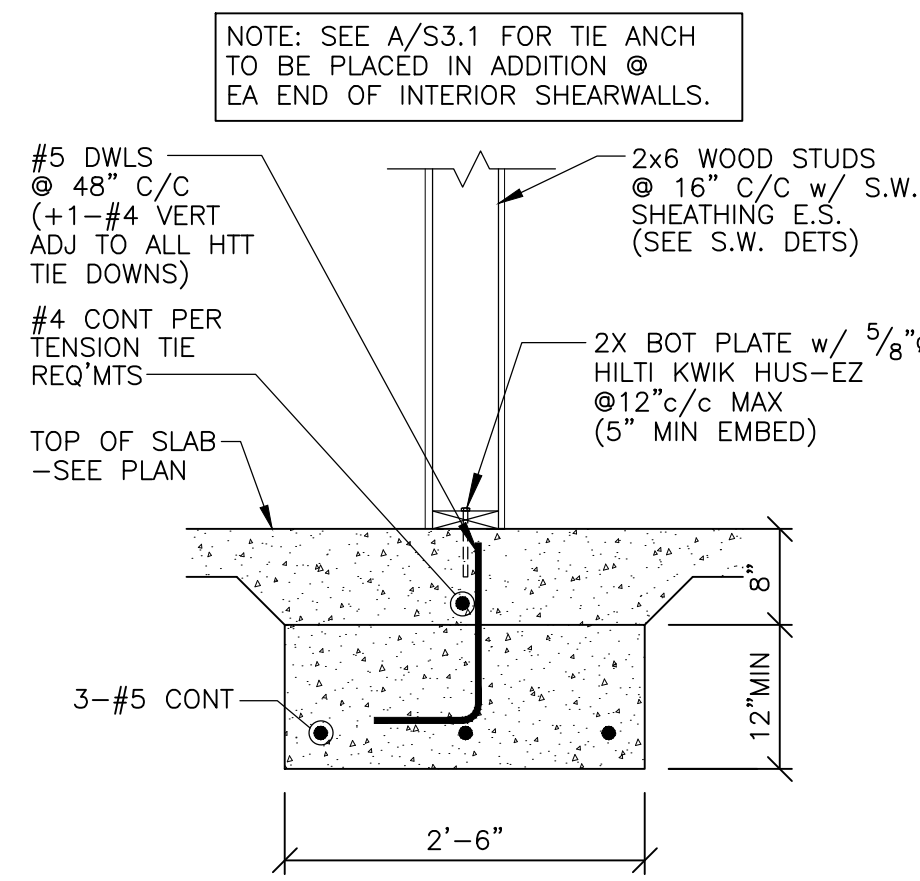
C-3 OR C-4 WITHIN EXTERIOR WALLS
SPECIAL BASE PLATE DETAILS

NOTE: BR TO SIT ATOP 6" CMU IN SECTION 3
ON THIS SHEET WITHIN THE EXTERIOR 2x6 WALL SPACE



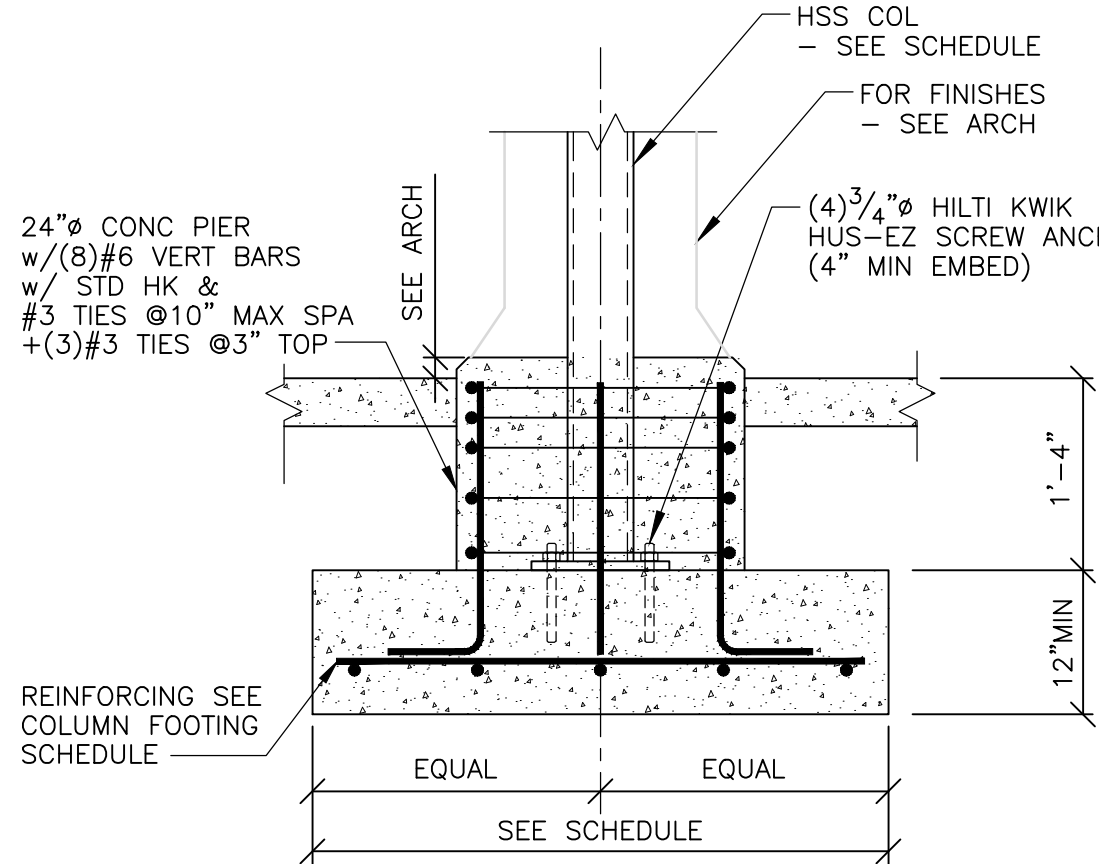
TYPICAL EXTERIOR FOOTING DETAIL

SECTION 3
3/4"=1'-0"

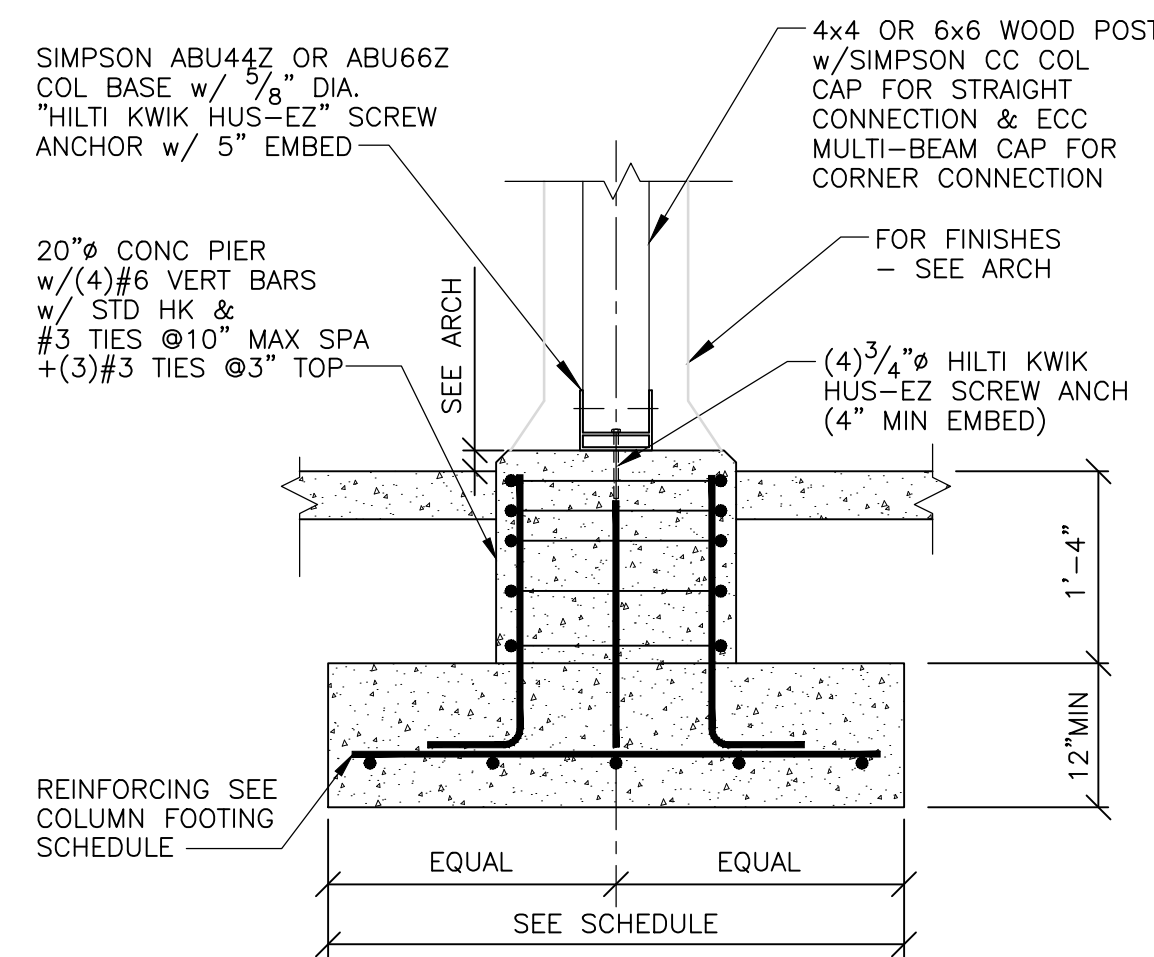


TYPICAL INTERIOR SHEARWALL
FOOTING DETAIL

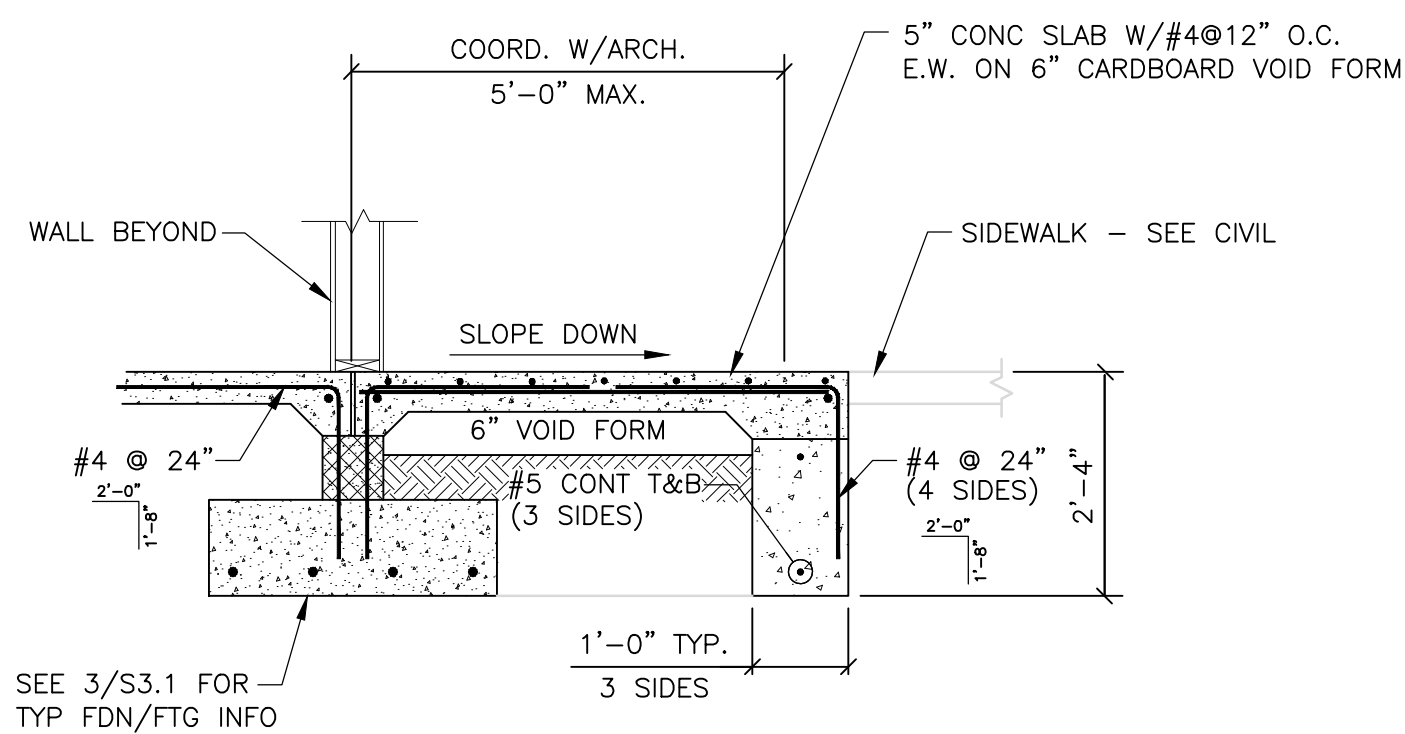
SECTION 4
3/4"=1'-0"



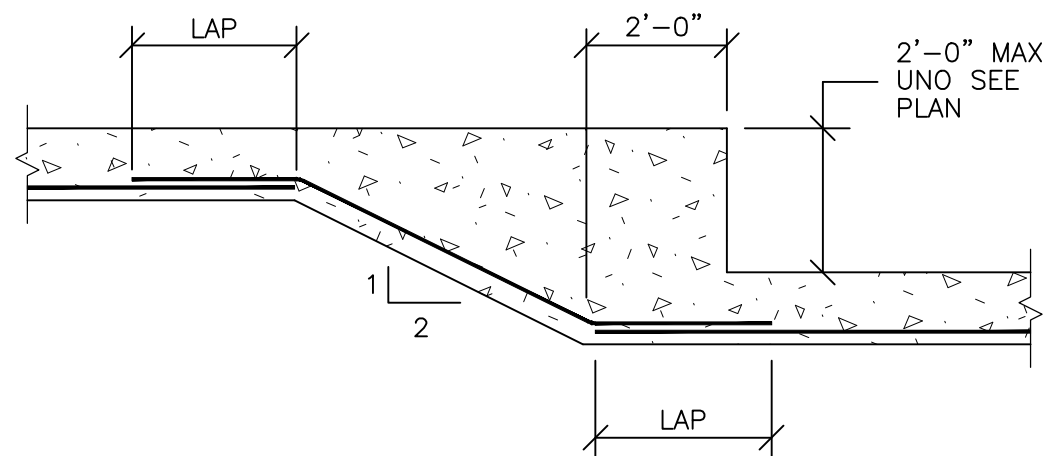
SECTION 5
3/4"=1'-0"



SECTION 6
3/4"=1'-0"

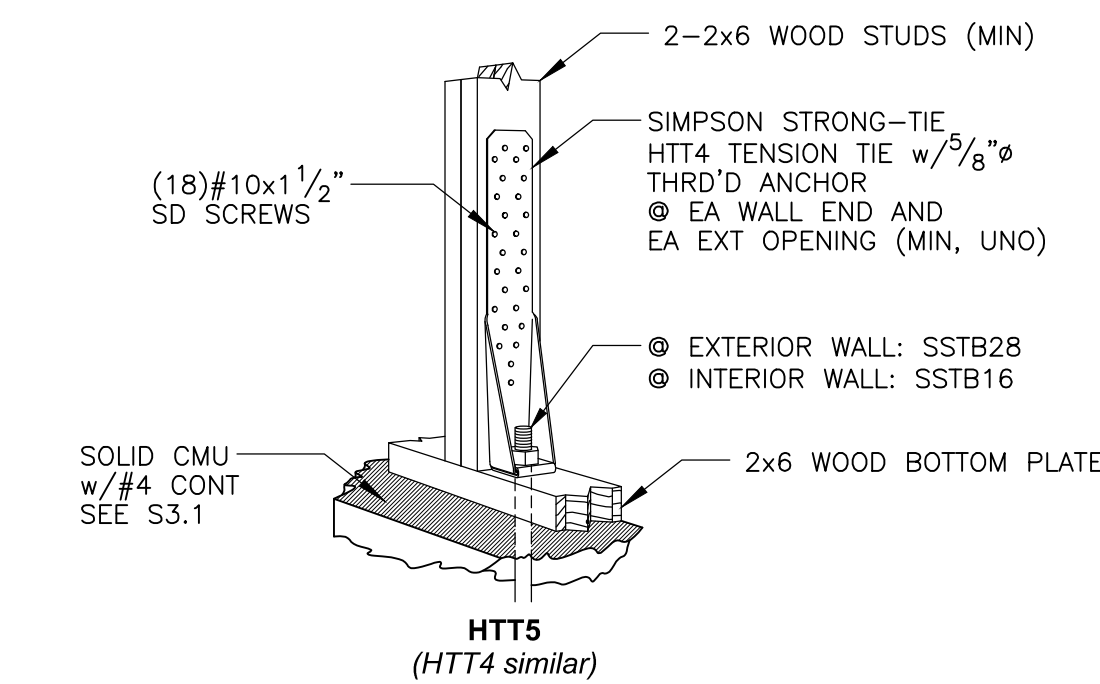


TYPICAL FROST SLAB DETAIL



FOOTING STEP DETAIL

GC TO COORD REQUIREMENT WITH CIVIL GRADES
AND GEOTECH 18" MINIMUM FROST DEPTH



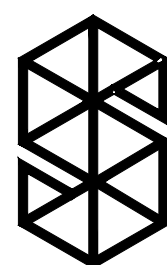
TYPICAL STUD WALL PANEL TENSION TIE

NTS

DETAIL A

NOTE:

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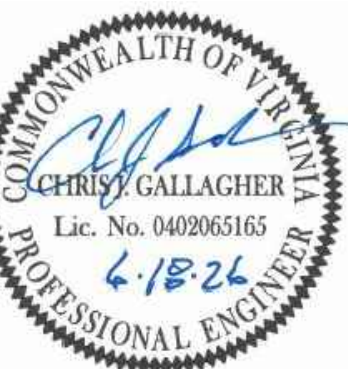


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PERMIT SUBMISSION:
06/19/26

BENNETT'S CREEK
HEALTH & REHABILITATION CENTER
193/1915 BRIDGE ROAD (HIGHWAY 17)
SUFFOLK, VIRGINIA

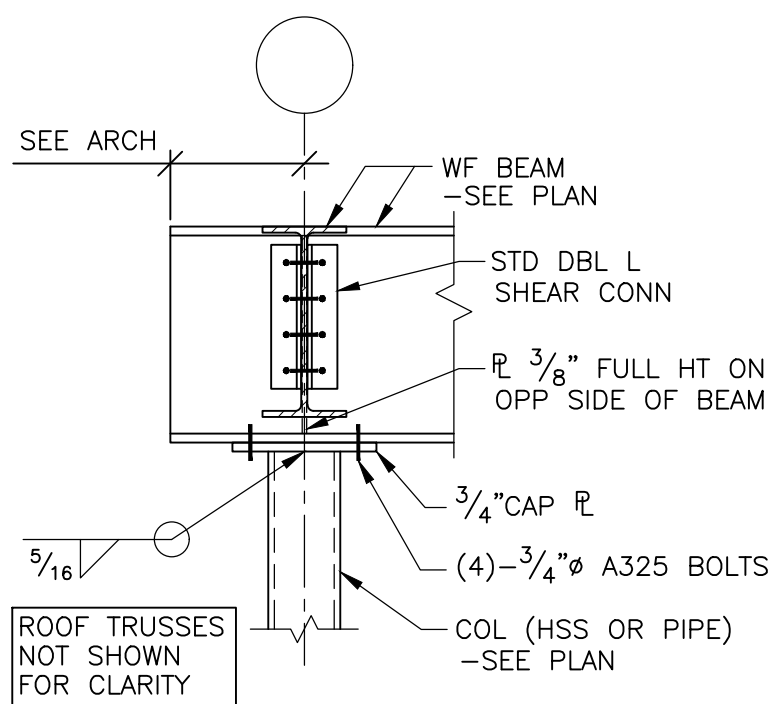


DRAWING NAME
FOUNDATION
DETAILS

DSCA PROJECT NO.
24.061

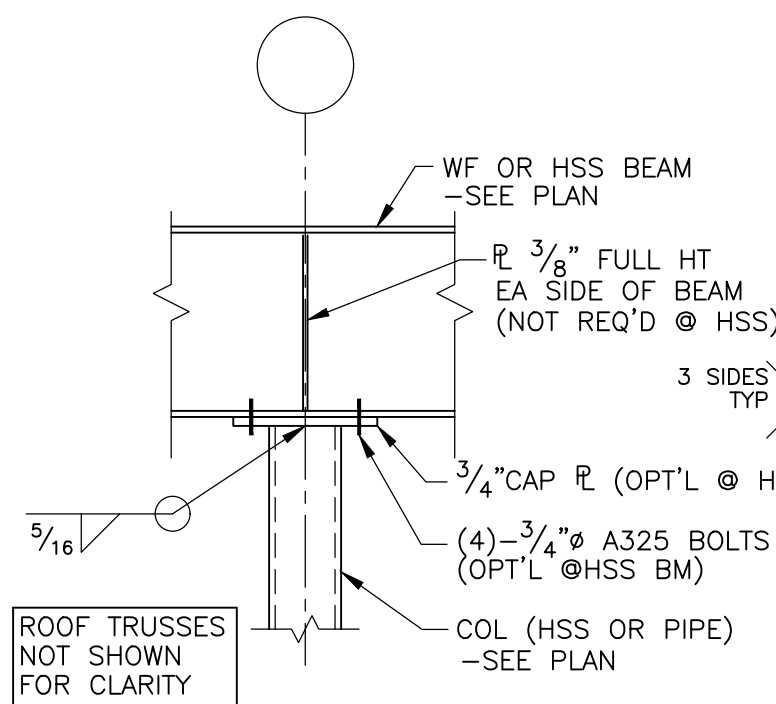
S3.1

S:\Jun 22, 2026 - 12:05pm - K:\DRAW-26126038 - Bennett's Creek, Suffolk VA\26038_S3.2.dwg - Layout: S3.2



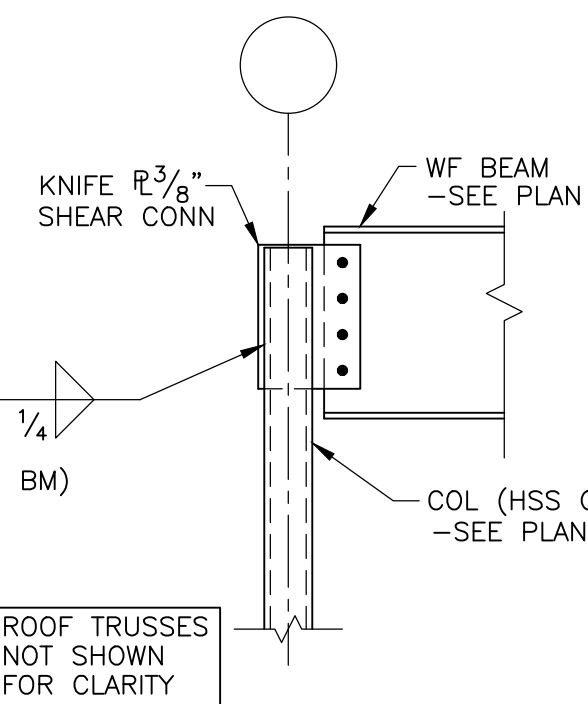
TYPICAL CANTILEVER BEAM

SECTION 1
3/4"=1'-0"



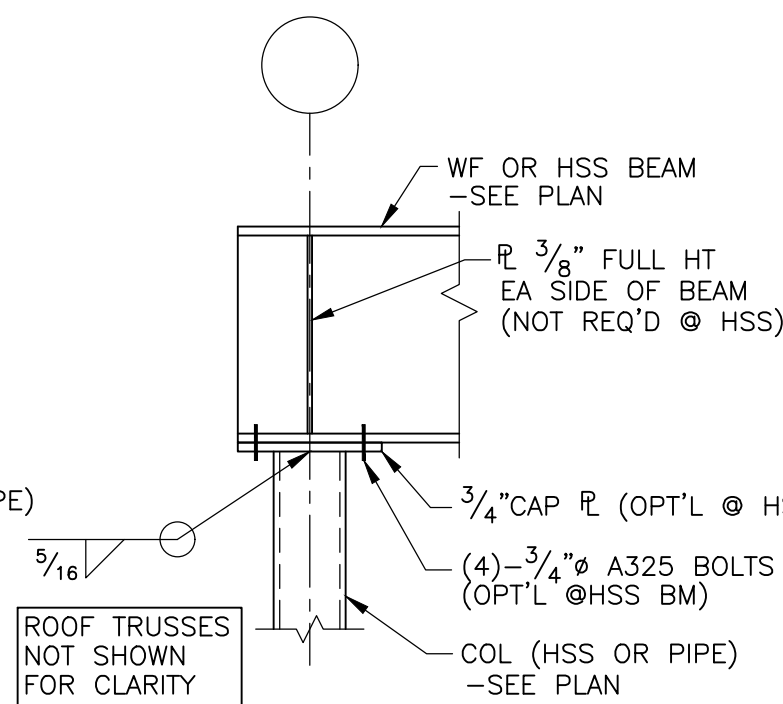
TYPICAL CONT BEAM

SECTION 2
3/4"=1'-0"



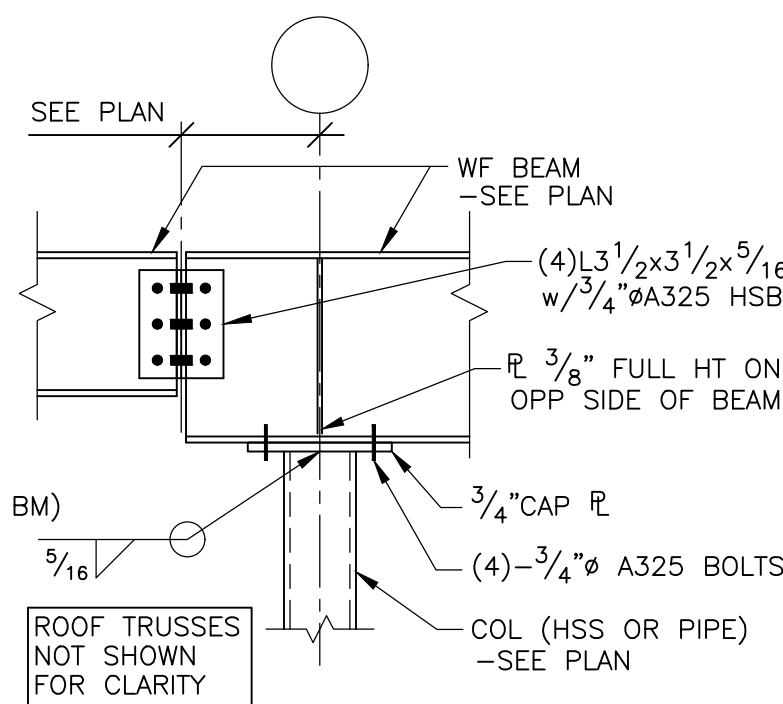
BEAM END CONN TYPE 1

EXTERIOR WALLS
SECTION 3
3/4"=1'-0"



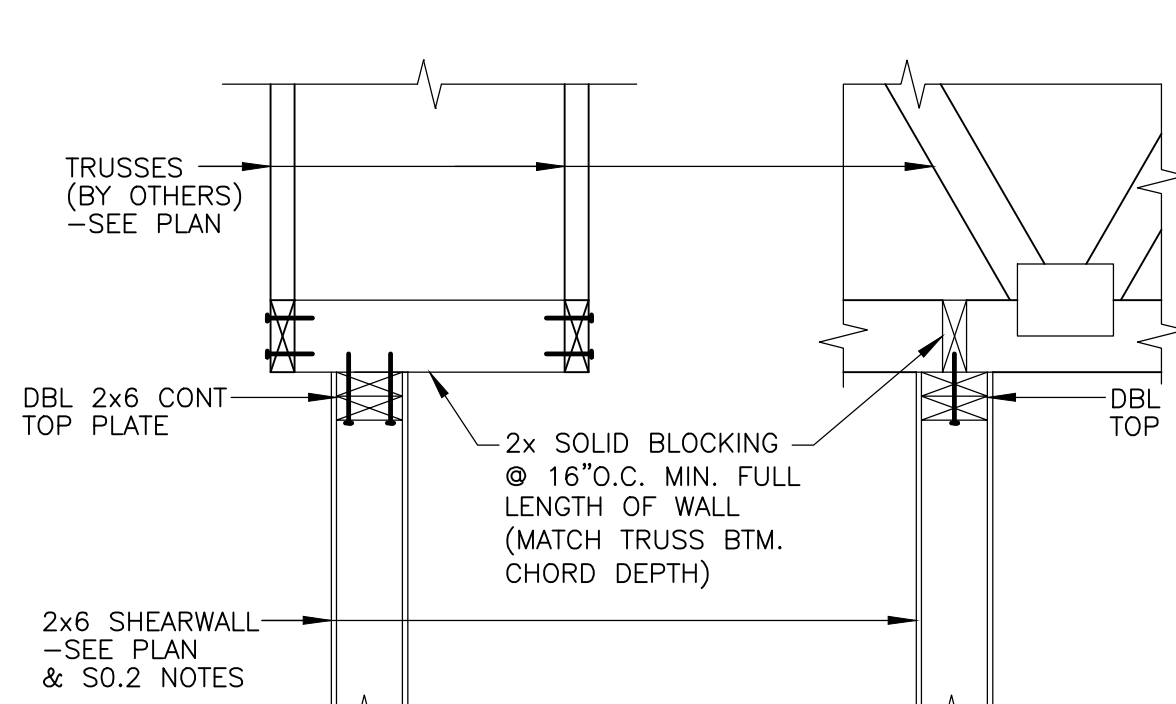
BEAM END CONN TYPE 2

INTERIOR WALLS
SECTION 4
3/4"=1'-0"



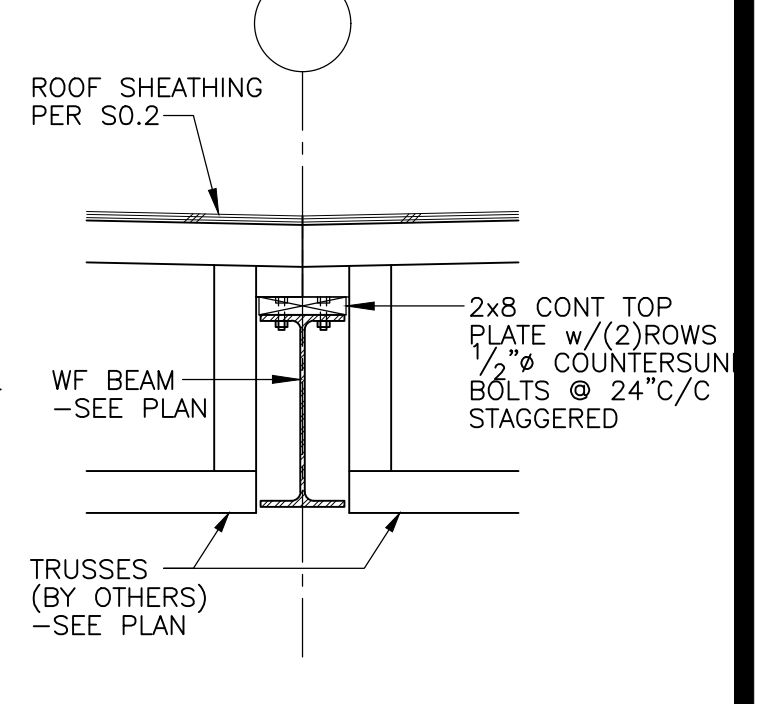
TYPICAL SPLICE DETAIL

SECTION 5
3/4"=1'-0"



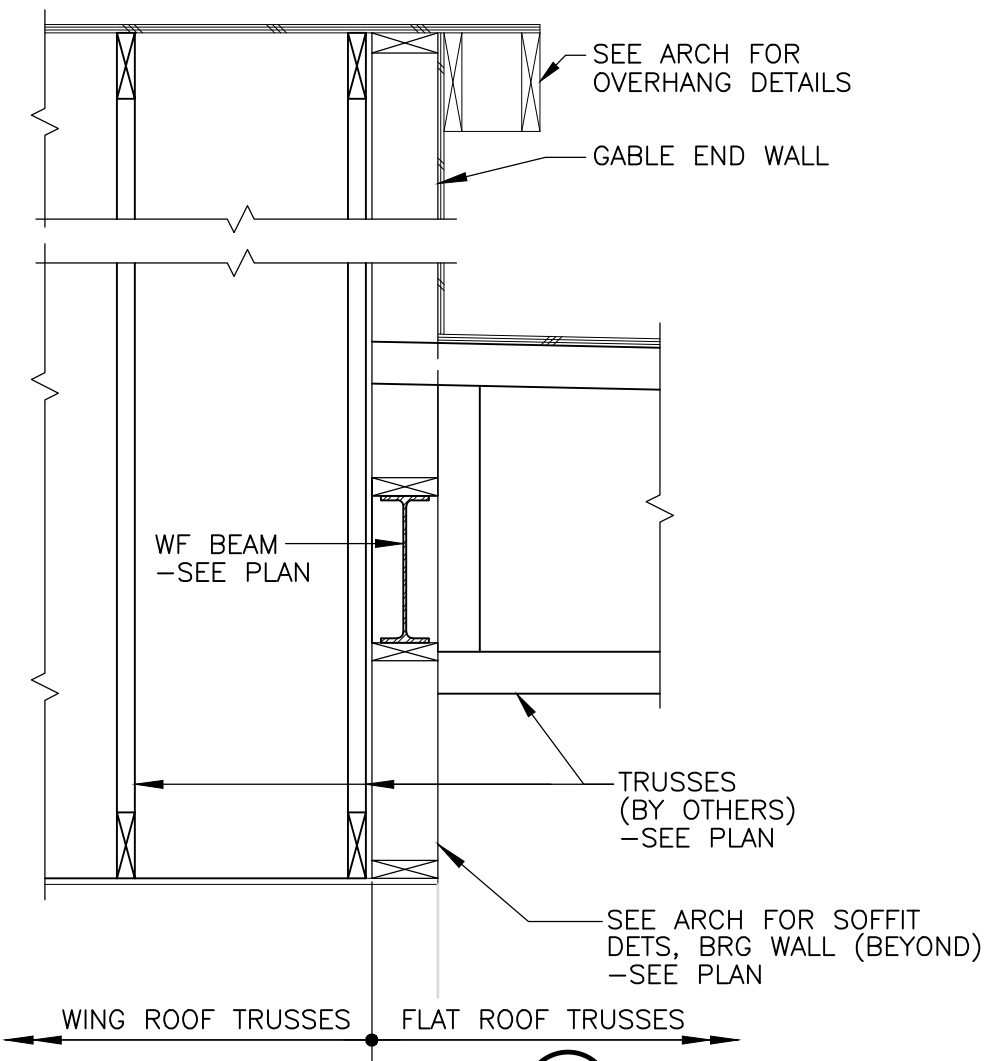
TOP OF SHEARWALL DETAIL

SECTION 6
3/4"=1'-0"

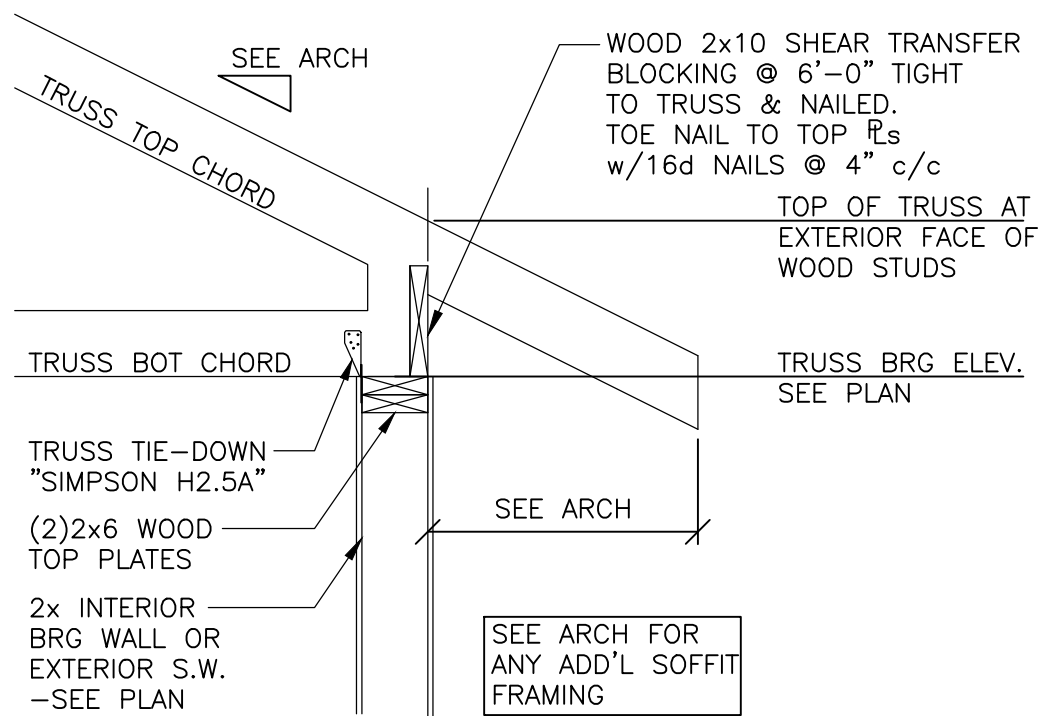


TYPICAL FLAT ROOF TRUSS BEARING

SECTION 7
3/4"=1'-0"

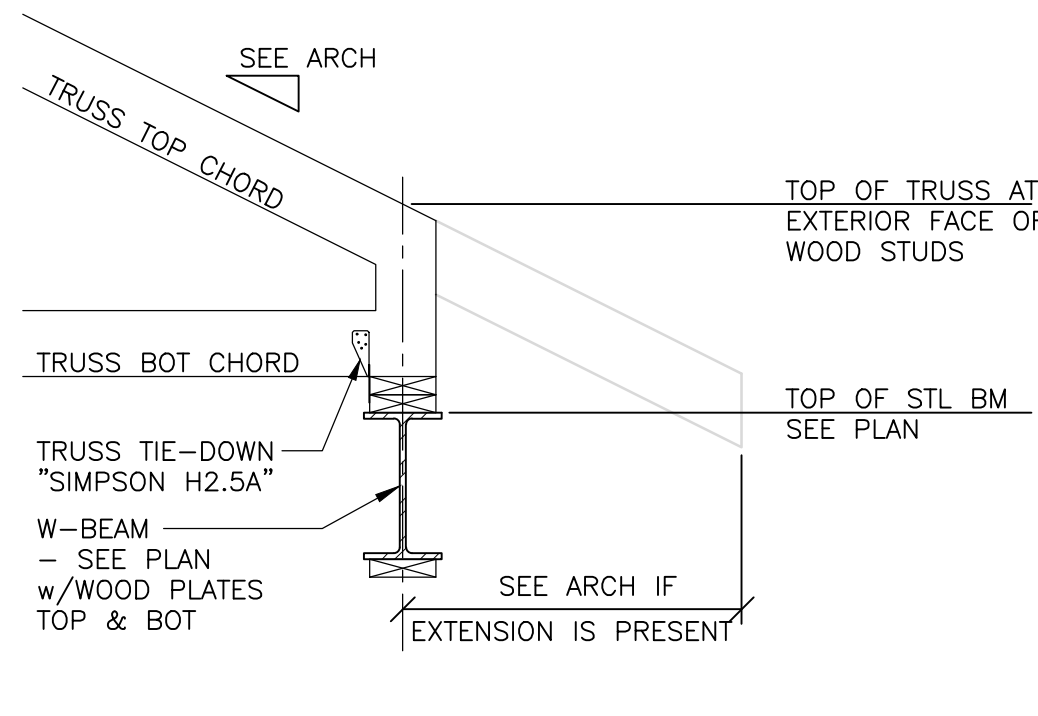


SECTION 8
3/4"=1'-0"



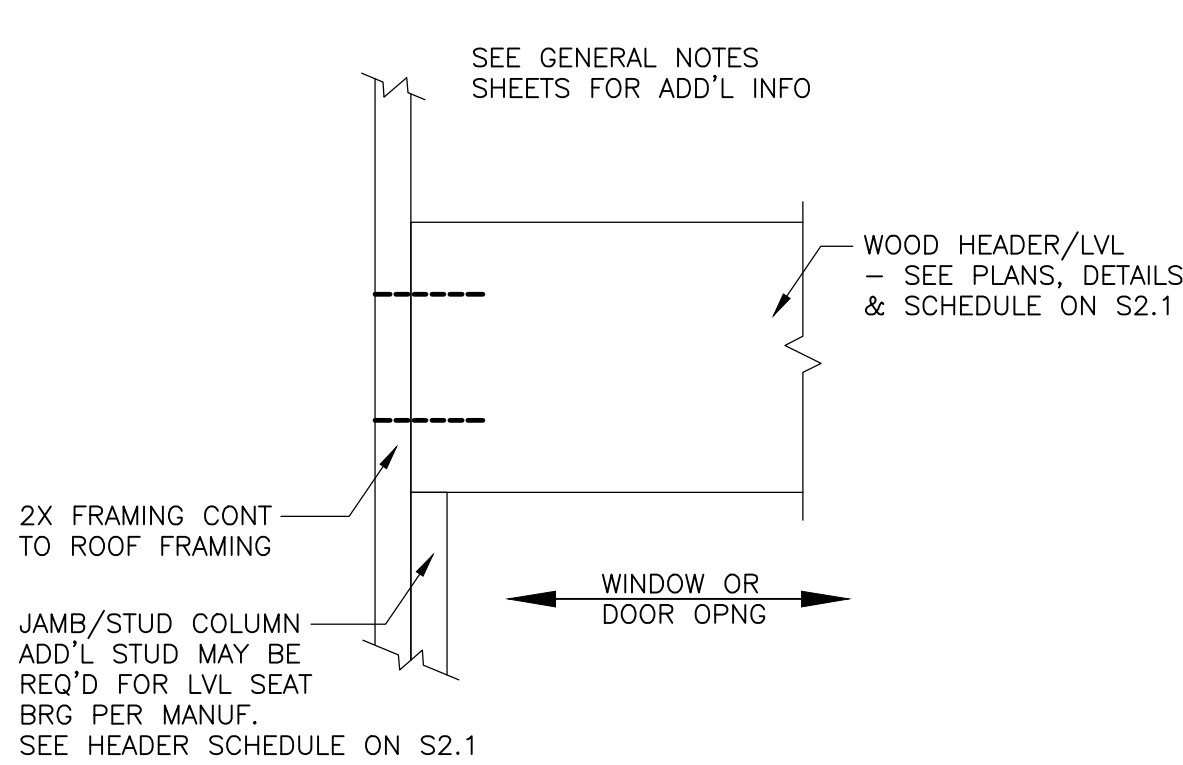
TYPICAL TRUSS BRG. END SECTION

SECTION 9
3/4"=1'-0"



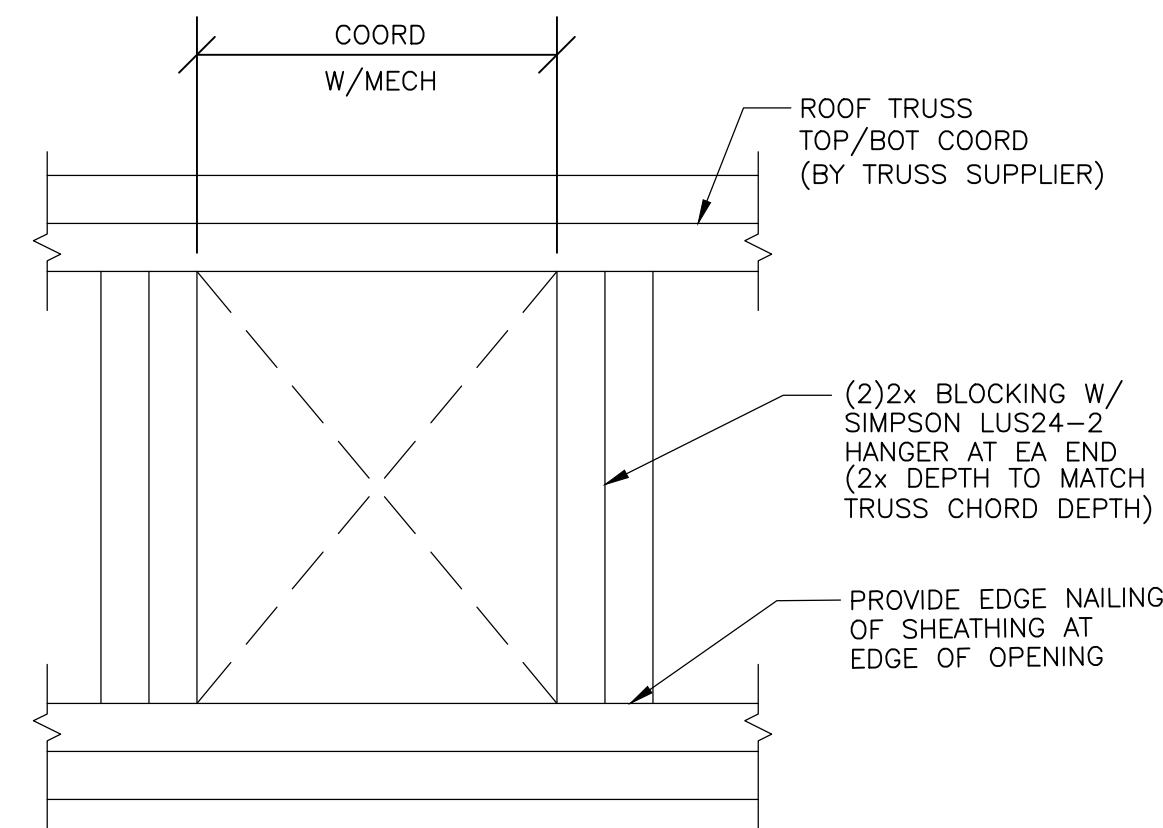
TYPICAL TRUSS END @ BEAM SECTION

SECTION 10
3/4"=1'-0"



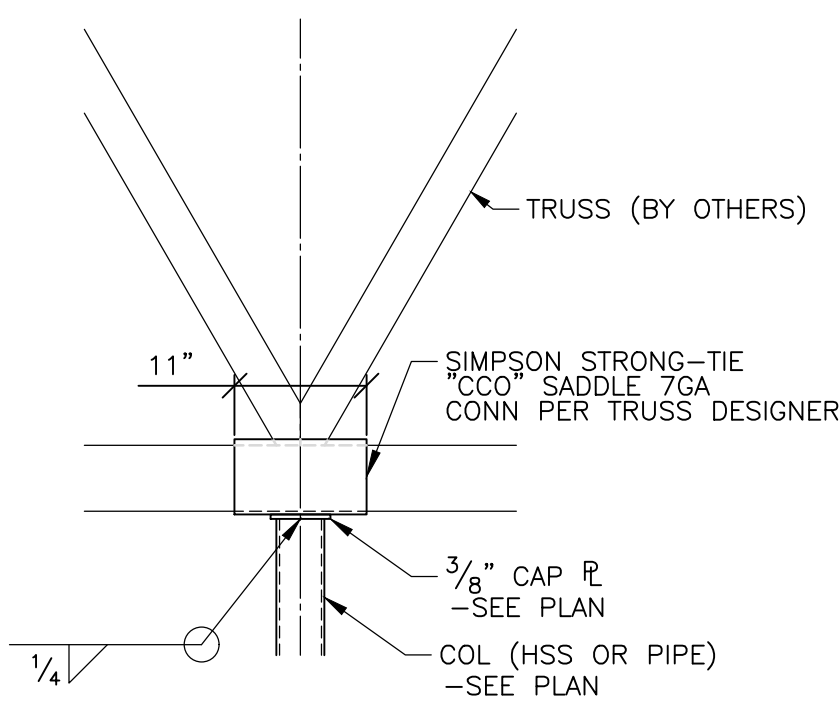
TYPICAL WOOD HEADER BEAM END DETAIL

SECTION 11
3/4"=1'-0"



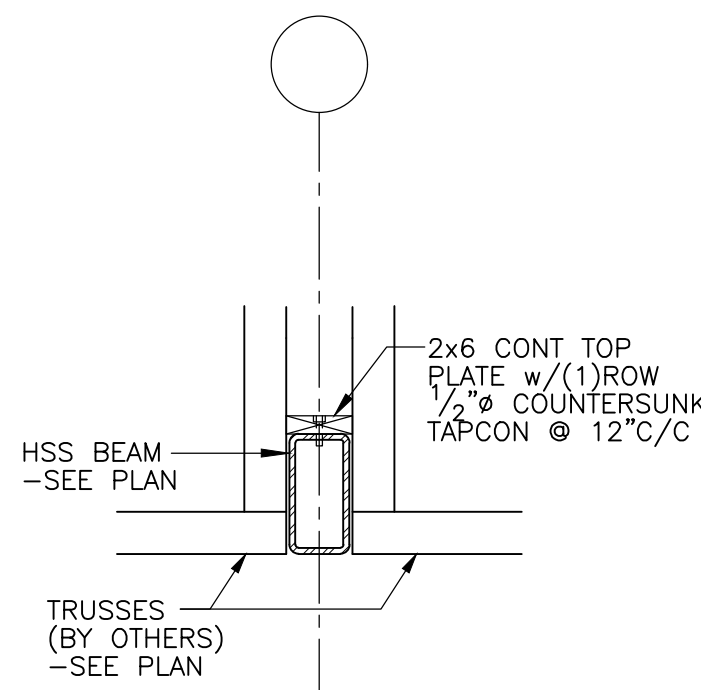
TYPICAL ROOF OPENING

SECTION 12
3/4"=1'-0"

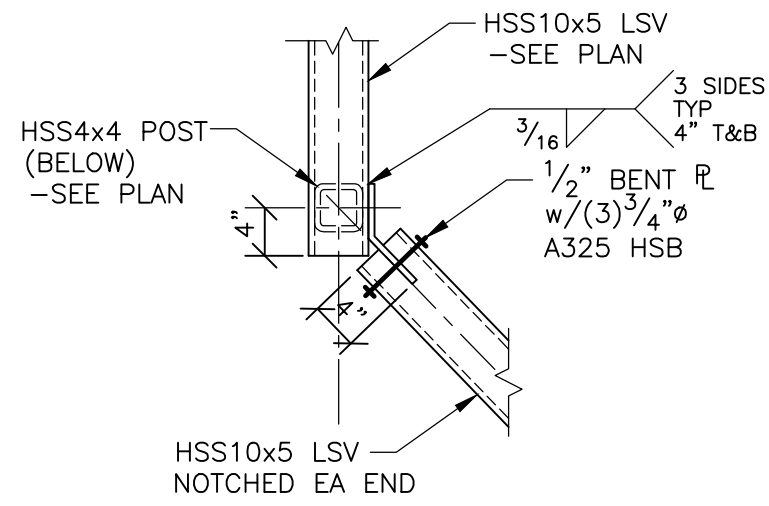


TYPICAL MID-SPAN POST @ GIRDER TRUSS

SECTION 13
3/4"=1'-0"



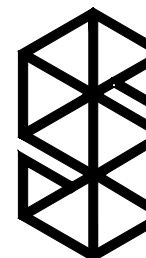
SECTION 14
3/4"=1'-0"



PLAN VIEW

SECTION 15
3/4"=1'-0"

NOTE:
SCALES INDICATED ARE BASED ON FULL SIZE 24"x36" DRAWING.
DIMENSIONS SHALL NOT BE DETERMINED BY COMPUTER SCALING OF DRAWINGS.

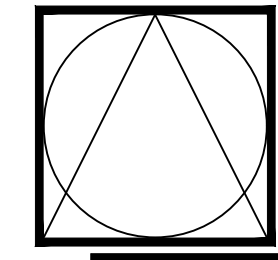


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DRAWING NAME
ROOF
FRAMING
DETAILS

DSCA PROJECT NO.
24.061

S3.2



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DATE:
PERMIT SUBMISSION:
06.19.26

BENNETT'S CREEK
HEALTH & REHABILITATION CENTER
1931/1915 BRIDGE ROAD (HIGHWAY 17)
SUFFOLK, VIRGINIA

